

1 Innovative Application of Waste Polyethylene  
2 Terephthalate (PET) Derived Additive as An  
3 Antistripping Agent for Asphalt Mixture:  
4 Experimental Investigation and Molecular Dynamics  
5 Simulation

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16 **Abstract:** Moisture-induced damage represents one of the major challenges to the durability of  
17 asphalt pavement, which may lead to various premature distresses, such as cracking, ravelling and  
18 potholes. To reduce the moisture-induced damages, antistripping agents have been commonly used  
19 in asphalt mixtures. On the other hand, the fast-growing waste polyethylene terephthalate (PET)  
20 from the end-of-life plastic products poses a serious problem to the environment. How to recycle  
21 and reuse the waste PET is becoming a major concern. This study aims to develop a value-added  
22 recycling approach of reusing waste PET as an antistripping agent for asphalt mixture. To achieve  
23 this objective, waste PET was first collected and went through an aminolysis reaction to yield the  
24 amine-based PET additive, which was used to modify bitumen. The infrared spectrum of the PET  
25 additive was then characterized, followed by the evaluation of moisture susceptibility through the  
26 boiling test and indirect tensile strength (ITS) test. In addition, molecular dynamics (MD)  
27 simulation was conducted to analyze the effect of the PET additive on the density and the cohesive  
28 energy density (CED) of the binder, and the interface bonding between binder and aggregate at the  
29 molecular level. The results from both experiments and MD simulation consistently indicate that  
30 the waste PET derived additive can effectively increase the resistance to moisture-induced damage  
31 of asphalt mixture.

32 **Keywords:** Polyethylene terephthalate (PET); Moisture damage; Adhesion; Antistripping;  
33 Molecular dynamics simulation

## 34 **1. Introduction**

35 Asphalt pavements are subjected to various damages caused by vehicle loads and environmental  
36 effects, among which, the moisture-induced damage is recognized as one of the most important  
37 factors affecting the durability of pavements [1]. Moisture-induced damage often leads to stripping

38 of asphalt mixture [2], as a result of the loss of interface adhesion between bitumen and aggregate  
39 or the reduction of cohesion within asphalt mastic under the effect of water [3]. It has been reported  
40 that moisture-induced damage could also accelerate premature failures of pavements such as  
41 cracking, ravelling and potholes [4, 5]. The potential of moisture-induced damage is largely  
42 dependent on the physico-chemical properties of bitumen and aggregate [6]. As most of the  
43 mineral aggregates are hydrophilic, their affinity to water normally exceeds that to bitumen [7-9].  
44 Thus, the binder film around aggregate is prone to be replaced by water as water diffuses to the  
45 asphalt-aggregate interface.

46 To reduce the moisture susceptibility, additives such as liquid fatty amines and hydrated lime have  
47 been applied as the antistripping agents for hot-mix asphalt [10, 11]. Due to the convenience of  
48 application, liquid amines are more favored [12]. Various studies have been conducted to  
49 investigate the effect of amine-based antistripping agents on moisture-induced damages of asphalt  
50 mixtures, and the results proved that these antistripping agents can effectively reduce the moisture  
51 susceptibility [13-15].

52 Polyethylene terephthalate (PET) is a plastic polymer, which is commonly used to make plastic  
53 bottles. The fast-growing consumption of plastic bottles has created a serious environmental  
54 problem in recent years. As the polymeric PET bottles are not bio-degradable, how to recycle and  
55 reuse the waste PET bottles poses a major challenge to environment. The application of waste PET  
56 in asphalt mixture provides a possible solution to reuse this waste material. Due to the fact that the  
57 melting point ( $\sim 250^\circ\text{C}$ ) of PET is very high [16], it is not feasible to prepare a homogeneous PET  
58 modified bitumen with the wet process. Instead, most previous studies used the dry process to  
59 prepare PET modified asphalt mixture, i.e., the PET flakes were added into asphalt mixture directly  
60 during the mixing of asphalt binder and aggregate. It was found that PET increased the fatigue life

61 of asphalt mixtures to some extent, while the resistance to moisture damage decreased [17-19],  
62 which was mainly due to the poor adhesion between asphalt binder and the PET flakes. On the  
63 other hand, chemical treatment of waste PET for the preparation of PET derived additive has been  
64 found to be a promising alternative to the aforementioned dry process [20-22]. Recently, the  
65 aminolysis product of waste PET was employed to modify bitumen by Leng, et al. [20, 23], and  
66 the results highlighted that the PET derived additive significantly increased the adhesion between  
67 asphalt binder and aggregate, therefore reducing the moisture-induced damage. Nevertheless, the  
68 adhesion mechanism between the PET additive modified asphalt binder and aggregate has not been  
69 fully understood yet. Previous studies have indicated that molecular dynamics (MD) simulation  
70 can provide fundamental insights in understanding the interface adhesion between binder and  
71 aggregate from the molecular or atomic level [24, 25]. It is therefore meaningful to explore the  
72 interface performance between the PET additive modified binder and aggregate employing the  
73 MD simulation.

74 MD simulation has been widely used to study various properties of asphalt binder and mixture,  
75 including the density, cohesion, and particularly adhesion properties [25-28]. Xu and Wang [24]  
76 investigated the interface adhesion between binder and aggregate using the bond energy  
77 parameters in both dry and wet conditions obtained from MD simulation. Their results indicated  
78 that both the aggregate and binder types affect the interfacial adhesion. The study by Yao, et al.  
79 [29] indicated that aggregate tends to affiliate more to water than bitumen, as the adhesion energy  
80 between aggregate and water is larger than that between aggregate and bitumen binder. Zhang, et  
81 al. [21] recently verified that the glycolysis treated PET can improve the adhesion between binder  
82 and aggregate through MD simulation. The aminolysis product of waste PET showed strong  
83 antistripping ability in our previous study [20], and the MD simulation can be applied to provide

84 more in-depth understanding of the effect of PET additive on the interface adhesion between binder  
85 and aggregate from the molecular perspective.

86 The main objective of this research is to investigate the antistripping performance of the PET  
87 derived additive in asphalt mixture through combined experimental investigation and MD  
88 simulation. To achieve this objective, the aminolysis treated waste PET additive was first used to  
89 produce modified bitumen. The moisture susceptibilities of the PET additive modified bitumen  
90 and asphalt mixture were then evaluated experimentally through the boiling test and indirect tensile  
91 strength (ITS) test. Finally, the MD simulation was implemented to evaluate the density and  
92 cohesive energy density (CED) of the PET additive modified binder, and the adhesion between  
93 the binder and aggregate in both dry and wet conditions.

## 94 **2. Materials and methods**

### 95 **2.1 Materials**

96 In this study, the PET additive was obtained through aminolysis treatment of waste PET. Waste  
97 PET bottles were collected after proper identification. Triethylenetetramine (TETA) was  
98 purchased from Sigma-Aldrich. Neat bitumen with a penetration grade of 60/70 (Pen 60/70) was  
99 used, and its basic properties are presented in Table 1. Crushed granite aggregate and mineral filler  
100 were obtained from a local supplier. The aggregate gradation for WC10 (a dense-graded wearing  
101 course mixture with a nominal maximum aggregate size of 10mm commonly used in Hong Kong)  
102 was used as shown in Figure 1.

103 **Table 1. Basic properties of bitumen**

Penetration (25 °C, 0.1 mm)	Softening point (°C)	Viscosity at 135 °C (mPa·s)
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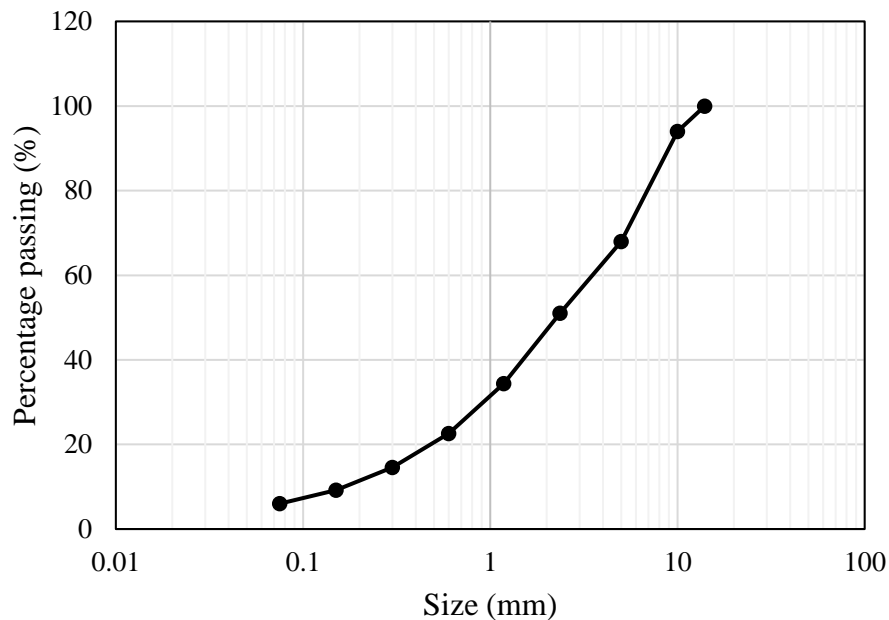
64.5

48.5

477.5

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104



105

106 **Figure 1.** Gradation of aggregate

## 107 **2.2 Aminolysis of waste PET**

108 The aminolysis of waste PET was carried out following the procedure as presented in Figure 2.

109 Prior to the aminolysis treatment, the PET bottles were first cleaned with normal detergent and

110 then cut into small flakes, which were subsequently dried in an oven at 80 °C for 2 h. A 500 ml

111 four-necked round flask equipped with a stirrer, reflux condenser and nitrogen gas, was heated in

112 an oil bath and charged with waste PET flakes and TETA. The weight ratio of PET to TETA was

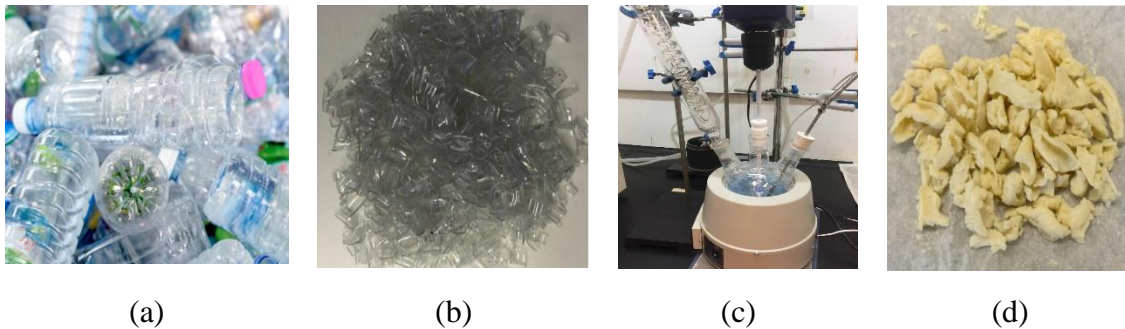
113 1:2 based on a previous study [20]. The aminolysis reaction of PET was conducted at around

114 140 °C for 2 h. After this treatment, the homogeneous product was cooled down to room

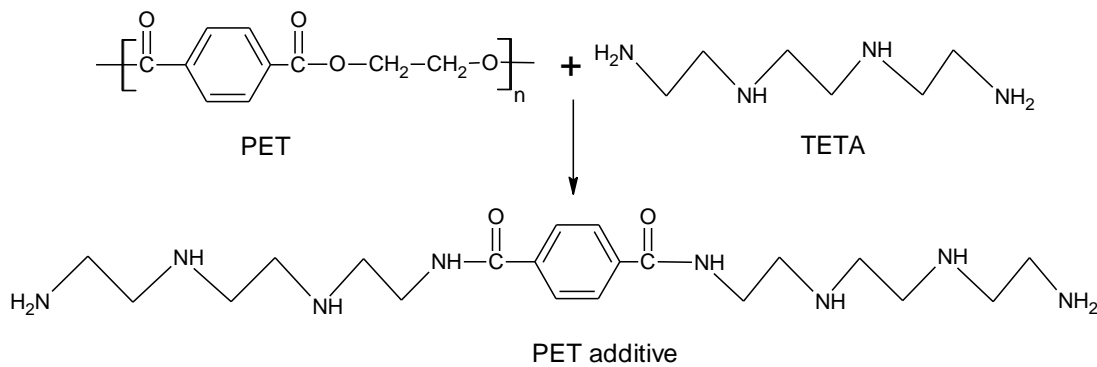
115 temperature and then filtrated with deionized water for at least three times to remove the excess

116 TETA and by-products (ethylene glycol. Finally, the substance remaining on the filter paper was

117 collected and dried in an oven at 60 °C for 2-3 h to obtain the PET additive. Figure 3 shows the  
118 chemical reaction between PET and TETA during aminolysis.



119  
120 **Figure 2.** Aminolysis treatment of waste PET bottles to produce PET additive: (a) waste PET  
121 bottles, (b) PET flakes, (c) aminolysis reaction, and (d) the prepared PET additive ready for use.



125 **Figure 3.** Aminolysis reaction of PET. The PET flakes reacted with TETA at a temperature of  
126 140 °C for 2 h to prepare the PET derived additive.

### 127 **2.3 Preparation of PET additive modified asphalt binder**

128 To prepare the PET additive modified asphalt binder, approximately 500 g of bitumen was first  
129 heated to fluid condition at 150 °C. The pre-weighted PET additive was then added into the binder  
130 and stirred at a speed of 500 rpm for 1 h. PET modified binders with two different weight  
131 percentages of PET additive, 1wt% and 3wt% based on bitumen, were prepared and denoted as  
PETB-1 and PETB-3, respectively. The neat binder Pen 60/70 without PET additive was used for  
comparison.

## 132 **2.4 Experimental characterizations**

133 Infrared spectra measurements were performed using a Bruker Tensor II Fourier Transform  
134 Infrared (FT-IR) spectrometer. The IR spectrum of PET additive was measured from  $600\text{ cm}^{-1}$  to  
135  $4,000\text{ cm}^{-1}$  with a resolution of  $4\text{ cm}^{-1}$ .

136 The conventional bitumen binder tests of penetration at  $25\text{ }^{\circ}\text{C}$ , softening point and rotational  
137 viscosity from  $120\text{ }^{\circ}\text{C}$  to  $175\text{ }^{\circ}\text{C}$  were carried out following ASTM D5, D36 and D4402,  
138 respectively.

139 The adhesion between bitumen and aggregates was evaluated through the boiling water test in  
140 accordance with ASTM D3625. Around 400 g of coarse aggregates with the size of 5-10 mm were  
141 mixed with bitumen for the boiling test. To prepare a layer of binder film with a thickness of 8-10  
142 microns, 2wt% of binder based on aggregates was applied [6, 30]. The mixtures were subsequently  
143 boiled for 10 min in a beaker. Finally, the boiled mixtures were scanned with a normal office  
144 scanner to obtain the images.

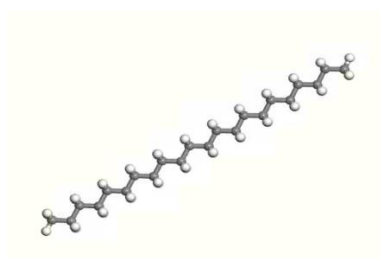
145 The moisture susceptibility of PET modified asphalt mixture was then evaluated through indirect  
146 tensile strength (ITS) test at  $20\text{ }^{\circ}\text{C}$  according to ASTM D4867. The compacted asphalt mixture  
147 with an air void of 6-8% was first prepared with the Marshall compactor. Two groups of specimens  
148 were fabricated and subjected to dry and wet conditioning, respectively. The dry group specimens  
149 were conditioned at room temperature ( $23 \pm 0.5\text{ }^{\circ}\text{C}$ ) for 24 h, and the wet group specimens were  
150 immersed in the  $60\text{ }^{\circ}\text{C}$  water for another 24 h following the dry conditioning procedure. The ITS  
151 tests for the two groups of specimens were then conducted, and the tensile strength ratio (TSR)  
152 between the wet conditioned strength and the dry strength were then calculated.



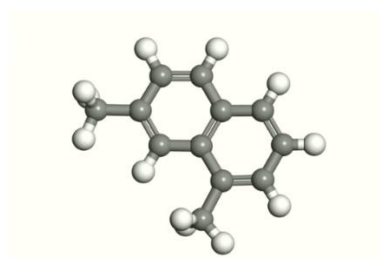
## 153 2.5. Molecular dynamics simulation

### 154 2.5.1 Molecular models

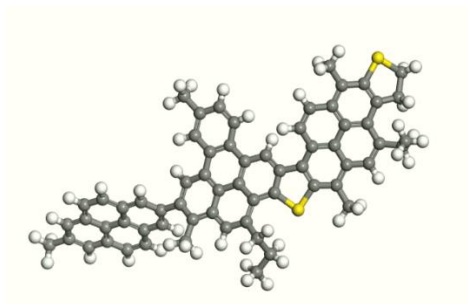
155 Bitumen, a byproduct originating from the crude oil distillation, is composed of various  
156 hydrocarbons. The main components of bitumen can be divided into asphaltenes, resins, aromatics,  
157 and saturates, based on their polarity and aromaticity [31]. Bitumen constitutes a wide range of  
158 different molecules whose structures are still not totally clear, which makes it challenging to study  
159 this material using MD simulation. Considering the complexity of bitumen, it is almost impossible  
160 to construct a molecular model that represents its accurate composition at present. Instead,  
161 simplified models that are composed of 3-12 representative molecule types were commonly used  
162 [32, 33]. Since the objective of this study is to evaluate the effect of PET additive on the  
163 antistripping performance of the binder, the three-component bitumen model [34] was adopted for  
164 its simplicity. The three components are asphaltene ( $C_{64}H_{52}S_2$ ), 1,7-Dimethylnaphthalene ( $C_{12}H_{12}$ ),  
165 and n-docosane ( $n-C_{22}H_{46}$ ), which represent asphaltene, naphthene aromatic and saturate,  
166 respectively. Figure 4 illustrates the molecular structures of the three-component bitumen model  
167 and that of the PET additive.



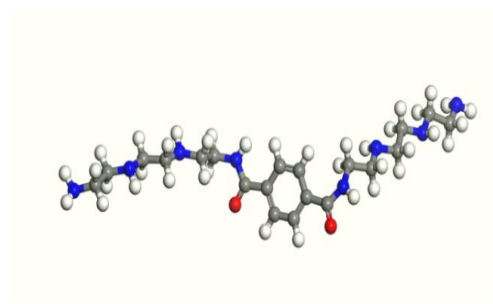
(a) n-docosane ( $n-C_{22}H_{46}$ )



(b) 1,7-dimethylnaphthalene ( $C_{12}H_{12}$ )



(c) Asphaltene ( $C_{64}H_{52}S_2$ )



(d) PET additive ( $C_{20}H_{38}O_2N_8$ )

168 **Figure 4.** Molecular structures of (a) n-docosane ( $n-C_{22}H_{46}$ ), (b) 1,7-dimethylnaphthalene ( $C_{12}H_{12}$ ),  
 169 (c) Asphaltene ( $C_{64}H_{52}S_2$ ), and (d) PET additive ( $C_{20}H_{38}O_2N_8$ ). Note that the grey atoms represent  
 170 C, the white atoms represent H, the yellow atoms represent S, the blue atoms represent N, and the  
 171 red atoms represent O. (Please refer to the online version for the colors of the atoms)

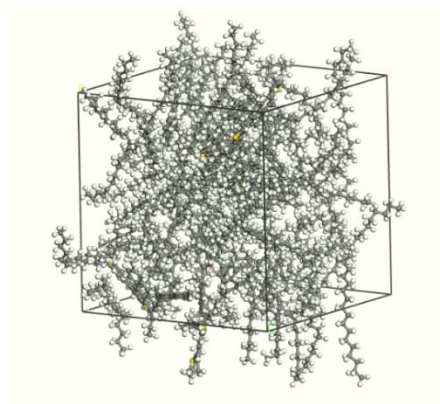
## 172 2.5.2 Bulk binder model

173 The molecular compositions of the neat bitumen and PET modified bitumen (PETB) are presented  
 174 in Table 2, the number of molecules were selected based on previous studies [34-36]. The mass  
 175 fractions of asphaltene, 1,7-dimethylnaphthalene, and n-docosane in the neat bitumen are 20.7wt%,  
 176 19.7wt%, and 59.6wt%, respectively. According to the experiment results, 1wt% of PET additive  
 177 could already achieve good antistripping performance, thus only 1 PET additive molecule was  
 178 added to construct the PETB model, which corresponds to 1.97wt% of PET additive based on  
 179 bitumen.

180 **Table 2. Molecular compositions of binder**

Sample-ID	Asphaltene ( $C_{64}H_{52}S_2$ )	1,7-dimethylnaphthalene ( $C_{12}H_{12}$ )	n-docosane ( $n-C_{22}H_{46}$ )	PET additive	PET additive wt%
Neat bitumen	5	27	41	/	/
PETB	5	27	41	1	1.97

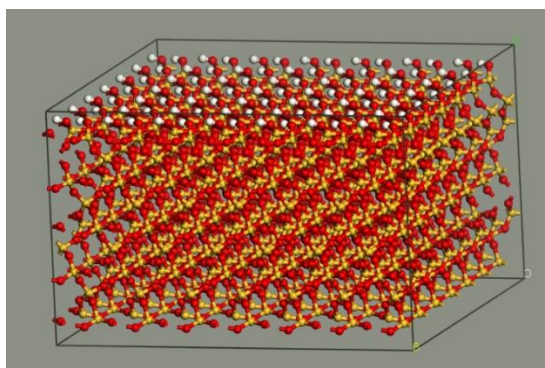
181 In this study, the commercial software BIOVIA Materials Studio was used to perform the  
182 molecular dynamics (MD) simulation. The COMPASS (Condensed-phase Optimized Molecular  
183 Potentials for Atomistic Simulation Studies) force field, developed using *ab initio* and empirical  
184 parameterization techniques by Sun [37], has been commonly applied to describe the interactions  
185 of bituminous materials [38-40]. The Nose-Hoover-Langevin (NHL) thermostat and the Andersen  
186 barostat were employed throughout the simulation to control the temperature and pressure,  
187 respectively. A cut-off distance of 15.5 Å was used to speed up the computation. To build the bulk  
188 binder model, an amorphous cell with an initial density of 0.1 g/cm<sup>3</sup> in 3D periodical condition  
189 was constructed first. After a geometry optimization process using the steepest descent algorithm,  
190 the system was then annealed for five cycles from 300 K to 500 K. Following a pre-equilibrium  
191 run for 100 ps with a time step of 1 fs under canonical ensemble (NVT), the system went through  
192 a 200 ps relaxation and shrank to a stable state with the isothermal-isobaric (NPT) ensemble.  
193 Finally, another simulation run of 100 ps was performed with the canonical ensemble to further  
194 equilibrate the system (Figure 5). Unless additionally stated, the temperature and pressure were set  
195 as 298 K and 0.0001 GPa (1atm), respectively, during the simulation.



196  
197 **Figure 5.** Bulk bitumen model after equilibrium. A total of 73 molecules are included, containing  
198 4026 atoms. (Please refer to the online version for the colors of the atoms)

199 2.5.3 Aggregate-bitumen interface model

200 Silica ( $\text{SiO}_2$ ), commonly used in the aggregate model for asphalt mixture simulation, was adopted  
201 in this study to represent aggregate. A unit silica model with the lattice parameters of  $a = b = 4.91$   
202  $\text{\AA}$ ,  $c = 5.402 \text{\AA}$ ,  $\alpha = \beta = 90^\circ$ , and  $\gamma = 120^\circ$  normally applied to build the siliceous aggregate model  
203 was selected in this study. The unit silica model was first cleaved in the (1 0 0) surface, which was  
204 then used to build a supercell with the size of  $35\text{\AA} \times 35\text{\AA} \times 23\text{\AA}$  (Figure 6). Geometry optimization  
205 of the supercell was performed before constructing the aggregate-bitumen interface. It was  
206 reported that the surface silicone atoms in siliceous aggregate are attached with hydroxyl groups  
207 (-OH) [41]. According to previous studies, the density of the surface hydroxyl groups was set as  
208  $4.56 \text{ OH/nm}^2$  [42, 43].



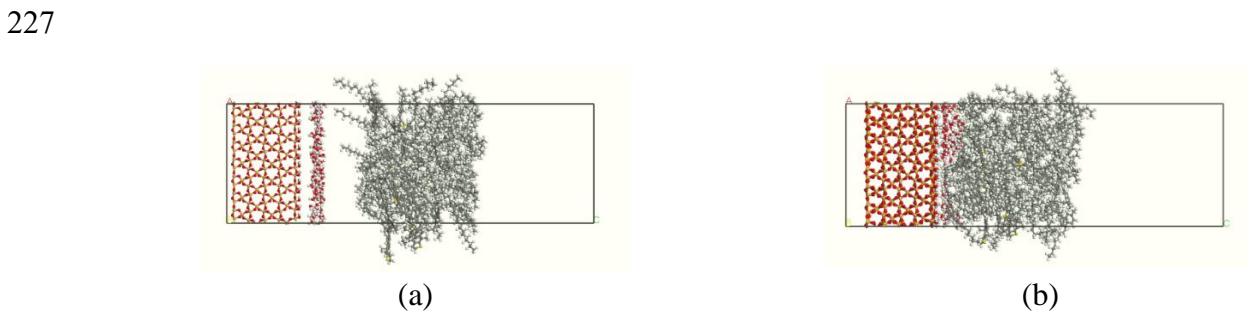
209 **Figure 6.** Supercell of  $\text{SiO}_2$ . The size of the  $\text{SiO}_2$  supercell is  $35\text{\AA} \times 35\text{\AA} \times 23\text{\AA}$ , and the surface  
210 silicon atoms are attached with hydroxyl groups -OH.  
211

212 The aggregate-bitumen interface was built by placing a confined bitumen layer on top of the  
213 aggregate layer. To avoid the interaction across the mirror image in the z-direction, a vacuum layer  
214 of  $30 \text{\AA}$  was added above the bitumen layer. After geometry optimization, the interface was  
215 equilibrated at the canonical ensemble for 200 ps. Figure 7 shows the initial configuration and the  
216 equilibrated configuration of the interface model in dry condition.

217 To evaluate the influence of water on the properties of binder-aggregate interface, 200 water  
218 molecules were placed between the aggregate layer and bitumen layer, based on the experience of  
219 the previous studies [24, 44]. The interface model with water was then subjected to the same  
220 equilibrium procedure as described for the interface model without water. Figure 8 shows the  
221 initial configuration and the equilibrated configuration of the interface model in wet condition.  
222



223  
224 **Figure 7.** Initial configuration (a) and equilibrated configuration (b) of the interface model in dry  
225 condition. A vacuum layer of Å was placed on top of the bitumen layer to avoid interaction across  
226 the mirror image.

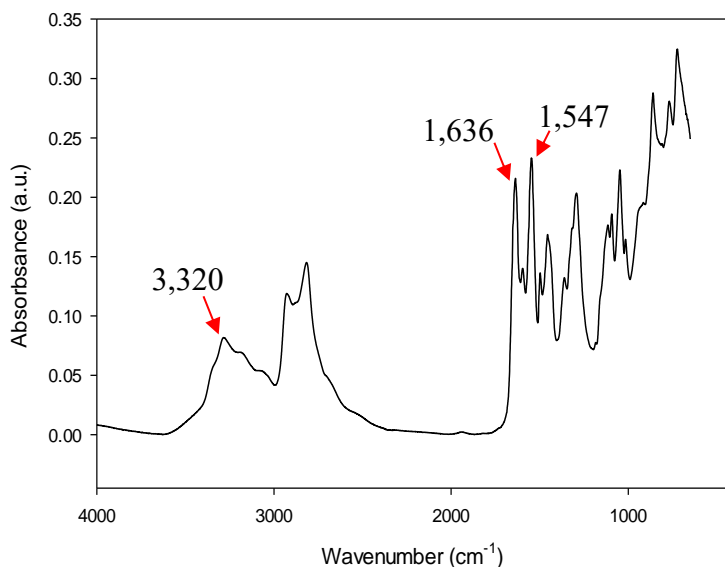


227  
228 **Figure 8.** Initial configuration (a) and equilibrated configuration (b) of the interface model in wet  
229 condition. 200 water molecules were inserted between the aggregate layer and bitumen layer.

### 230 3. Experimental results

#### 231 3.1 FT-IR test

232 Figure 9 illustrates the FT-IR spectrum of the PET additive. It can be observed that the distinct  
233 peak at  $1,720\text{ cm}^{-1}$  corresponding to the carbonyl ( $\text{-C=O}$ ) stretching vibration of the ester group in  
234 PET molecules disappeared, while the two peaks at  $1,636\text{ cm}^{-1}$  and  $1,547\text{ cm}^{-1}$  corresponding to  
235 the characteristic carbonyl ( $\text{-C=O}$ ) stretching vibration and N-H bending vibration of the amide  
236 group in PET additive, respectively, emerged. In addition, the peak at  $3,320\text{ cm}^{-1}$  attributed to the  
237 stretching vibration of the N-H in the amine group also showed up. This result indicated the  
238 successful aminolysis of the waste PET by TETA.



239  
240  
241 **Figure 9.** FT-IR spectrum of PET additive. The peaks at  $1,636\text{ cm}^{-1}$  and  $1,547\text{ cm}^{-1}$  were attributed  
242 to the carbonyl ( $\text{-C=O}$ ) stretching vibration and N-H bending vibration of the amide group in PET  
243 additive, respectively, and the peak at  $3,320\text{ cm}^{-1}$  corresponds to the stretching vibration of the N-  
244 H in amine group.

245 **3.2 Basic properties of binders**

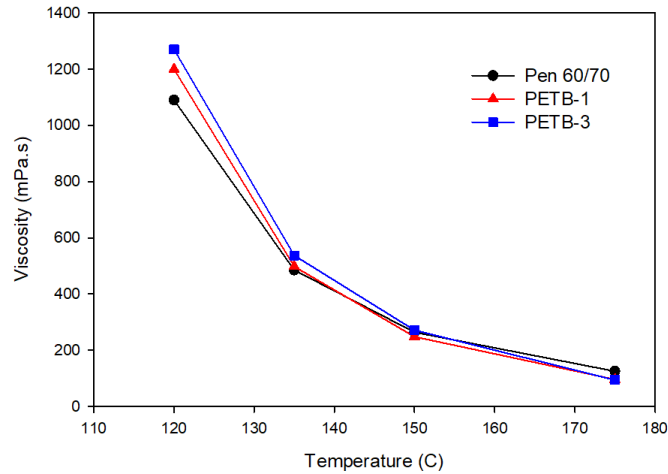
246 Table 3 compares the values of penetration and softening point of the base bitumen and the PET  
247 additive modified bitumens. It can be seen that penetration decreased while the softening point  
248 increased slightly with the incorporation of the PET additive. In addition, the binders with 1wt%  
249 and 3wt% of PET additive resulted similar values of penetration and softening point.

250 **Table 3. Penetration and softening point of binders**

Sample-ID	Penetration (25 °C, 0.1 mm)	Softening point (°C)
Pen 60/70	64.5	48.5
PETB-1	53.6	50.0
PETB-3	53.2	49.6

251

252 The viscosity of PET additive modified binders as a function of temperature from 120-175 °C is  
253 shown in Figure 10. It is observed that the viscosity increased a little bit with addition of PET  
254 additive at lower temperature (120 °C). With the increase of temperature, the viscosity become  
255 similar for all the binders. In fact, the viscosity did not show much difference with temperatures  
256 above 150 °C. The underlying reason is that the PET additive may form a crystal at ambient  
257 temperature [23], due to the linearity of its molecule structure and the presence of amino groups  
258 that leads to hydrogen bonds between the molecules. When temperature increases, the additive  
259 melts and turns into liquid. Overall, it is concluded from these tests that the PET additive does not  
260 have much influence on the basic properties of bitumen binder.



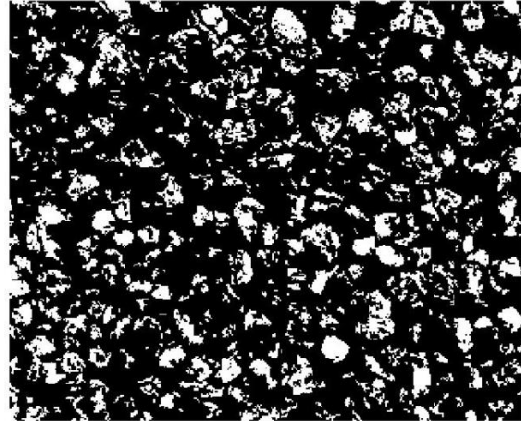
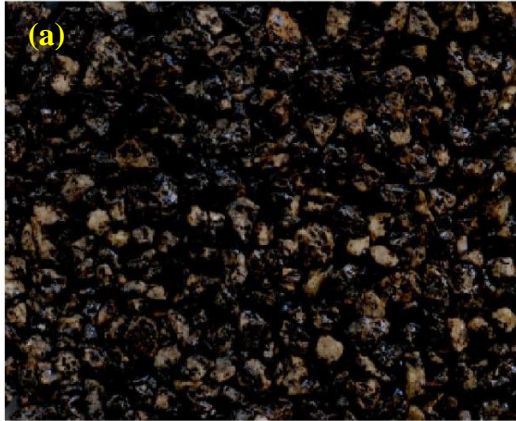
261  
 262 **Figure 10.** Viscosity measurement as a function of temperature

263 **3.3 Boiling test**

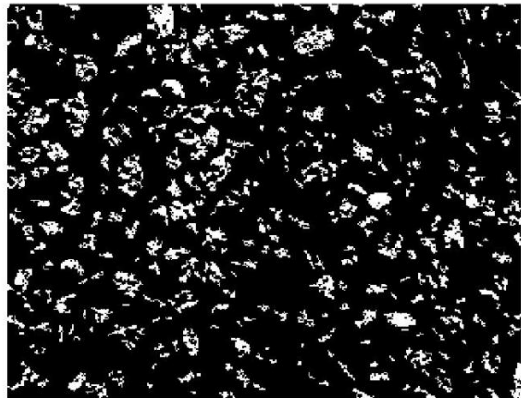
264 The images of the binder coated aggregate after the boiling tests were analyzed to compare the  
 265 moisture susceptibility of PET additive modified asphalt binders. Figure 11 presents the original  
 266 scanned images after boiling for 10 min and the corresponding binary images of the loose asphalt  
 267 mixtures. The binary images were obtained through image analysis using MATLAB®, and the  
 268 white area ratios representing the percentages of the stripped aggregates were computed [45] as  
 269 shown in Figure 12. What stands out in the graph is that the boiling loss area for the control mixture  
 270 was around 18.0%, which dropped considerably to around 10.0% with only 1wt% of PET additive.  
 271 With further increasing of the PET additive dosage, the boiling loss area did not change much. As  
 272 aforementioned, the PET additive may form crystal structures. When the concentration of PET  
 273 additive increases, part of the additive can attract with each other and the amount of PET additive  
 274 at the bitumen-aggregate interface may not change significantly. Therefore, the moisture  
 275 susceptibility of the asphalt mixture can be effectively reduced with the incorporation of 1wt% of  
 276 PET additive.



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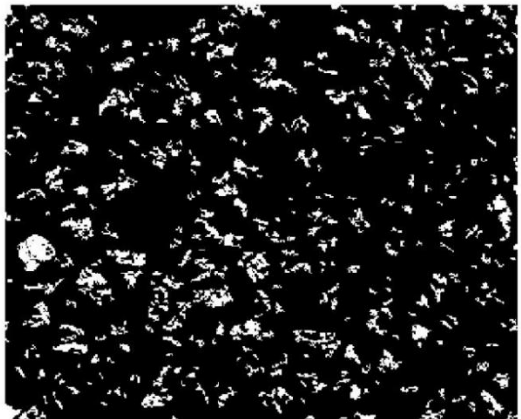


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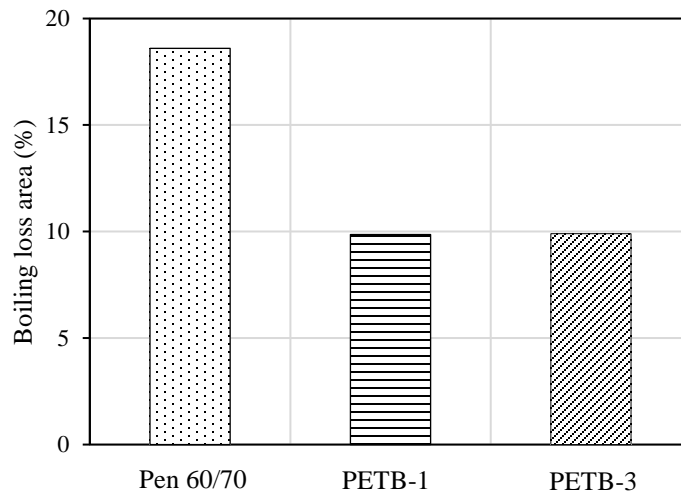
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**Figure 11.** Original (left) and binary (right) images after boiling tests: (a) Pen 60/70, (b) PETB-1, and (c) PETB-3.



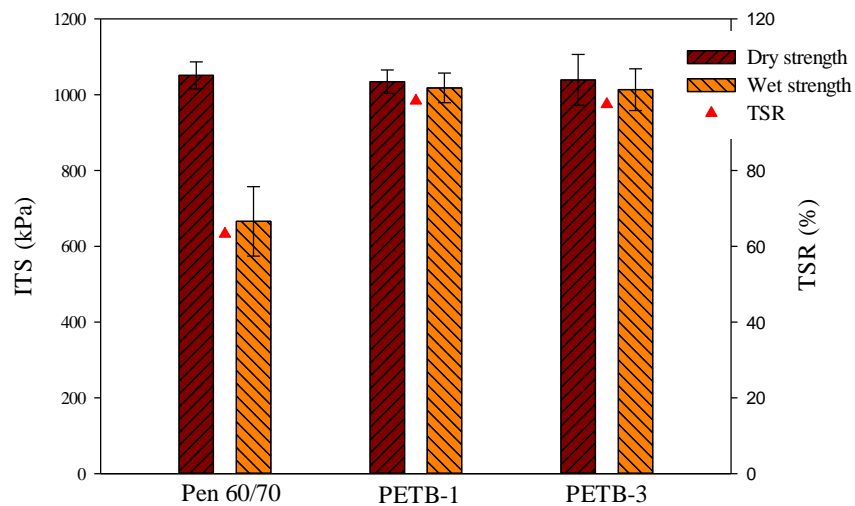
283

284 **Figure 12.** Comparison of boiling loss area.

285 **3.4 ITS test**

286 To further evaluate the effect of PET additive on the resistance to moisture-induced damage of  
 287 asphalt mixture, the ITS tests were conducted on the compacted asphalt mixtures containing  
 288 different dosages of PET additive. Figure 13 shows that the ITS values for the dry group were very  
 289 close, around 1,000 kPa. However, after conditioning in water at 60 °C for 24 h, the ITS of the  
 290 control asphalt mixture decreased sharply to 665.9 kPa, while there is no significant change in the  
 291 wet conditioned ITS values for the PET modified asphalt mixtures. The TSR for the control asphalt  
 292 mixture was around 60%, which increased dramatically to over 90% with the incorporation of only  
 293 1wt% of PET additive. With further increase of the PET additive dosage, the TSR did not change  
 294 much. The fracture interface images of the samples in the wet-conditioned group after the ITS test  
 295 are presented in Figure 14. It is clear that most of the Pen 60/70 bitumen was stripped off from the  
 296 aggregate surface (Figure 14(a)), representing a distinct adhesive failure. However, no significant  
 297 stripping was observed in the asphalt mixtures containing PET additive, indicating that the failure  
 298 occurred within the bitumen binder or mastic phase, which is typically cohesive failure. Such

299 phenomenon indicated that the PET additive effectively increased the resistance to moisture-  
 300 induced damage of asphalt mixture. The granite aggregate is acid, and the base bitumen binder is  
 301 also acidic because of the presence of carboxylic and/or sulfonic groups [46]. Thus, the bonding  
 302 between the granite aggregate and base bitumen binder is poor. The PET additive contains amine  
 303 groups, which is an alkaline additive. The PET additive is expected to increase the bonding  
 304 between the granite and bitumen binder for the following two reasons: 1) the amine groups in PET  
 305 additive can form the strong hydrogen bonds with the hydroxyl groups at the aggregate surface,  
 306 and 2) the alkaline amine groups can also increase the interaction between the granite aggregate  
 307 and bitumen binder.



308 **Figure 13.** ITS of asphalt mixtures with different concentrations of PET additive at 20 °C (TSR  
 309 represents the ratio between the wet tensile strength to the dry tensile strength).  
 310

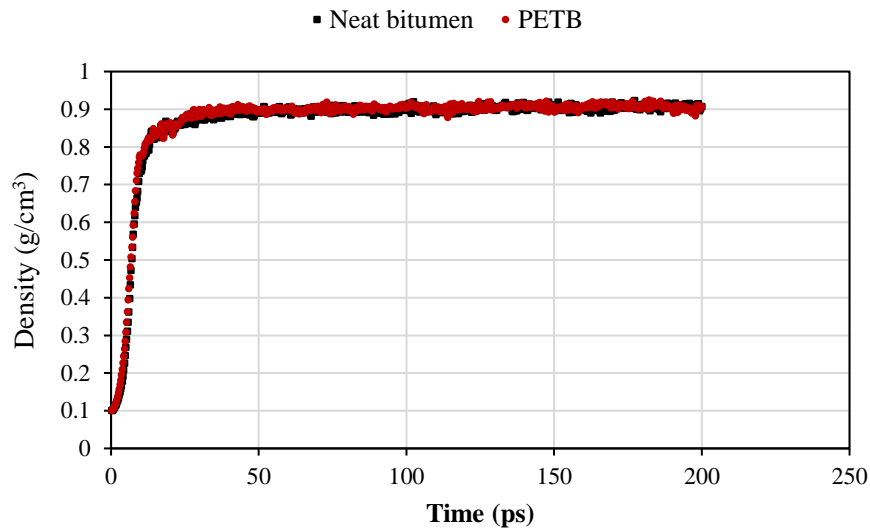


312 **Figure 14.** Fracture interfaces for the asphalt mixtures that had been subjected to 24 h water  
313 conditioning at 60 °C: (a) Pen 60/70, (b) PETB-1, and (c) PETB-3.

#### 314 4. MD simulation

##### 315 4.1 Density

316 During the isothermal-isobaric (NPT) ensemble equilibration, the volume of the bitumen system  
317 decreased gradually, and the density increased accordingly. The density evolutions of the neat  
318 bitumen and PET additive modified bitumen (PETB) are shown in Figure 15. It can be seen that  
319 the densities of both neat bitumen and PETB increased sharply within the first 30 ps and then  
320 stabilized after 50 ps. Considering the low content of the PET additive, no significant difference  
321 was observed between the densities of the two binders. The equilibrated densities (averaged over  
322 the last 50 ps) of the neat bitumen and PETB are 0.906 g/cm<sup>3</sup> and 0.907 g/cm<sup>3</sup>, respectively. The  
323 densities are consistent with those in the previous studies using the three-component bitumen  
324 model [24, 34].



325

326 **Figure 15.** Densities of neat bitumen and PETB during equilibrium. The isothermal-isobaric (NPT)  
 327 ensemble was implemented.

#### 328 **4.2. Cohesive energy density (CED) of binder**

329 The cohesive energy density (CED) is used to quantify the magnitude of attraction between  
 330 different molecules inside a substance. It is obtained from the difference of the total nonbond  
 331 energy and the intramolecular nonbond energy, and the square root of CED is defined as the  
 332 solubility parameter  $\delta$  (Eq. (1)).

$$333 \quad \delta = \sqrt{CED} \quad (1)$$

334 Table 4 presents the computed CED of the neat bitumen and PETB. With the incorporation of PET  
 335 additive, the CED of PETB became larger than that of the neat bitumen, indicating that PET  
 336 additive increased the molecular interaction. The result could be explained by that the PET additive  
 337 is more polar than bitumen binder, thus both the van der Waals force and electrostatic force  
 338 between different molecules increased, leading to stronger cohesion of PETB.

339 **Table 4. Cohesive energy density (CED) of binder**

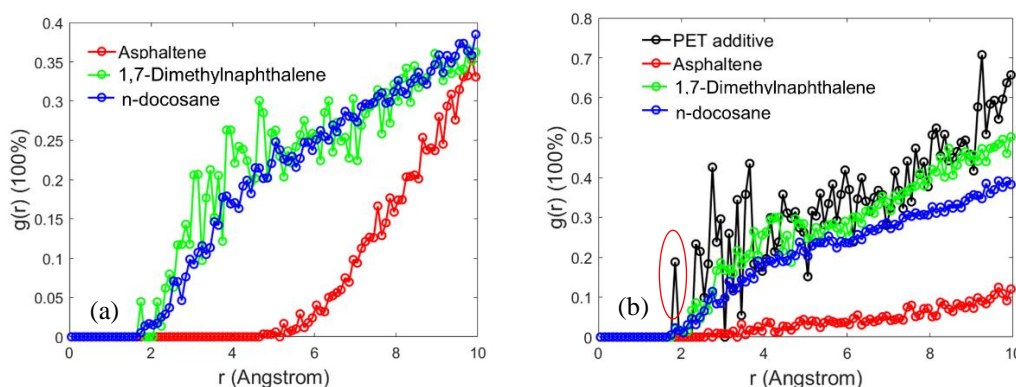
Sample-ID	Van der Waals ( $10^6\text{J/m}^3$ )	Electrostatic ( $10^6\text{J/m}^3$ )	CED ( $10^6\text{J/m}^3$ )	Solubility Parameter ( $(\text{J/cm}^3)^{0.5}$ )
Neat bitumen	326.4	-1.221	332.4	18.232
PETB	334.5	0.610	342.6	18.509

340 Note: PETB represents the PET additive modified bitumen, in which 1 PET additive molecule was  
 341 added into the bitumen model, corresponding to 1.97wt% of the PET additive based on bitumen.

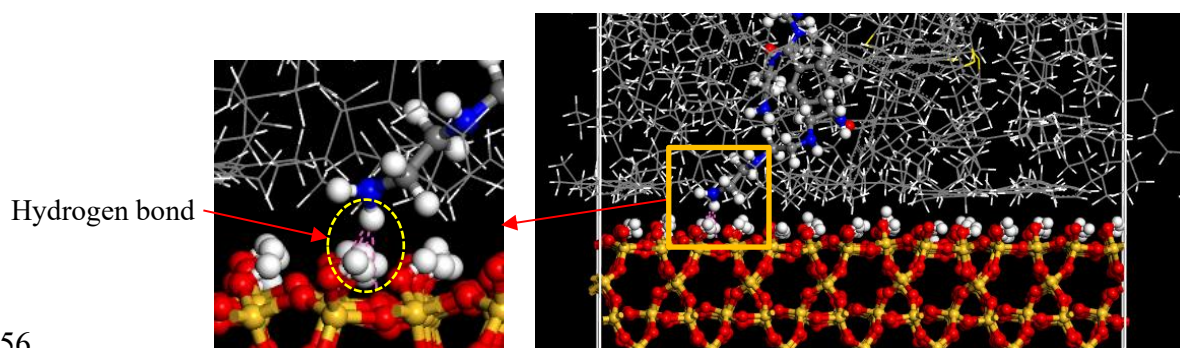
#### 342 **4.3 Radial distribution**

343 Figure 16 depicts the radial distribution of each component in the binder with respect to the  
 344 aggregate surface. It can be seen that the concentration of 1,7-dimethylnaphthalene is the highest  
 345 at the interface for the neat bitumen, followed by n-docosane and asphaltene. The first peak at

346 around 2 Å in Figure 16(b) belongs to the PET additive in PETB, indicating that the PET additive  
 347 was the closest to the aggregate surface. As demonstrated in Figure 17, the amine groups in PET  
 348 additive were very close to the aggregate surface. Two reasons may contribute to such  
 349 phenomenon: (1) the higher polarity of PET additive makes it more affiliate to the polar aggregate  
 350 surface, and (2) the amine groups in the PET additive and the hydroxyl groups in the aggregate  
 351 surface can form hydrogen bond, which is much stronger compared with other non-bonded  
 352 interactions.



353  
 354 **Figure 16.** Radial distribution of each component respect to the aggregate surface: (a) neat bitumen;  
 355 (b) PETB.



356  
 357 **Figure 17.** Interfacial interaction of PETB and aggregate.

#### 358 4.4 Work of adhesion between binder and aggregate

359 The work of adhesion  $W_{adhesion}$  is defined as the work required to separate the binder from  
360 aggregate at the interface, which determines the resistance to the interface detachment. A negative  
361 value of  $W_{adhesion}$  indicates attraction of the two parts, whereas a positive value indicates repulsion.  
362 The work of adhesion  $W_{adhesion}$  is given by Eq. (2), and the adhesion energy  $\Delta E_{adhesion}$  is obtained  
363 from Eq. (3).

$$364 \quad W_{adhesion} = \Delta E_{adhesion} / A \quad (2)$$

$$365 \quad \Delta E_{adhesion} = E_{total} - (E_{binder} + E_{aggregate}) \quad (3)$$

366 where  $E_{total}$  is the total potential energy of the binder and aggregate interface system after  
367 equilibration,  $E_{binder}$  and  $E_{aggregate}$  represent the potential energies of the binder and the aggregate  
368 separated at the equilibrium state, respectively, and  $A$  is the interface contact area.

369 Because of the hydrophilic nature of mineral aggregate, water is more likely to be absorbed onto  
370 the aggregate surface. As a result, asphalt binder is prone to detach from the aggregate in the  
371 presence of water. The adhesion energy  $\Delta E_{adhesion\_water}$  and the work of adhesion  $\Delta W_{adhesion\_water}$  in  
372 the presence of water are calculated using Eq. (4) and Eq. (5), respectively. A lower value of  
373  $\Delta E_{adhesion\_water}$  represents a lower energy potential for water to displace the binder from the  
374 aggregate surface [47], or the displacement is more difficult to happen and the resistance to  
375 moisture-induced damage is stronger.

$$376 \quad \Delta E_{adhesion\_water} = \Delta E_{bit\_water} + \Delta E_{agg\_water} - \Delta E_{bit\_agg} \quad (4)$$

$$377 \quad \Delta W_{adhesion\_water} = \Delta E_{adhesion\_water} / A \quad (5)$$

378 where  $\Delta E_{bit\_water}$  is the interaction energy between binder and water,  $\Delta E_{agg\_water}$  is the interaction  
 379 energy between aggregate and water, and  $\Delta E_{bit\_agg}$  is the interaction energy between binder and  
 380 aggregate. All the energies were obtained at the equilibrium state.

381 The energy ratio (ER), i.e., the ratio between the work of adhesion in dry condition and the work  
 382 of adhesion in wet condition, is usually used to represent the moisture susceptibility of asphalt  
 383 mixture [43, 47]. A larger value of ER indicates lower moisture sensitivity. The ER is given as,

$$384 \quad ER = W_{adhesion} / W_{adhesion\_water} = \Delta E_{adhesion} / \Delta E_{adhesion\_water} \quad (6)$$

385 Table 5 shows the adhesion energy and work of adhesion between binder and aggregate in the dry  
 386 condition.  $\Delta E_{vdw}$  and  $\Delta E_{elec}$  represent the energy difference associated with the van der Waals part  
 387 and the electric part, respectively. Since both bitumen and aggregate possess high surface energy  
 388 in isolated condition, when they attach to each other to form an interface, the total surface area  
 389 becomes smaller, thus decreasing the potential energy of the system. This is the reason why the  
 390 interaction energies in the table are negative. It is clear to see that the absolute value of the adhesion  
 391 energy  $\Delta E_{adhesion}$  and work of adhesion  $W_{adhesion}$  for PETB and aggregate are larger than that of  
 392 neat bitumen and aggregate, indicating that PET additive increased the interfacial adhesion  
 393 between the binder and aggregate. Furthermore, it is observed that both the van der Waals and  
 394 electric energy increased with the addition of PET additive. The increase of interface adhesion can  
 395 be mainly attributed to the polarity of the PET additive and the formation of hydrogen bonding  
 396 between the PET additive and aggregate surface hydroxyl groups.

397 **Table 5. Adhesion energy of binder and aggregate in dry condition**

Sample-ID	$\Delta E_{vdw}$ (kcal/mol)	$\Delta E_{elec}$ (kcal/mol)	$\Delta E_{adhesion}$ (kcal/mol)	$W_{adhesion}$ (mJ/m <sup>2</sup> )
Agg_NB	-201.201	-1.897	-208.03	-118.03



Agg_PETB	-213.066	-8.658	-226.81	-128.68
----------	----------	--------	---------	---------

398 **Note:** Agg\_NB represents the interface between aggregate and neat bitumen in dry condition, and  
399 Agg\_PETB represents the interface between aggregate and PET additive modified bitumen in dry  
400 condition.

401  
402 Table 6 presents the adhesion energy in wet condition as well as the calculated energy ratio. It can  
403 be noticed that the absolute value of  $\Delta E_{agg\_water}$  is larger than that of  $\Delta E_{bit\_agg}$ , which  
404 demonstrates that water is more likely to be absorbed to the aggregate surface than bitumen binder  
405 due to the hydrophilic nature of the mineral aggregate. As shown in Table 6, the  $\Delta E_{adhesion\_water}$   
406 of the aggregate-water-PETB system is about 21% lower than that of the aggregate-water-NB  
407 system, indicating that the coating of PETB on the aggregate is harder to be replaced by water  
408 compared with neat bitumen. On the contrary, the ER of the PETB system reached 1.24, which is  
409 larger than that of the neat bitumen system (0.88). The simulation results revealed that the moisture  
410 sensitivity of the asphalt mixture was decreased by the incorporation of PET additive, which is  
411 consistent with the findings obtained from the preceding experiments.

412 **Table 6. Adhesion energy of binder and aggregate in wet condition and the calculated ER**

Sample-ID	$\Delta E_{bit\_water}$ (kcal/mol)	$\Delta E_{agg\_water}$ (kcal/mol)	$\Delta E_{bit\_agg}$ (kcal/mol)	$\Delta E_{adhesion\_water}$ (kcal/mol)	ER
Agg_water_NB	-105.1	-200.0	-73.9	-231.3	0.88
Agg_water_PETB	-145.1	-158.3	-120.5	-182.9	1.24

413 **Note:** Agg\_water\_NB represents the interface between aggregate and neat bitumen in wet  
414 condition, and Agg\_water\_PETB represents the interface between aggregate and PET additive  
415 modified bitumen in wet condition.

## 416 **5. Conclusions**

417 In this study, the antistripping ability of the waste PET derived additive on asphalt mixture was  
418 investigated through both laboratory tests and MD simulation. The following points summarize  
419 the main conclusions of this study:

- 420 • The experimental results indicated that the boiling stripping area decreased significantly  
421 with the incorporation of PET additive. In addition, the TSR increased dramatically to over  
422 90% with only 1wt% of PET additive, and the failure mode transferred from adhesive  
423 failure to cohesive failure.
- 424 • The MD simulation showed that PET additive improved the cohesive energy density of the  
425 binder.
- 426 • The adhesion between the binder and aggregate was strengthened by PET additive in both  
427 dry and wet condition. PET additive tends to approach the binder-aggregate interface, and  
428 form strong hydrogen bonding, thus increasing the interfacial adhesion. The higher polarity  
429 of PET additive in comparison with bitumen further increased the adhesion with the polar  
430 aggregate.

431 Both the experimental results and MD simulation indicated that the PET additive can improve the  
432 adhesion between asphalt binder and aggregate, thus increasing the resistance to moisture-induced  
433 damage of asphalt mixture. It is therefore concluded that the PET additive obtained from the  
434 aminolysis treated waste PET bottles can be successfully used as a sustainable antistripping agent  
435 in asphalt mixture. This technology transferred the waste PET into a value-added additive that is  
436 applicable in the pavement industry. In future studies, the effects of the PET derived antistripping  
437 additive on asphalt mixtures with different aggregates will be investigated, and the economic and

438 environmental performances of this technology will be evaluated through life cycle cost analysis  
439 and life cycle assessment, respectively.

#### 440 **Notes**

441 The authors declare no competing financial interest.

#### 442 **Author Contributions**

443 **Rui Li:** Conceptualization, Methodology, Experiments, Test result analysis, Manuscript drafting,  
444 Manuscript revision and editing. **Zhen Leng:** Conceptualization, Methodology, Test result  
445 analysis, Manuscript revision and editing. **Jun Yang:** Test result analysis, Manuscript revision and  
446 editing. **Guoyang Lu:** Test result analysis, Manuscript revision and editing. **Man Huang:** Test  
447 result analysis, Manuscript revision and editing. **Jingting Lan:** Experiments, Test result analysis.  
448 **Hongliang Zhang:** Test result analysis. **Yawei Bai:** Test result analysis, Manuscript revision.  
449 **Zejiao Dong:** Test result analysis, Manuscript revision.

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457 in Switzerland.

#### 458 **Appendix**

459 The COMPASS force field, developed using *ab initio* and empirical parameterization techniques  
 460 by Sun [37], can well describe the interactions of both organic and inorganic materials. Previous  
 461 studies [38, 39] have successfully applied the COMPASS force field to simulate the bitumen and  
 462 aggregate. The total potential energy of COMPASS force field constitutes the valence energy ( $E_{val}$ ,  
 463 *bonded*) and non-bonded energy ( $E_{non-bond}$ ),

$$464 \quad E_{total} = E_{val} + E_{non-bond}$$

465 The valence energy is composed by,

$$466 \quad E_{val} = E_b + E_\theta + E_\varphi + E_\chi + E_{bb'} + E_{b\theta} + E_{b\varphi} + E_{\theta\varphi} + E_{\theta\theta'} + E_{\theta\theta'\varphi}$$

467 in which  $E_b$ ,  $E_\theta$ ,  $E_\varphi$ , and  $E_\chi$  represent the bond stretching, angle bending, internal torsion, and out-  
 468 of-plane bending terms, respectively, and  $E_{bb'}$ ,  $E_{b\theta}$ ,  $E_{b\varphi}$ ,  $E_{\theta\varphi}$ ,  $E_{\theta\theta'}$ ,  $E_{\theta\theta'\varphi}$  stand for cross-coupling  
 469 terms between two or three internal coordinates.

470 More detailly, these valence terms are described as follow,

$$471 \quad E_b = \sum_b [k_2(b-b_0)^2 + k_3(b-b_0)^3 + k_4(b-b_0)^4]$$

$$472 \quad E_\theta = \sum_\theta [k_2(\theta-\theta_0)^2 + k_3(\theta-\theta_0)^3 + k_4(\theta-\theta_0)^4]$$

$$473 \quad E_\varphi = \sum_\varphi [k_1(1-\cos\varphi) + k_2(1-\cos 2\varphi) + k_3(1-\cos 3\varphi)]$$

$$474 \quad E_\chi = \sum_\chi k_2\chi^2$$

$$475 \quad E_{bb'} = \sum_{b,b'} k(b-b_0)(b'-b'_0)$$

$$476 \quad E_{b\theta} = \sum_{b,\theta} k(b-b_0)(\theta-\theta_0)$$

$$477 \quad E_{b\varphi} = \sum_{b,\varphi} (b-b_0)(k_1 \cos\varphi + k_2 \cos 2\varphi + k_3 \cos 3\varphi)$$

478 
$$E_{\theta\varphi} = \sum_{\theta,\varphi} (\theta - \theta_0)(k_1 \cos \varphi + k_2 \cos 2\varphi + k_3 \cos 3\varphi)$$

479 
$$E_{\theta\theta'} = \sum_{\theta,\theta'} k(\theta - \theta_0)(\theta' - \theta'_0)$$

480 
$$E_{\theta\theta'\varphi} = \sum_{\theta,\theta',\varphi} k(\theta - \theta_0)(\theta' - \theta'_0) \cos \varphi$$

481 The non-bonded interaction includes the Coulomb electrostatic energy ( $E_{elec}$ ) and van der Waals

482 ( $E_{vdW}$ ) energy,

483 
$$E_{non-bond} = E_{elec} + E_{vdW}$$

484 and they are described as,

485 
$$E_{elec} = \sum_{i,j} \frac{q_i q_j}{r_{ij}}$$

486 
$$E_{vdW} = \sum_{i,j} \varepsilon_{ij} \left[ 2 \left( \frac{r_{ij}^0}{r_{ij}} \right)^9 - 3 \left( \frac{r_{ij}^0}{r_{ij}} \right)^6 \right]$$

487 The simulation details are presented in Table a1.

488 **Table a1 Simulation details**

Bulk bitumen binder model							
Procedure	Timestep (fs)	Duration (ps)	Ensemble	Temperature (K)	Pressure (GPa)	Thermostat	Barostat
Anneal	1	5	NVT	300-500 (5 cycles)	/	NHL	/
Pre-equilibrium	1	100	NVT	298	/	NHL	/
Equilibrium	1	200	NPT	298	0.0001	NHL	Andersen
Final equilibrium	1	100	NVT	298	/	NHL	/
Bitumen-aggregate interface model							
Equilibrium	1	200	NVT	298	/	NHL	/

489 Note: NHL represents the Nose-Hoover-Langevin thermostat.

490

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