

Material properties and residual stresses of cold-formed octagonal hollow sections

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Abstract: This paper presents an experimental investigation on the material properties and residual stress distributions in cold-formed steel octagonal hollow sections. Octagonal hollow sections (OctHSs) manufactured through two fabrication routes were considered. Two batches of steel plates with measured yield strengths of 546.5 MPa and 580.7 MPa were employed. A total of 46 tensile coupons taken from the parent plates and cold-formed hollow tubes were tested to obtain the material properties of parent metals and to assess strength variations within the sections caused by press-braking and welding process. A new bilinear plus nonlinear strain-hardening material model was proposed for the materials at flat regions and an existing material model for cold-formed steel was adopted for the materials at corners. These models yielded satisfactory agreements with the test results in terms of stress-strain relationships. Up to date, investigations on the residual stress distributions in cold-formed OctHSs remain limited. Subsequently, residual stress measurements on 4 cold-formed octagonal hollow sections with different fabrication routes and different flat side width-to-thickness ratios were performed. Sectioning method was adopted in this study with 74 strips extracted and more than 800 strain readings taken. Results of residual stress magnitudes and distributions are presented and discussed. Membrane residual stresses were typically found to be as high as 62% of the material yield strength, whilst bending residual stresses were found to be below 39% of the material yield strength. Simplified models for the residual stress magnitudes and distributions in the cold-formed octagonal hollow sections were proposed on the basis of test results.

Keywords: Material properties; Residual stresses; Cold-formed; Octagonal hollow sections.

1. Introduction

Tubular structural members have been widely adopted in civil structural applications owing to their structural efficiency and aesthetic appearance [1]. Numerous experimental and numerical investigations on tubular members with circular hollow sections (CHSs), elliptical hollow sections (EHSs), square hollow sections (SHSs) and rectangular hollow sections (RHSs) in both normal strength steel and high strength steel have been undertaken [2-6]. Design rules for steel hollow sections have been published worldwide such as EN 1993-1-1 [7], AISC 360-16 [8], and AISI S100-16 [9]. In recent years, steel polygonal hollow sections, in particular octagonal hollow

41 sections (OctHSs), have attracted significant attention from architects, structural engineers and
42 researchers [10-17], and have been utilised in civil applications such as transmission poles,
43 antennas and lighting structures [12, 17]. Example of an OctHS lighting column is depicted in Fig.
44 1. An obvious attraction of OctHSs is that the flat side width of the plate element is smaller than
45 square counterparts with the same perimeter. OctHSs therefore exhibit stronger local buckling
46 resistance than that of SHSs/RHSs [16-17]. In addition, OctHSs provide flat surfaces for easier
47 beam-to-column connection constructions as compared with CHSs [16].

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49 For cold-formed hollow sections, unlike plate products and built-up sections, the cold-forming
50 processes such as cold-rolling and press-braking alter the material properties within the hollow
51 section when compared to parent metal [18]. Hence, the material properties can differ from part to
52 part within the hollow section. One important reason causing the variation of the material
53 properties is the plastic deformation at the cold-formed regions which leads to excessive plastic
54 strain, resulting in strain-hardening [16, 18]. Moreover, welding process introduces heat energy to
55 the weld metal which also influences the material properties around the welding seam and causes
56 heat affected zone (HAZ). Material properties within HAZ can be different from those of the
57 materials outside HAZ [19, 20]. The non-uniform thermal and plastic strains also result in residual
58 stresses within the hollow section. Residual stresses within the hollow section affect the structural
59 behaviours of the members by causing premature yielding through part of the material thickness,
60 which may further lead to instability in compression members [21, 22]. Without good
61 understanding of the variation of the material properties and the residual stresses within hollow
62 sections, inaccurate predictions of the structural performance might be obtained. To date,
63 tremendous amount of investigations, both experimentally and numerically, have been carried out
64 regarding OctHSs on material properties, residual stresses, and stabilities [10-17]. However, for
65 cold-formed OctHSs, investigations on material properties at corner regions and measurements on
66 residual stresses are rather limited. Fang et al. [16] experimentally investigated the variation of
67 material properties and residual stresses of OctHSs manufactured from S690 steel plates. Three
68 different fabrication routes were used. Zhu et al. [17] carried out experimental tests on the material
69 properties, residual stresses and stub column behaviour of OctHSs focusing on S355 steel plates.
70 In [17], only one fabrication route was considered. Studies incorporating 460 steel are scarce,
71 potentially inhibiting attempts to develop effective design recommendations for OctHSs.
72 Therefore, for the purpose of developing design guidelines for OctHSs, it is of vital significance
73 to thoroughly investigate the material properties and residual stresses of cold-formed OctHSs using
74 460 steel plates.

75
76 The material properties and residual stresses of cold-formed OctHSs manufactured from 460 steel
77 plates were investigated herein. A total of 46 coupon tests consisting of flat coupons, corner
78 coupons and weld coupons were conducted. Due to cold-forming process, strength enhancement
79 was observed at corner regions. Essentially unchanged material properties were observed in the

80 flat regions when compared to parent metals. Existing material models for hot-rolled steel and
81 cold-formed steel were discussed and parameters for predicting stress-strain curves at flat and
82 corner portions were given. Residual stress measurements on four OctHS specimens were carried
83 out. Distributions of the residual stress were presented and discussed. Simplified predictive models
84 for the residual stress distributions were then proposed.

85

86 **2. Material property investigations**

87

88 **2.1 General**

89

90 In order to investigate the effect of fabrication process on the material properties of cold-formed
91 octagonal hollow sections (OctHSs), tensile coupons were extracted from parent plates, flat and
92 corner portions of hollow tubes. Two batches of steel plates with nominal yield strength of 460
93 MPa and with nominal thicknesses of 3 mm and 6 mm were employed in the study. To form an
94 OctHS, normally there are three fabrication routes: (1) welding two cold-formed half-sections at
95 two corners, (2) welding two cold-formed half-sections at two flat surfaces and (3) welding eight
96 flat plates, as discussed in [16]. As this study focuses on cold-formed sections, fabrication routes
97 (1) and (2) were adopted, denoted as CF1 and CF2 respectively as shown in Fig. 2. The former has
98 three cold-bent corners in one cold-formed half-section, while the latter has four cold-bent corners.
99 The difference between these two fabrication routes was the location of the welding seam.

100

101 **2.2 Tensile coupon tests**

102

103 To obtain the material properties, some 46 tensile coupon tests were conducted: 6 flat coupons
104 taken from parent plates (3 for each plate), 28 flat coupons extracted from flat portions of cold-
105 formed hollow tubes, 8 corner coupons removed from the corner portions, and 4 coupons cut from
106 welding seams. All the coupon tests were carried out in accordance with EN ISO 6892-1:2016 [23]
107 using an in-house Instron 8803 fatigue testing system with a capacity of 500 kN. Electrical strain
108 gauges were affixed at the mid-length of both sides of the tensile coupons. In addition, an
109 extensometer with the extensometer gauge length of either 25 mm or 50 mm was used to record
110 full engineering stress-strain responses. The extensometer elongation at fracture $\varepsilon_{f,25}$ or $\varepsilon_{f,50}$ was
111 directly captured by the 25 mm or 50 mm extensometer attached on the coupons. According to EN
112 ISO 6892-1:2016, proportional elongation at fracture ε_f is based on an original gauge length of
113 $5.65\sqrt{A_0}$, where A_0 is cross-sectional area within the original gauge length of the coupon. In this
114 study, if the original gauge length of the coupon is approximately equal to 25 mm or 50 mm
115 (extensometer gauge length), ε_f was directly obtained by the final strain reading of corresponding
116 extensometer; otherwise, it was determined by carefully fitting the fractured pieces and comparing
117 the proportional gauge length after fracture to the original proportional gauge length. For each
118 coupon, the original proportional gauge length was initially marked by fine lines. During each

119 tensile coupon test, the loading was paused twice near yield and ultimate strength for 120 s for
120 stress relaxation to obtain the static stress-strain curves [24]. The obtained static stress-strain
121 curves were then used to determine the static material properties, such as yield strength and
122 ultimate strength.

123

124 *2.2.1 Parent plate coupon tests*

125

126 Material properties of parent metals were first measured by flat tensile coupon tests. Three coupons
127 were taken from each batch of the parent plates. To measure the Poisson's ratio, four strain gauges
128 were mounted at the mid-length of each tensile coupon, with two mounted in longitudinal direction
129 and the other two in transverse direction. The test arrangement of flat tensile coupon tests is shown
130 in Fig. 3. The mean measured material properties of parent plates are summarised in Table 1, where
131 $E_{s,p}$ is elastic modulus of steel, ν is Poisson's ratio, $f_{y,p}$ is yield strength, $f_{u,p}$ is ultimate strength,
132 $\epsilon_{sh,p}$ is strain-hardening strain (the strain at which strain-hardening initiates), $\epsilon_{u,p}$ is strain at the
133 ultimate strength, $\epsilon_{f,25p}$ or $\epsilon_{f,50p}$ is elongation at fracture determined by a 25 mm or 50 mm
134 extensometer, and $\epsilon_{f,p}$ is proportional elongation at fracture. The letter "p" in the subscript of the
135 symbols indicates parent metal. Typical full stress-strain curves of the tensile coupons taken from
136 each parent metal are presented in Fig. 4.

137

138 *2.2.2 Quarter-section coupon tests*

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140 As discussed in Section 1, manufacturing process affects the material properties within the hollow
141 section. To examine the variation of the material properties, tensile coupons were extracted from
142 a quarter-section. This is based on an assumption that the effect of manufacturing process is doubly
143 symmetric due to geometric symmetry. A total of 4 cold-formed OctHSs, namely CF1-75×3, CF1-
144 75×6, CF2-75×3 and CF2-75×6, were chosen. The designation of the specimens firstly gives the
145 fabrication route which is followed by nominal cross-sectional size (flat side width by thickness).
146 The mean measured cross-sectional dimensions of the 4 sections are reported in Table 2. For each
147 cold-formed section, a total of 10 coupons were extracted from different locations within a quarter-
148 section. One coupon was cut from the welding seam to obtain the material properties of the weld
149 metal, one coupon was removed at the location 10 mm away from the centre line of the welding
150 seam to investigate the effect of welding process, and the remaining 8 coupons were taken
151 successively in the remaining portion to check the variation of material properties with the section.
152 The locations where the coupons were extracted are depicted in Fig. 5.

153

154 For all the quarter-section coupon tests, a 25 mm gauge length extensometer was used to capture
155 the full stress-strain responses and two strain gauges were attached at the mid-length on both sides
156 of the coupon to measure the elastic modulus of the coupon. All flat coupons had a width of 5 mm
157 along the 25 mm extensometer gauge length. The width of all the corner coupons along the parallel

158 length was 4 mm to minimise the load eccentricity introduced during the testing process. For the
159 coupons that the original gauge length is not approximately equal to 25 mm (extensometer gauge
160 length), a series of fine lines were marked on the coupon surface to capture the elongation at
161 fracture ε_f after the test. It was observed that the corner coupons curved after removal, which is
162 due to the release of the through-thickness residual stress. No attempt was made to straighten the
163 coupons prior to testing. Two 10 mm diameter holes were drilled on both ends of the corner
164 coupons and a pair of specially grips with two pins were used to avoid load eccentricity. Test
165 arrangements of flat and corner tensile coupons taken from hollow tubes are demonstrated in Fig.
166 6. The measured material properties of the 4 quarter-section coupons are reported in Tables 3-6,
167 in which E_s is elastic modulus, f_y is yield strength (taken as lower yield strength for steel exhibiting
168 yield plateau or 0.2% proof strength for steel with rounded stress-strain relationship), f_u is ultimate
169 strength, ε_u is strain at the ultimate strength, $\varepsilon_{f,25}$ is elongation at fracture determined by a 25 mm
170 extensometer, and ε_f is proportional elongation at fracture. Typical stress-strain curves of flat and
171 corner coupons extracted from a hollow tube are shown in Fig. 7. Tensile coupons taken from the
172 quarter-section of CF1-75×3 are shown in Fig 8. The distributions of measured yield strength and
173 ultimate strength in CF1-75×3, CF1-75×6, CF2-75×3 and CF2-75×6 are presented in Figs. 9-12
174 respectively.

175
176 Essentially similar stress-strain responses were observed for the flat and corner coupons of each
177 steel batch. The mean measured material properties of flat coupons are reported in Table 7, in
178 which $E_{s,f}$ is elastic modulus, $f_{y,f}$ is yield strength, $f_{u,f}$ is ultimate strength, $\varepsilon_{u,f}$ is strain at the ultimate
179 strength, $\varepsilon_{f,25f}$ is elongation at fracture determined by a 25 mm extensometer, and $\varepsilon_{f,f}$ is proportional
180 elongation at fracture. The letter “f” after comma in the subscript denotes flat coupons cut from
181 hollow tubes. Similarly, the mean measured material properties of corner coupons are listed in
182 Tables 8, where $E_{s,c}$ is elastic modulus, $f_{y,c}$ is yield strength (taken as 0.2% proof strength), $f_{u,c}$ is
183 ultimate strength, $\varepsilon_{u,c}$ is strain at ultimate strength, $\varepsilon_{f,25c}$ is elongation at fracture determined by the
184 25 mm extensometer, and $\varepsilon_{f,c}$ is proportional elongation at fracture. The letter “c” in the subscript
185 designates corner coupons extracted from hollow tubes. It was observed, by comparing the
186 material properties in Tables 7-8 to that of parent plates (Table 1), that the fabrication routes of
187 combined press braking and welding produces essentially unchanged properties in the flat regions,
188 but causes large strength enhancements at the corners which is the result of excessive strain-
189 hardening. It was noted that the strength enhancement is accompanied by a reduction of ductility.

190 191 **2.3 Strength enhancement at corner region**

192
193 The coupon test results reported in Section 2.2 reveal that the yield strength of the corner portions
194 is greatly enhanced due to the pronounced strain-hardening. The strength enhancement ratios,
195 which is defined as the corner yield strength over flat yield strength ratio $f_{y,c} / f_{y,f}$, of the 3 mm and
196 6 mm steel are 1.21 and 1.27, respectively. Karren [25] suggested that the influence of the strength

197 enhancement at cold-formed regions should be taken into account in calculations of cross-section
 198 resistance. A predictive model for the strength enhancement was then proposed for press-braked
 199 and cold-rolled sections by Karren [25], as given in Eq. (1). This model has been standardised in
 200 AISI S100-16 [9]. To further assess the applicability of this model to cold-formed OctHSs, a
 201 database composed of 26 corner coupon test results reported in [16, 17] and this study was
 202 developed. All the corner coupons were extracted from cold-formed OctHSs fabricated by a
 203 combination of press-braking and welding. Three steel grades, namely 355, 460 and 690, were
 204 covered with the yield strength ranging from 373 MPa to 810 MPa. This predictive model
 205 reasonably predicts the strength enhancement, yielding a mean prediction-to-test value of 1.03
 206 with a corresponding CoV (coefficient of Variance) of 0.04. The assessment of the predictive
 207 model is presented in Fig. 13.

$$\frac{f_{y,c}}{f_{y,p}} = \frac{B_c}{(r_i / t)^m} \quad (1)$$

209 where

$$B_c = 3.69 \frac{f_{u,p}}{f_{y,p}} - 0.819 \left(\frac{f_{u,p}}{f_{y,p}} \right)^2 - 1.79$$

$$m = 0.192 \frac{f_{u,p}}{f_{y,p}} - 0.068$$

210

211 **2.4 Material models**

212

213 With the advancements of finite element modelling (FEM) techniques, some commercial finite
 214 element packages such as ABAQUS [26] are capable of replicating steel behaviour under
 215 extremely large strain. Precise stress-strain models up to ultimate strength are of great importance
 216 for FEM to capture behaviours of steel members like beam. This section briefly reviews the
 217 existing material models for hot-rolled and cold-formed steels as they can be used to describe the
 218 material properties at flat and corner regions respectively. Parameters for obtaining stress-strain
 219 curves are then given.

220

221 *2.4.1 Hot-rolled material model*

222

223 The hot-rolled structural steel normally exists a linear elastic range and a sharply defined yield
 224 point followed by a pronounced plastic yield plateau. For a description of the stress-strain
 225 behaviour of hot-rolled structural steel, a three-stage nonlinear model (bilinear plus nonlinear
 226 hardening model) was proposed by Yun and Gardner [27] on a basis of a coupon test database
 227 comprising over 500 samples. This model is expressed as follows:

228

$$\sigma = \begin{cases} E_s \varepsilon & \text{for } \varepsilon \leq \varepsilon_y \\ f_y & \text{for } \varepsilon_y < \varepsilon \leq \varepsilon_{sh} \\ f_y + (f_u - f_y) \left[0.4 \varepsilon_x + 2 \varepsilon_x / (1 + 400 \varepsilon_x^5)^{1/5} \right] & \text{for } \varepsilon_{sh} < \varepsilon \leq \varepsilon_u \left(\varepsilon_x = \frac{\varepsilon - \varepsilon_{sh}}{\varepsilon_u - \varepsilon_{sh}} \right) \end{cases} \quad (2)$$

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where E_s is elastic modulus, f_y is yield strength, f_u is ultimate strength, ε_y is yield strain, ε_{sh} is strain-hardening strain, ε_u is ultimate strain at ultimate strength. If ε_{sh} and ε_u are not reported, they could be determined by empirical equations as expressed in Eqs. 3 and 4 [27].

$$\varepsilon_{sh} = 0.1 \frac{f_y}{f_u} - 0.055 \quad \text{but } 0.015 \leq \varepsilon_{sh} \leq 0.03 \quad (3)$$

$$\varepsilon_u = 0.6 \left(1 - \frac{f_y}{f_u} \right) \quad \text{but } \varepsilon_u \geq 0.06 \quad (4)$$

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It is noted that the shape of the stress-strain curve in the nonlinear strain-hardening range is similar to that of initial part of Ramberg-Osgood model up to the 0.2% proof strength, as shown in Fig. 14 and 15. Hence, it is proposed herein to use a modified Ramberg-Osgood model to describe the stress-strain behaviour of the nonlinear strain-hardening range. The modified Ramberg-Osgood model is obtained by a linear transformation of the stress and strain in the strain-hardening range from point (ε_{sh}, f_y) to the origin, as given in Eq. 5, in which $\varepsilon^* = \varepsilon - \varepsilon_{sh}$, $\sigma^* = \sigma - f_y$, $f_u^* = f_u - f_y$, $E^* = E_{sh}$, $\varepsilon_p^* = \varepsilon_u - \varepsilon_{sh} - (\sigma - f_y)/E_{sh}$ and m is the strain-hardening exponent. Commencing at the strain-hardening point, the modified Ramberg-Osgood model can be expressed by Eq. 6, in which E_{sh} is the initial slope of the stress-strain curves in the strain-hardening range, termed as hardening modulus. E_{sh} can be directly obtained from material tests. If the value is not reported, an equation (see Eq. 7) proposed in [27] might be used to determine the value.

$$\varepsilon^* = \frac{\sigma^*}{E^*} + \varepsilon_p^* \left(\frac{\sigma^*}{f_u^*} \right)^m \quad (5)$$

$$\varepsilon - \varepsilon_{sh} = \frac{\sigma - f_y}{E_{sh}} + \left(\varepsilon_u - \varepsilon_{sh} - \frac{f_u - f_y}{E_{sh}} \right) \left(\frac{\sigma - f_y}{f_u - f_y} \right)^m \quad (6)$$

$$E_{sh} = \frac{f_u - f_y}{0.4(\varepsilon_u - \varepsilon_{sh})} \quad (7)$$

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Hence a full range material model can be obtained by combining the bilinear model in Eq. 2 with the modified Ramberg-Osgood model for nonlinear strain-hardening range as given in Eq. 8.

$$(1) \text{ Bilinear: } \sigma = \begin{cases} E_s \varepsilon & \text{for } \varepsilon \leq \varepsilon_y \\ f_y & \text{for } \varepsilon_y < \varepsilon \leq \varepsilon_{sh} \end{cases} \quad (8)$$

$$(2) \text{ nonlinear: } \varepsilon = \varepsilon_{sh} + \frac{\sigma - f_y}{E_{sh}} + \left(\varepsilon_u - \varepsilon_{sh} - \frac{f_u - f_y}{E_{sh}} \right) \left(\frac{\sigma - f_y}{f_u - f_y} \right)^m \quad \text{for } \sigma > f_y$$

251
 252 To make direct comparisons, a constant m value of 6.0 was firstly used. Representative
 253 comparisons of the nonlinear hardening stress-strain curves between the proposed predictive
 254 model and test results reported in [16, 17] and this study are shown in Fig. 16. It can be seen from
 255 Fig. 16 (a) to (c) that the proposed model yields a better prediction of the material responses than
 256 the model proposed by Yun and Gardner [27]. For the S690 test in [16], the result of the proposed
 257 model is significantly improved by changing the m value from 6.0 to 3.0 (see Fig. 17). A possible
 258 reason for this phenomenon is that the exponent m is related to the yield strength and ultimate
 259 strength of the materials. An empirical equation to predict the value of m might be derived based
 260 on a large size of test database.

261 262 2.4.2 Cold-formed material model

263
 264 Unlike hot-rolled steel, cold-formed steel generally exhibits a rounded stress-strain relationship,
 265 which is similar in shape to stainless steel. In this case, 0.2% proof strength $f_{0.2}$ is frequently used
 266 as an equivalent yield strength. Adopting 0.2% proof strength and corresponding strain in the
 267 original form of Ramberg-Osgood expression, Eq. 9 can be obtained [28]. This equation performs
 268 well in predicting stress-strain curves up to 0.2% proof strength but tends to overestimate the
 269 material responses after the yield point [29].

$$\varepsilon = \frac{\sigma}{E_s} + 0.002 \left(\frac{\sigma}{f_{0.2}} \right)^n \quad (9)$$

271
 272 It is noted that the stress-strain curve in the range between 0.2% proof strength and ultimate
 273 strength is similar in shape to the initial range up to 0.2% proof strength [29]. A two-stage full
 274 range stress-strain curve was then proposed and could be written out as Eq. 10,

$$\varepsilon = \begin{cases} \frac{\sigma}{E_s} + 0.002 \left(\frac{\sigma}{f_{0.2}} \right)^n & \text{for } \sigma \leq f_{0.2} \\ \frac{\sigma - f_{0.2}}{E_{0.2}} + \left(\varepsilon_u - \varepsilon_{0.2} - \frac{f_u - f_{0.2}}{E_{0.2}} \right) \left(\frac{\sigma - f_{0.2}}{f_u - f_{0.2}} \right)^m + \varepsilon_{0.2} & \text{for } \sigma > f_{0.2} \end{cases} \quad (10)$$

276

277 where n and m are strain-hardening exponents in the first stage and second stage respectively and
 278 can be obtained by Eq. 11 and 12, $E_{0.2}$ is tangential modulus of elasticity at 0.2% proof strength
 279 and is given in Eq. 13, and $\varepsilon_{0.2}$ is total strain at 0.2% proof strength. In Eq. 11, $f_{0.01}$ is 0.01% proof
 280 strength.
 281

$$n = \frac{\ln(20)}{\ln(f_{0.2} / f_{0.01})} \quad (11)$$

$$m = 1 + 3.5 \frac{f_{0.2}}{f_u} \quad (12)$$

$$E_{0.2} = \frac{E_s}{1 + 0.002nE_s / f_{0.2}} \quad (13)$$

282
 283 It is found in [30] that using the 0.05% proof strength $f_{0.05}$ instead of 0.01% proof strength to
 284 determine the strain-hardening exponent n provides a better prediction of stainless steel stress–
 285 strain curves. Hence Eq. 14 could be used to replace Eq. 11.
 286

$$n = \frac{\ln(4)}{\ln(f_{0.2} / f_{0.05})} \quad (14)$$

287
 288 It was found by Ma et al. [22] that the strain-hardening exponent n increased with the increase in
 289 plastic strain ε_p . A new model, in which the strain-hardening exponent n_{pro} is formulated as a
 290 function of plastic strain, was proposed, as expressed in Eq. 15,

$$n_{\text{pro}} = f(\varepsilon_p) = n + K\varepsilon_p^m \quad (15)$$

291
 292 where n is the original strain-hardening exponent, m and K are the exponent and coefficient for
 293 calculating n_{pro} , ε_p is plastic strain. The coefficient K is determined by modulus of elasticity, 0.2%
 294 proof strength, ultimate strength and strain at ultimate strength by Eq. 16. The exponent m is
 295 obtained by try and error analysis fitting the predicted curve to the test results. The plastic strain
 296 is equal to total strain minus elastic strain. Hence, Eq. 17 is obtained.
 297

$$K = \left[\log_{f_u / f_{0.2}} \left(\frac{\varepsilon_u - f_u / E_s}{0.002} \right) \right] / (\varepsilon_u - f_u / E_s)^m \quad (16)$$

$$\varepsilon_p = \varepsilon - \frac{\sigma}{E} = 0.002 \left(\frac{\sigma}{f_{0.2}} \right)^{n_{\text{pro}}} \quad (17)$$

298
 299 Substituting Eq. 15 into Eq. 17, the explicit form of the proposed stress-strain model is given in
 300 Eq. 18. The stress-strain curve can be determined when the material parameters, namely elasticity

301 modulus, 0.01% proof strength (or 0.05% proof strength), 0.2% proof strength, ultimate strength,
 302 strain at ultimate strength and exponent m , are obtained.
 303

$$\left\{ \begin{array}{l} \sigma = \left(\frac{\varepsilon_p}{0.002} \right)^{\left(\frac{1}{n+K\varepsilon_p^m} \right)} f_{0.2} \\ \varepsilon = \varepsilon_p + \frac{\sigma}{E_s} = \varepsilon_p + \frac{f_{0.2}}{E_s} \left(\frac{\varepsilon_p}{0.002} \right)^{\left(\frac{1}{n+K\varepsilon_p^m} \right)} \end{array} \right. \quad (18)$$

304
 305 The material parameters for corner coupons are summarised in Table 9. The comparisons of the
 306 stress-strain curves between the model predictions and test results are depicted in Fig. 18. The
 307 results of the comparisons indicate that the model proposed in [22] correlated well with test data.
 308

309 **3. Residual stress measurements**

310
 311 Residual stresses, which are introduced by manufacturing process, exist in both longitudinal and
 312 transverse directions of cold-formed steel members. The longitudinal residual stresses are more
 313 influential on the structural responses than that in transverse direction [16, 21]; hence, the focus of
 314 this study was put on the longitudinal residual stresses which has membrane and bending residual
 315 stress components (as shown in Fig. 19). There are various residual stress measurement techniques
 316 which are classified as destructive and non-destructive methods, such as neutron or electron
 317 diffraction, ultrasonic method and sectioning method. A comprehensive comparison of the
 318 different techniques is detailed in [31]. Sectioning method is a commonly used destructive method
 319 which determines residual stresses by measuring the deformation of strips extracted from cold-
 320 formed members. It has been extensively used to obtained residual stress distributions in structural
 321 steel [16, 17, 22] and stainless steel [21] sections. In this study, sectioning method was used.
 322

323 **3.1 Specimens**

324
 325 Sectioning was performed on four cold-formed OctHSs to quantify the longitudinal residual
 326 stresses namely membrane and bending residual stresses. Two fabrication routes, CF1 and CF2 as
 327 shown in Fig. 2, were adopted and two cross-sectional dimensions with nominal flat side width by
 328 thickness of 75×6 and 105×3 were chosen. The four specimens were labelled as CF1-75×6, CF1-
 329 105×3, CF2-75×6, and CF2-105×3. All the specimens prepared for measurements were 300 mm
 330 in length. A quarter of the sections was marked into longitudinal strips because of the symmetrical
 331 distribution of residual stresses. The widths of strips in the 75×6 and 105×3 sections were 10 mm
 332 and 12 mm, respectively. Strain gauges with a 5 mm gauge length were mounted onto both external
 333 and internal surfaces of the marked longitudinal strips at the mid-length of each strip. Each strain

334 gauge was carefully coated by water-proof tape to avoid any damage in the strain gauge during the
 335 sectioning process. The external and internal views of specimen CF2-75×6 are presented in Fig.
 336 20.

337
 338 Initial readings of the strain gauges on the external and internal surfaces were recorded (denoted
 339 as $\varepsilon_{\text{ext},i}$ and $\varepsilon_{\text{int},i}$, respectively) prior to the conduction of sectioning process. After the completion
 340 of the sectioning process, the residual stresses introduced by the manufacturing process within the
 341 cross-section were released. The readings of each strain gauge were taken again (indicated as $\varepsilon_{\text{ext},f}$
 342 and $\varepsilon_{\text{int},f}$, respectively) for all the cut strips. For each strain gauge, three readings were taken, and
 343 the mean value was used for subsequent analyses. The residual strains at the external ε_{ext} and
 344 internal ε_{int} surfaces can be calculated by subtracting the initial readings from the final readings.
 345 The membrane and bending residual strains are subsequently determined by Eqs. 19 and 20. The
 346 residual stresses are then obtained by Hook's Law ($\sigma = E\varepsilon$). It should be noted that positive and
 347 negative values indicate tensile and compression residual stresses respectively.

348

$$\varepsilon_m = -\left(\frac{\varepsilon_{\text{ext}} + \varepsilon_{\text{int}}}{2}\right) = -\left(\frac{\varepsilon_{\text{ext},f} - \varepsilon_{\text{ext},i}}{2} + \frac{\varepsilon_{\text{int},f} - \varepsilon_{\text{int},i}}{2}\right) \quad (19)$$

$$\varepsilon_{b,\text{ext}} = -\varepsilon_{b,\text{int}} = -\left(\frac{\varepsilon_{\text{ext}} - \varepsilon_{\text{int}}}{2}\right) = -\left(\frac{\varepsilon_{\text{ext},f} - \varepsilon_{\text{ext},i}}{2} - \frac{\varepsilon_{\text{int},f} - \varepsilon_{\text{int},i}}{2}\right) \quad (20)$$

349
 350 **3.2 Results**

351
 352 The magnitudes and distributions of longitudinal residual stress in CF1-75×6, CF1-105×3, CF2-
 353 75×6, and CF2-105×3 were determined. Examples of deformed strips extracted from CF1-105×3
 354 after sectioning are depicted in Fig. 21. The measured residual stresses were firstly normalised by
 355 the yield strength of parent metals. Then, the magnitudes and distributions of the normalised
 356 residual stresses including membrane residual stresses and bending residual stresses on the
 357 external surfaces are plotted with respect to the distance from welding seam, as demonstrated in
 358 Fig. 22. It was noted that OctHSs fabricated from different routes exhibited different distributions
 359 and magnitudes of residual stresses, indicating that the fabrication routes should be accounted for
 360 in the analysis. For the press-braked sections, the corner strips have larger curvatures than the
 361 others because of large bending residual stresses. This is because that the corner materials
 362 experienced pronounced plastic deformations during the fabrication process. The bending residual
 363 stresses at corners were typically found to be below $0.39f_{y,p}$ tensile stress on the external surface.
 364 The magnitudes of the bending residual stresses at the other locations were much lower than those
 365 of corners. In terms of maximum membrane residual stresses, the normalised values were found
 366 to be as $0.62f_{y,p}$ tensile stress, which were much higher than the bending residual stress amplitude.

367 The magnitudes of membrane residual stresses near the welding seam were greatly larger than
368 other locations.

369

370 *3.3 Simplified predictive models*

371

372 The test results of the magnitudes and distributions of the membrane and bending residual stresses
373 in cold-formed OctHSs were subsequently utilised to develop simplified predictive models. These
374 models might be incorporated in structural analyses such as finite element modelling (FEM) to
375 obtain better predictions in member behaviours. For easy application purpose, the distribution
376 patterns were simplified as multi-linear curves with constant values at corner regions. Magnitudes
377 of the residual stresses in the simplified models were obtained as the mean measured stresses at
378 specific parts. Within the scope of this study, simplified predictive models for the longitudinal
379 membrane and bending residual stress distributions in CF1 and CF2 sections (as shown in Fig. 2)
380 were proposed. The distributions of the residual stresses are plotted over quarter-sections, as shown
381 in Fig. 23.

382

383 The test results of residual stresses in cold-formed OctHS (CF2) manufactured from S355 steel in
384 [17] is presented in Fig. 24(a). The yield strength of the S355 steel is 379 MPa. Simplified
385 predictive models were derived based on the test results and the models are depicted in Fig. 24(b).
386 Fang et al. [16] undertook residual stress measurements on OctHSs using S690 steel. Predictive
387 models were also proposed. Fig. 25 shows the predictive models for CF2 sections. It can be
388 observed from Figs. 23-25 that the distributions of the residual stresses in the proposed models are
389 similar in shape, though slight difference exists in the distribution of membrane residual stress near
390 the welding seam. It was noted that the magnitudes of residual stresses at specific parts are different.
391 The residual stresses are influenced by fabrication routes and are also dependent on the steel yield
392 strength. More test results regarding the residual stresses in OctHSs are needed for future research
393 to develop generalised predictive models.

394

395 **4. Conclusions**

396

397 The material properties and residual stress distributions in cold-formed octagonal hollow sections
398 (OctHSs) have been investigated in this study. Two fabrication routes involving combined press-
399 braking and welding were considered. A total of 46 tensile coupons extracted from parent plates
400 and cold-formed OctHSs were tested. Strength enhancement up to 27% was observed at corners
401 of cold-formed sections due to the cold-forming effect, while materials at flat portions exhibit
402 similar responses to parent metals. The applicability of a predictive model for the strength
403 enhancement in yield strength was assessed on a basis of 26 corner coupons extracted from OctHSs.
404 Predictions from the model match well with test results yielding a mean prediction-to-test value of
405 1.03 with a corresponding CoV of 0.04. In terms of stress-strain curves, a new bilinear plus

406 nonlinear strain-hardening constitutive model was proposed for materials at flat portions, which
407 can give satisfactory prediction in the strain-hardening range. As for cold-formed steel, an existing
408 model was adopted to describe the stress-strain relationships at corners, which shows good
409 agreements with test results. The magnitudes and distributions of longitudinal residual stresses in
410 cold-formed OctHSs from different fabrication routes were also experimentally investigated. The
411 maximum membrane and bending residual stresses were found to be up to 62% and 39% of the
412 material yield strength. In terms of bending residual stresses, magnitudes in the flat regions were
413 relatively low while magnitudes at the corners were much higher due to the cold-formed process.
414 As for the membrane residual stresses, magnitudes near the welding seam were greatly larger than
415 other locations. It was also noted that OctHSs fabricated from different routes exhibited different
416 distributions and magnitudes of residual stresses. Simplified predictive models for the distributions
417 of residual stresses in cold-formed OctHSs fabricated through different routes were proposed as
418 well based on the test results.

419

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421

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429

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- 498

Table 1. Test results of flat tensile coupons taken from parent plates.

Steel	$E_{s,p}$ (GPa)	ν	$f_{y,p}$ (MPa)	$f_{u,p}$ (MPa)	$\epsilon_{sh,p}$ (%)	$\epsilon_{u,p}$ (%)	$\epsilon_{f,25p}$ OR $\epsilon_{f,50p}$ (%)	$\epsilon_{f,p}$ (%)
3 mm	209.5	0.28	546.5	625.8	2.2	10.9	27.1	26.0
6 mm	213.3	0.28	580.7	666.1	2.3	10.1	25.4	25.4

Table 2. Mean measured cross-sectional dimensions.

Specimen	Width, H (mm)	Side, B (mm)	Thickness, t (mm)	Outer radius, r_o (mm)	Inner radius, r_i (mm)	Side width, b (mm)	b/t
CF1-75×3	184.8	76.6	3.04	10.0	7.0	68.4	22.5
CF1-75×6	184.4	76.4	3.05	10.0	7.5	68.1	22.3
CF2-75×3	183.5	76.0	5.83	19.0	13.5	60.0	10.3
CF2-75×6	184.7	76.5	5.83	18.5	12.5	61.2	10.5

Table 3. Quarter-section tensile coupon test results of section CF1-75×3.

Coupon	E_s (GPa)	f_y (MPa)	f_u (MPa)	ϵ_u (%)	$\epsilon_{f,25}$ (%)	ϵ_f (%)
W1	205.7	442.8	518.1	6.54	15.5	18.6
F2	211.5	549.8	620.0	8.97	23.6	25.5
F3	210.3	548.5	614.1	9.51	26.1	27.3
F4	208.4	543.0	618.6	10.84	26.5	27.0
F5	208.0	544.3	626.6	10.86	27.6	28.3
C6	200.6	661.1	696.2	1.26	10.5	16.3
F7	207.7	546.2	630.1	11.56	27.8	29.5
F8	209.1	545.7	624.5	11.03	27.2	29.0
F9	209.6	541.5	628.7	11.32	26.4	28.9
C10	202.8	661.4	690.0	1.07	- ^a	15.8

Note: a-For coupon C10 of CF1-75×3, fracture occurred outside the extensometer gauge length thus $\epsilon_{f,25}$ was not obtained.

Table 4. Quarter-section tensile coupon test results of section CF1-75×6.

Coupon	E_s (GPa)	f_y (MPa)	f_u (MPa)	ϵ_u (%)	$\epsilon_{f,25}$ (%)	ϵ_f (%)
W1	209.4	448.2	538.9	13.95	- ^a	26.9
F2	213.0	589.7	676.9	11.40	- ^a	29.2
F3	215.0	582.7	670.5	10.75	32.8	28.3
F4	213.5	580.0	666.0	10.74	30.5	27.7
F5	213.8	589.8	672.8	11.82	30.1	27.6
C6	200.5	730.6	776.3	1.44	14.3	14.1
F7	215.6	577.5	672.2	14.11	32.3	28.9
F8	214.3	577.3	671.1	11.10	30.2	28.8
F9	214.1	576.3	672.1	11.80	31.1	29.1

C10 198.9 732.8 770.2 1.26 13.3 14.6

Note: a-For coupons W1 and F2 of CF1-75×6, fracture occurred outside the extensometer gauge length thus $\epsilon_{f,25}$ was not obtained.

Table 5. Quarter-section tensile coupon test results of section CF2-75×3.

Coupon	E_s (GPa)	f_y (MPa)	f_u (MPa)	ϵ_u (%)	$\epsilon_{f,25}$ (%)	ϵ_f (%)
W1	206.2	453.1	532.7	7.91	16.8	23.2
F2	209.2	524.0	596.6	10.43	25.6	27.5
F3	212.7	541.6	611.3	9.75	24.1	25.6
C4	199.0	653.9	689.1	1.36	11.2	15.6
F5	211.8	538.8	621.8	10.96	26.7	28.4
F6	209.7	534.9	626.0	10.56	25.0	27.7
F7	209.2	539.6	634.3	11.90	27.8	28.8
C8	202.0	644.1	684.2	1.28	- ^a	15.8
F9	209.6	532.1	627.9	11.34	24.2	27.4
F10	209.7	541.4	636.9	11.87	25.2	28.0

Note: a-For coupon C8 of CF2-75×3, fracture occurred outside the extensometer gauge length thus $\epsilon_{f,25}$ was not obtained.

Table 6. Quarter-section tensile coupon test results of section CF2-75×6.

Coupon	E_s (GPa)	f_y (MPa)	f_u (MPa)	ϵ_u (%)	$\epsilon_{f,25}$ (%)	ϵ_f (%)
W1	207.7	459.8	541.7	9.15	- ^a	24.4
F2	210.4	568.9	647.1	11.40	22.1	27.1
F3	215.4	581.7	671.9	13.74	33.1	29.3
C4	196.6	737.7	778.3	1.32	14.7	14.7
F5	215.7	586.8	666.9	11.42	31.3	27.8
F6	214.9	580.3	669.2	11.74	30.8	27.6
F7	214.1	580.1	669.4	10.91	31.0	27.7
C8	196.6	738.9	779.0	1.32	14.5	14.1
F9	212.2	580.1	669.9	11.50	30.2	27.4
F10	212.8	582.6	671.9	10.95	- ^a	27.5

Note: a-For coupons W1 and F10 of CF2-75×6, fractures occurred outside the extensometer gauge length thus $\epsilon_{f,25}$ was not obtained.

Table 7. Mean material properties for flat coupons taken from OctHSs.

Steel	$E_{s,f}$ (GPa)	$f_{y,f}$ (MPa)	$f_{u,f}$ (MPa)	$\epsilon_{u,f}$ (%)	$\epsilon_{f,25f}$ (%)	$\epsilon_{f,f}$ (%)
3 mm	209.8	540.8	622.7	10.8	26.2	27.8
6 mm	213.9	581.0	669.1	11.7	30.5	28.1

Table 8. Mean material properties for corner coupons taken from OctHSs.

Steel	$E_{s,c}$ (GPa)	$f_{y,c}$ (MPa)	$f_{u,c}$ (MPa)	$\epsilon_{u,c}$ (%)	$\epsilon_{25,c}$ (%)	$\epsilon_{f,c}$ (%)
3 mm	201.1	655.1	689.9	1.24	10.8	15.9
6 mm	198.2	735.0	775.9	1.33	14.2	14.4

Table 9. Corner coupon material properties for material models.

Steel	$E_{s,c}$ (GPa)	$f_{0.01}$ (MPa)	$f_{0.2}$ (MPa)	$f_{u,c}$ (MPa)	$\epsilon_{u,c}$ (%)	Ma et al. [22]		Eq. (10)	
						n	m	n	m
3 mm	201.1	445	655.1	689.9	1.24	7.75	0.75	7.75	4.3
6 mm	198.2	499	735.0	775.9	1.33	7.74	0.80	7.74	4.3



Fig. 1. Octagonal section lighting column (West Lafayette, US).

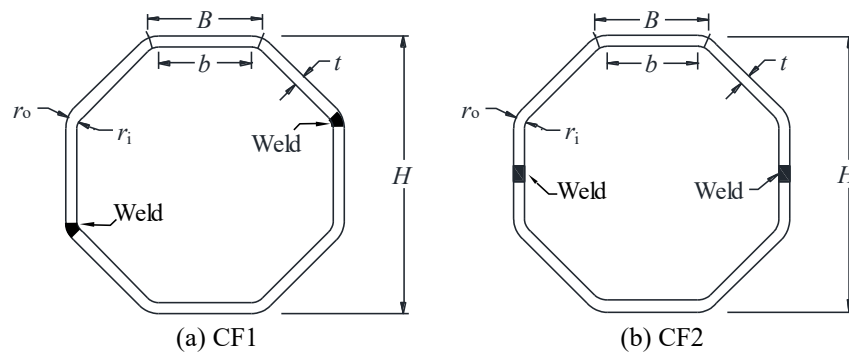


Fig. 2. Fabrication routes for octagonal steel hollow section.

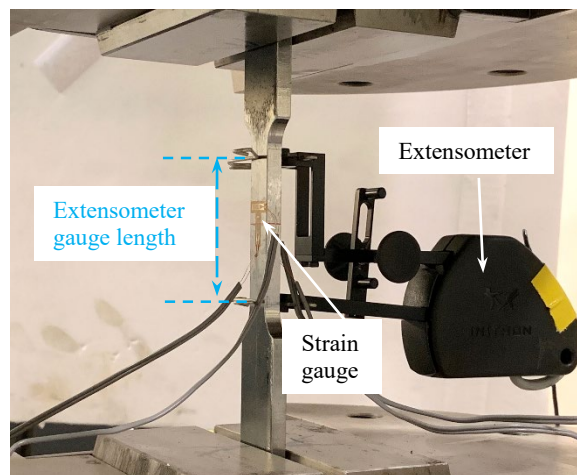
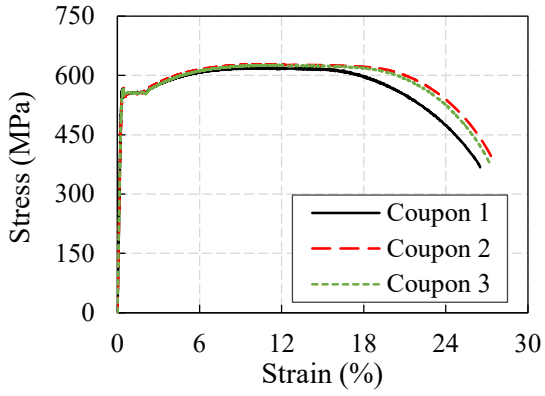
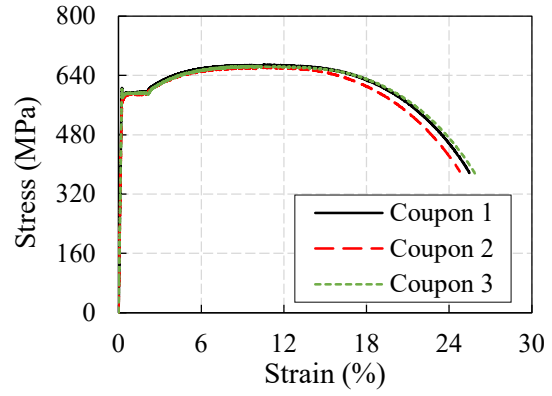


Fig. 3. Test arrangement of parent plate coupon.



(a) 3 mm steel plate



(b) 6 mm steel plate

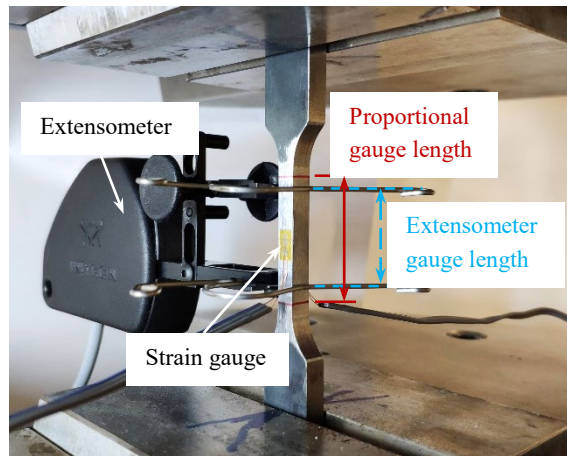
Fig. 4. Stress-strain curves of the tensile coupons taken from parent plates.



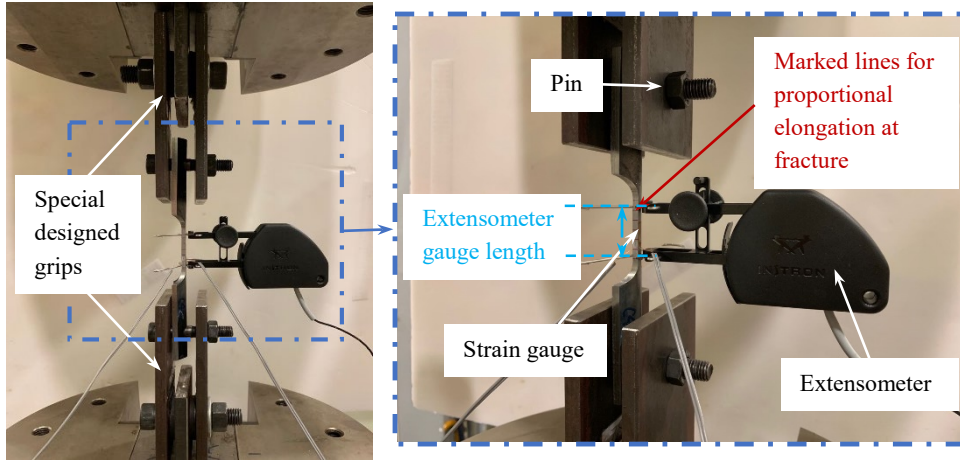
(a) CF1

(b) CF2

Fig. 5. Locations of the tensile coupons taken from hollow tubes.



(a) Flat coupon tests.



(b) Corner coupon tests.

Fig. 6. Test arrangement of flat and corner coupons extracted from quarter-section.

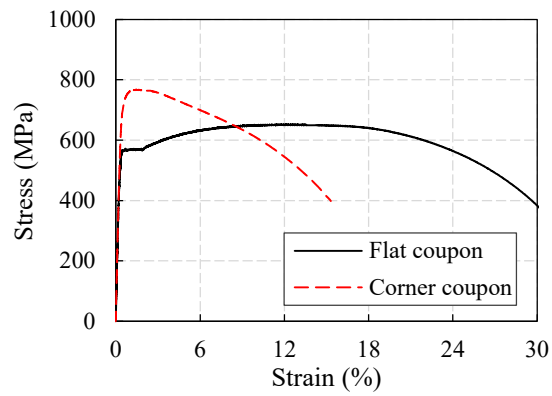


Fig. 7. Typical stress-strain curves of flat and corner coupons.



(a) Before test.

(b) Failed specimens.

Fig. 8. Tensile coupons extracted from a quarter-section of CF1-75x3.

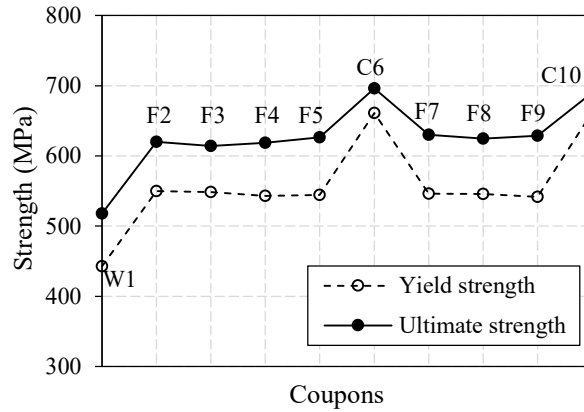


Fig. 9. Measured yield strength and ultimate strength distribution in CF1-75x3.

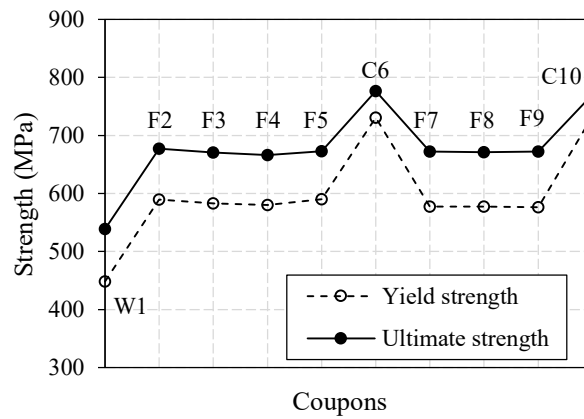


Fig. 10. Measured yield strength and ultimate strength distribution in CF1-75x6.

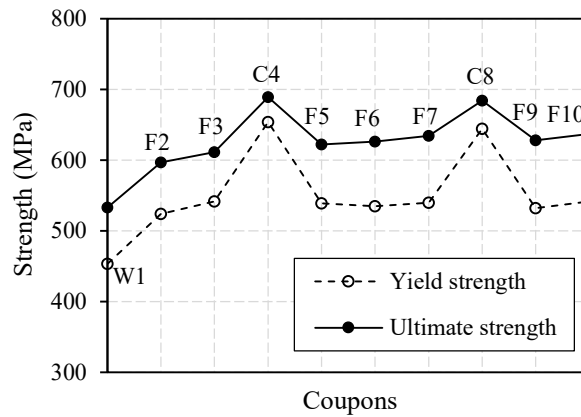


Fig. 11. Measured yield strength and ultimate strength distribution in CF2-75x3.

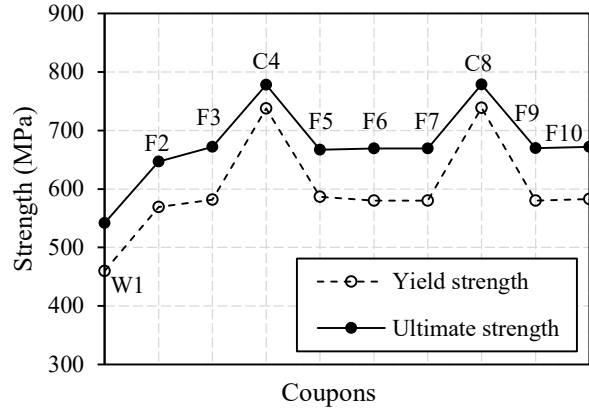


Fig. 12. Measured yield strength and ultimate strength distribution in CF2-75x6.

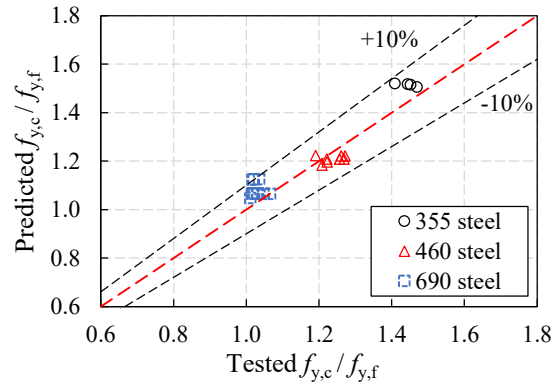


Fig. 13. Assessment of the strength enhancement predictive model.

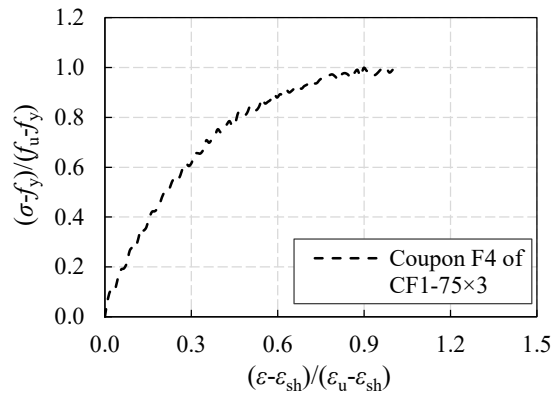


Fig. 14. Normalised nonlinear strain-hardening range of a flat coupon.

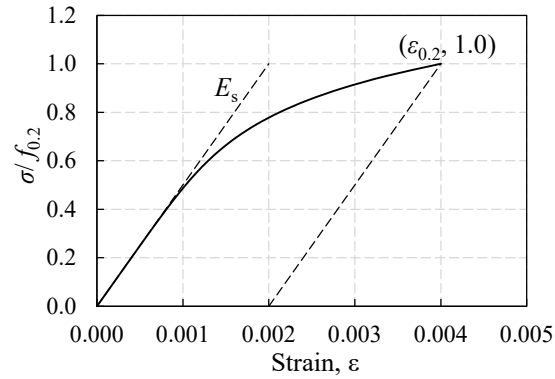


Fig. 15. Initial stress-strain curve of Ramberg-Osgood model.

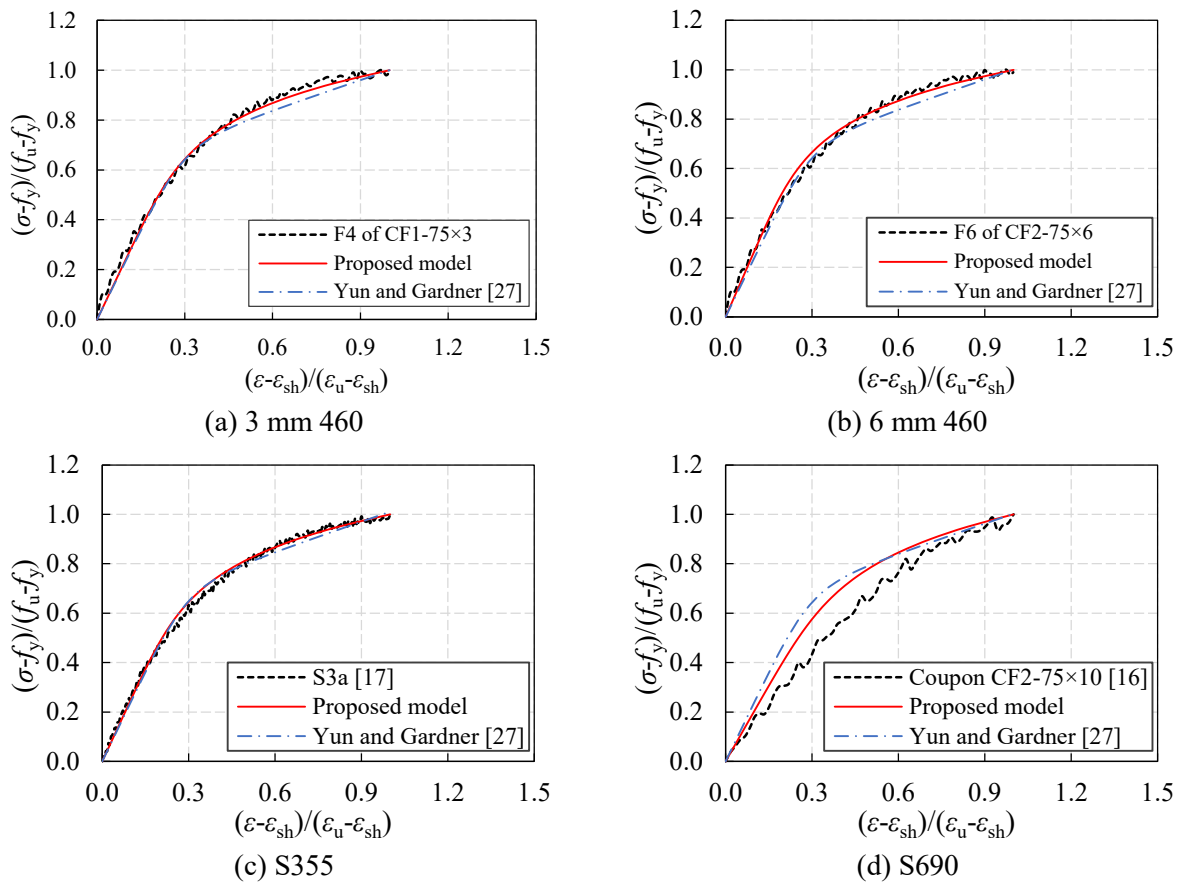


Fig. 16. Comparison of the proposed model with flat coupon test results ($m = 6.0$).

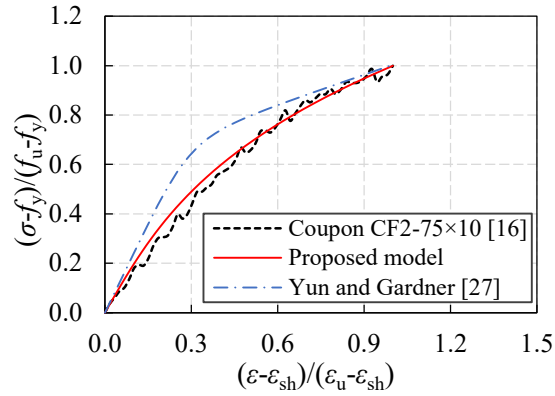
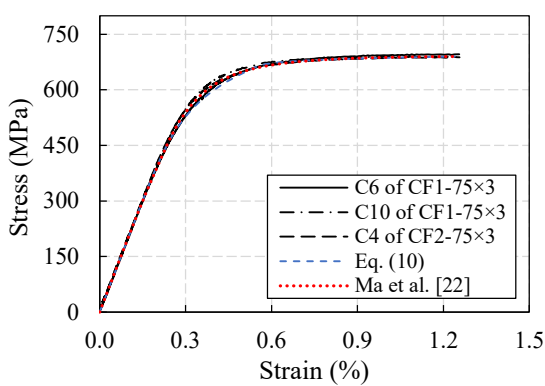
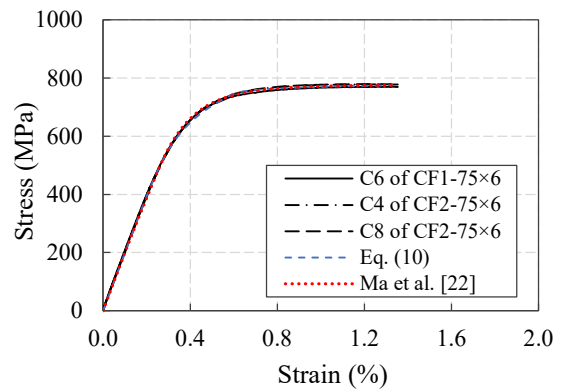


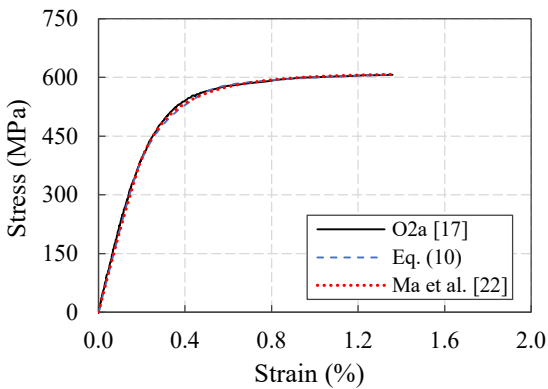
Fig. 17. Comparison of the proposed model with S690 flat coupon test results ($m = 3.0$).



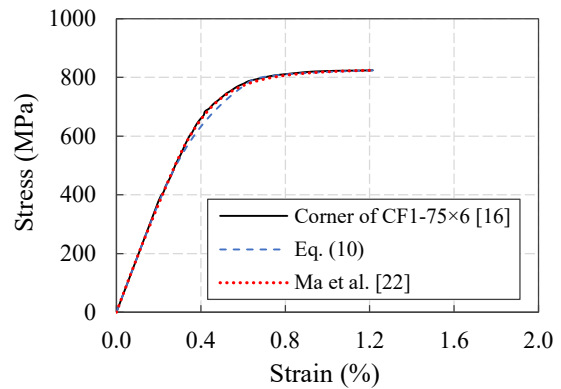
(a) 3 mm Q460



(b) 6 mm Q460

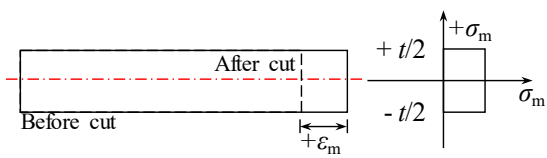


(c) S355

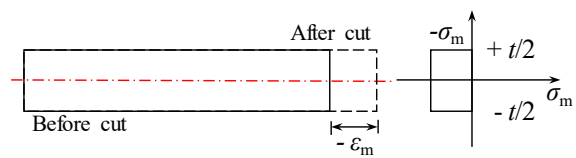


(d) S690

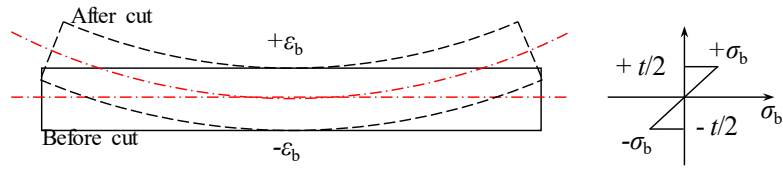
Fig. 18. Comparison of the predictive model with corner coupon test results.



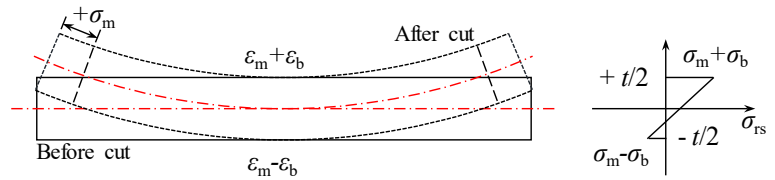
(a) Membrane residual stress σ_m acting in tension



(b) Membrane residual stress σ_m acting in compression



(c) Bending residual stress σ_b



(d) Combined bending and tensile membrane residual stresses σ_{rs}

Fig. 19. Effects of membrane and bending residual stresses.



Fig. 20. Prepared specimen for sectioning (CF2-75x6).

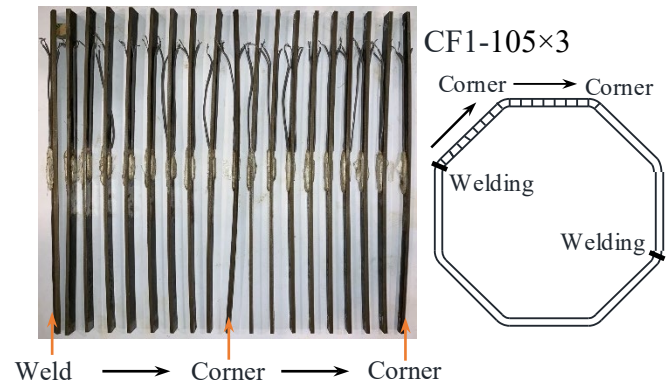
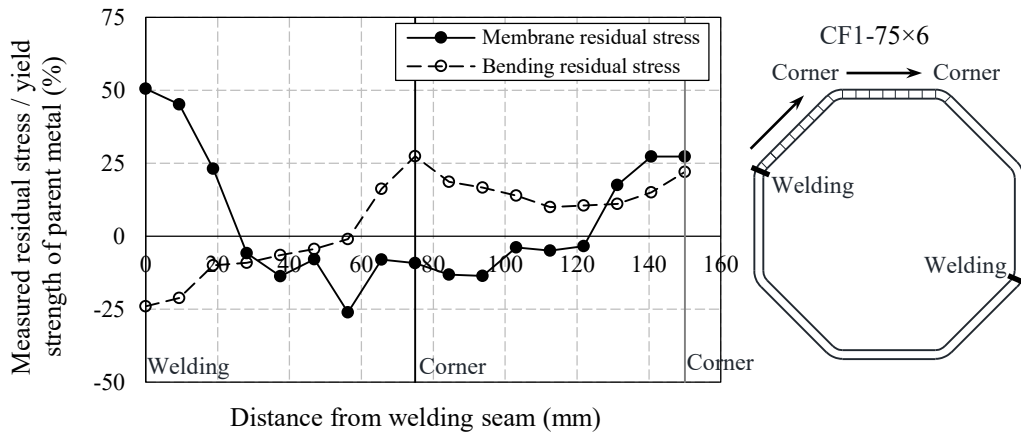
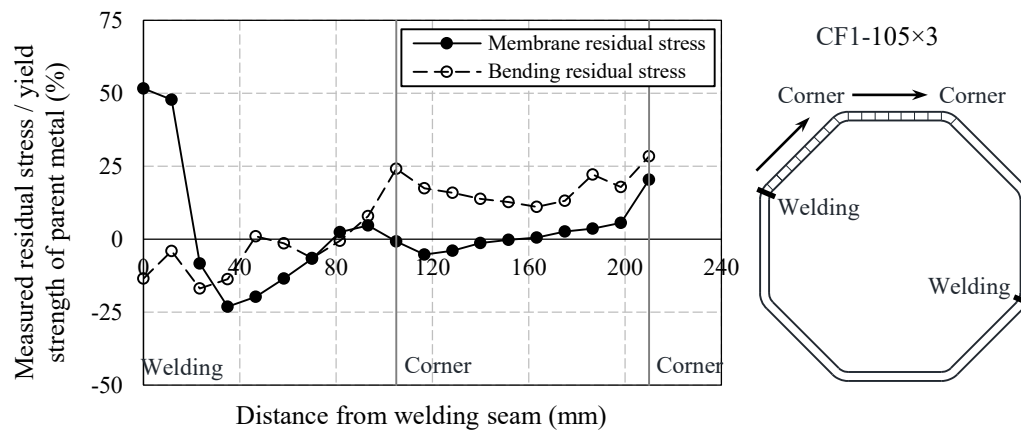


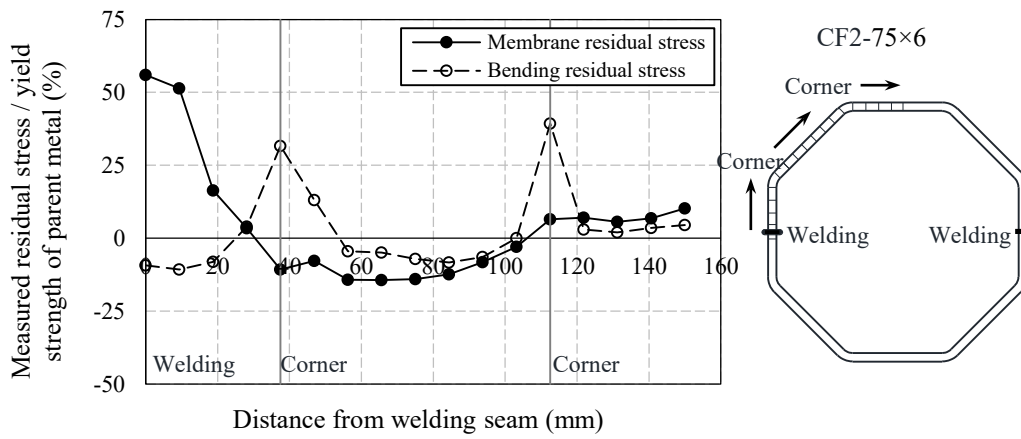
Fig. 21. Deformed strips extracted from CF1-105x3 after sectioning.



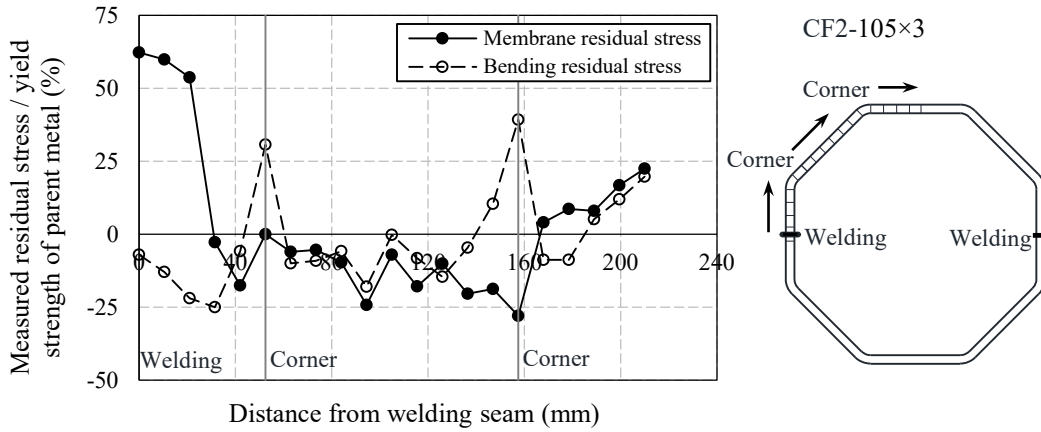
(a) CF1-75×6



(b) CF1-105×3

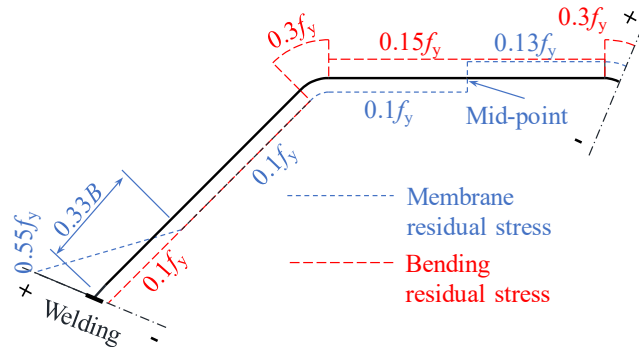


(c) CF2-75×6

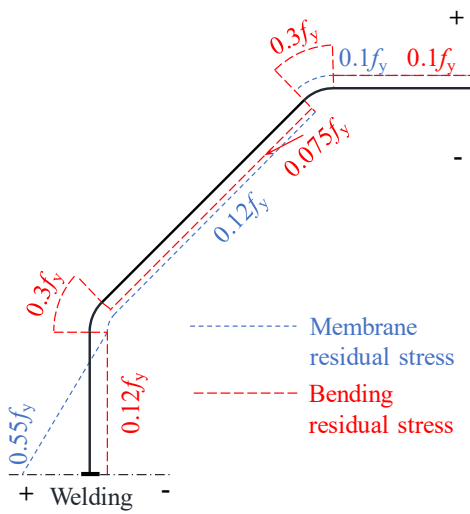


(d) CF2-105×3

Fig. 22. Longitudinal residual stress distributions of cold-formed OctHSs.

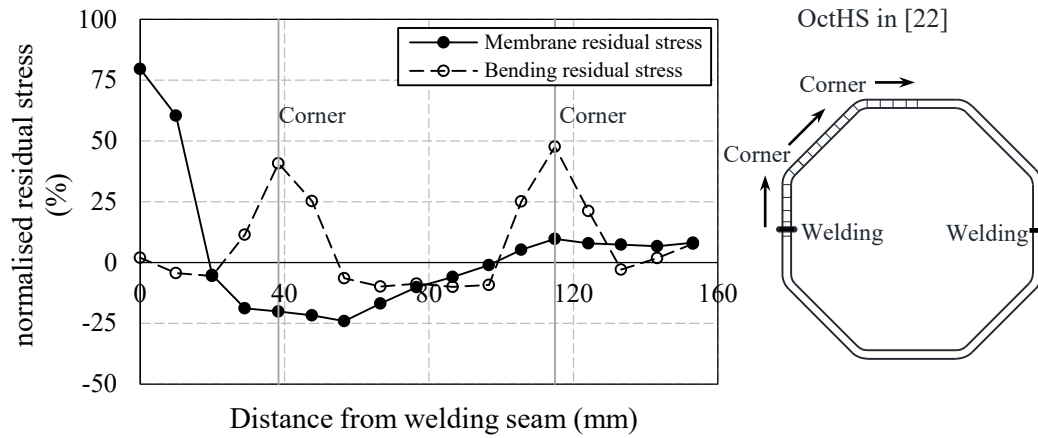


(a) CF1

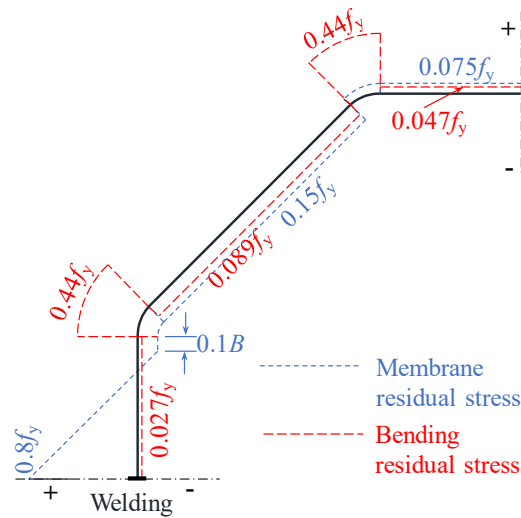


(b) CF2

Fig. 23. Predictive models for residual stress distributions in cold-formed OctHSs.



(a) Longitudinal residual stress distributions.



(b) Predictive models for residual stress distributions

Fig. 24. Longitudinal residual stress measurement in [22] (S355 steel).

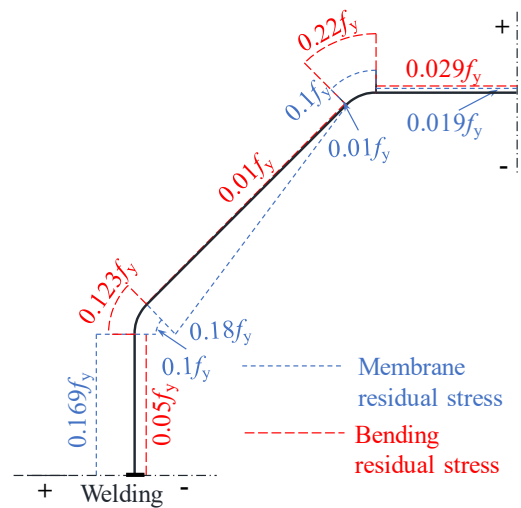


Fig. 25. Predictive models for residual stress distributions in [21] (S690 steel).