

1 **Abstract**

2 The oxidative aging of asphalt binder affects the long-term performance of flexible pavement. Apart from
3 the intrinsic property of asphalt binder, binder aging in an asphalt mixture is also affected by the mixture's
4 oxygen transport efficiency, which can be characterized by the oxygen diffusion coefficient. In this study,
5 the diffusion coefficients (D_s) of compacted asphalt mixtures of different designs were determined. The
6 same mixture samples were also aged in one dimension for a relatively long period of time. Asphalt binders
7 from different slices of the aged specimens were extracted and recovered, and their aging states were
8 determined. Influences of D_s and air void content on binder aging gradient and binder aging extent in the
9 mixture samples were systematically examined and compared. The results indicate that, as compared to air
10 voids, D_s is more closely related to the aging susceptibility of asphalt binders in asphalt mixtures. It may
11 serve as a useful and convenient parameter in mixture design optimization, modeling and prediction of in-
12 situ pavement aging, and quality control and quality assurance (QC/QA) in pavement construction.

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14 **Keywords:**

15 Oxygen diffusion, asphalt aging, asphalt mixtures, aging in asphalt mixtures, aging gradient.

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1 1. Introduction

2 As an organic material, asphalt binder in asphalt mixture is prone to oxidative aging (1, 2, 3). Aging
3 increases the stiffness and brittleness of asphalt binder and affects the long-term performance of asphalt
4 pavement (4). Therefore, the aging susceptibility of asphalt binder and asphalt mixture is one of the key
5 concerned parameters in material characterization and an important quality characteristic in pavement
6 quality control and quality assurance (QC/QA).

7 Aging-related issues of asphalt binder have been investigated in numerous studies (e.g., 5, 6, 7, 8, 9). It is
8 commonly believed that binder's oxidative aging only occurs at the top surface of asphalt pavements, as
9 the surface layer is directly exposed to the atmosphere (10, 11). However, more recent studies including
10 field investigations (12, 13, 14, 15) clearly show that asphalt binders age throughout the pavement layers.
11 Hence, it is necessary to understand factors that affect binder aging in asphalt mixtures, the aging gradient
12 of asphalt binders along the pavement depth, and the effects of binder aging on pavement performance.
13 Such information is critically important in selecting right materials for the job and predicting pavement
14 performance over time.

15 The extent of binder aging varies from the surface of an asphalt mixture to its core. The aging gradient of
16 an asphalt mixture is highly dependent on its oxygen transport efficiency, which is determined by the
17 volumetric properties of the mixture. Several pioneering studies at the Texas A&M University (16, 17, 18,
18 19, 20) investigated the issues of oxygen transport in asphalt mixtures. Assisted with X-ray computed
19 tomography (CT) technology, Prapaitrakul (19) developed an oxygen transport and reaction model, where
20 oxygen was modeled to be transported in air void (AV) channels and diffused into asphalt binder shells,
21 and the average AV size and distance between adjacent pores were used to predict binder oxidation. Han
22 (17) refined the model by including temperature and the number of air voids (AVs) in oxidation prediction.
23 Rose (18) made further contributions to AV characterization and incorporated AV properties in oxidation
24 modeling. Based on the earlier models, Han *et al.* (21) improved the asphalt oxidation model by introducing
25 the measurement of oxygen diffusivity in modified/unmodified binders and mastics. Furthermore, Jin *et al.*
26 (22) introduced a field factor to account for some other factors affecting the oxidative aging of asphalt
27 binders, including oxygen path around aggregates, binder absorption, and micro cracks. In addition, an
28 asphalt mixture morphology framework was developed and applied at the Royal Institute of Technology
29 (KTH) to investigate the aging susceptibility of asphalt mixtures by considering aggregate gradation,
30 asphalt content, and compaction level (23). Although these investigations provide valuable insights, the use
31 of CT images to characterize AVs has several limits, including inadequate CT image resolution to
32 characterize small AVs (17), challenges in computing algorithms to differentiate AVs from surrounding
33 materials accurately (24), and difficulty and cost in practical implementation (25). Considering the
34 limitation of X-ray CT, Rose (18) recommended to find alternative parameters for oxidation modeling
35 without relying on the morphological details of AVs in asphalt mixtures.

36 By using embedded oxygen sensors, Wang *et al.* (26) and Fini A *et al.* (27) found that oxygen
37 concentrations in the air underneath pavement slabs are lower than that in the atmosphere. The difference
38 in oxygen concentration drives oxygen in the air to diffuse into deeper pavement layers. Inspired by studies
39 (28, 29) on gaseous diffusions in porous media, researchers (30) developed a set of equipment and
40 procedure to measure the rate of oxygen diffusion in compacted asphalt mixtures, and they also identified
41 methods to compute the oxygen diffusion coefficients (D_s) of compacted asphalt mixtures. The coefficient
42 provides an overall indication of asphalt binder's accessibility to oxygen in a mixture and the mixture's
43 oxygen transport efficiency. Thus, apart from the intrinsic aging susceptibility of an asphalt binder, D_s is
44 expected to serve as a valuable surrogate to assess the aging susceptibility of the same asphalt binder in
45 different mixtures and to assist modeling the oxidative aging of asphalt mixture in field pavements.
46 However, relationships between D_s and asphalt mixtures' aging susceptibility remain to be systematically
47 examined.

1 To fill this research gap, this study aims to investigate the effects of D_s on the aging susceptibility of asphalt
 2 mixtures. For comparison purposes, the effects of AV content on the aging susceptibility of asphalt mixtures
 3 are also examined. A total of 32 specimens based on four mixture types were prepared for the diffusion test,
 4 and the D_s of different specimens were identified based on the test results. After the D_s was determined,
 5 all the specimens were placed into an oven at 60 °C for 21 days to simulate the long-term aging in asphalt
 6 pavements. Since D_s characterizes the one-dimensional oxygen diffusion in asphalt mixtures, the aging
 7 process should be created to simulate one-dimensional oxygen transport, too. Therefore, in simulated aging
 8 treatments, wax with a melting point of 80 °C was wrapped around the specimen to prevent oxygen diffusion
 9 from the sides of the specimens. After aging treatments, all the specimens were cut into five layers from
 10 top to bottom. Asphalt binder from each layer was extracted, recovered, and tested for the aging state using
 11 the Fourier transform infrared spectroscopy (FTIR) technique. The carbonyl (C=O) index derived from the
 12 FTIR test was used to characterize the aging state of the extracted asphalt binders. Variations in the aging
 13 state of asphalt binders from different layers reflect the aging gradient of the asphalt mixture along the depth
 14 profile. Correlations between D_s and aging gradient and between AV content and aging gradient were
 15 investigated to evaluate which parameter better predicts the aging susceptibility of compacted asphalt
 16 mixtures.

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18 2. Materials

19 2.1 Aggregate and asphalt binder

20 Crushed granite obtained from a local quarry and neat asphalt binder with a penetration grade of 60/70
 21 (typically PG64-16) supplied by Shell Hong Kong Limited Company were used to prepare the asphalt
 22 mixtures for testing. Table 1 shows some properties of the asphalt binder.

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24 **TABLE 1.** Technical properties of asphalt binder

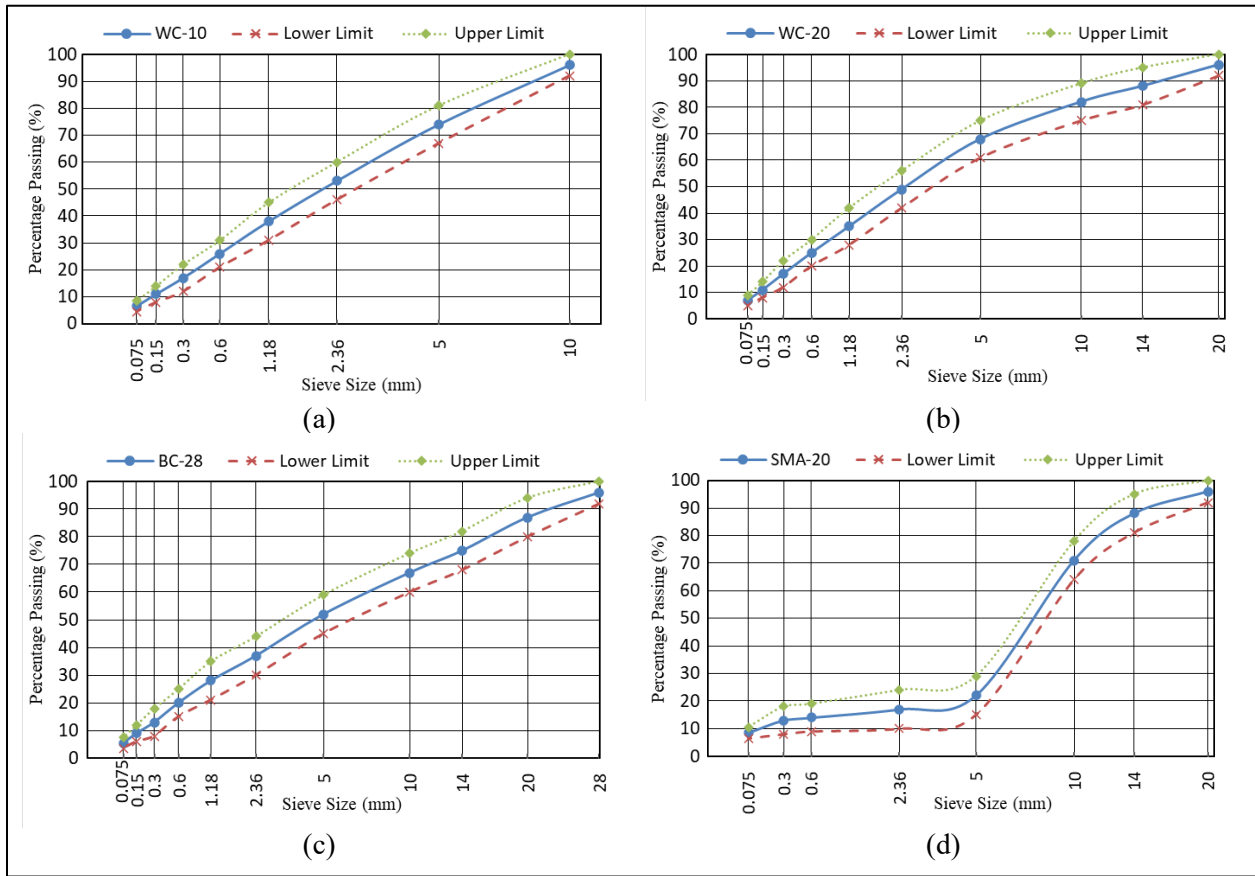
Properties	Unit	ASTM Method	Value
Specific gravity @ 25 °C		D 70	1.039
Softening Point, R&B	°C	D 36	46.2
Penetration @ 25 °C	0.1 mm	D 5	68
Ductility @ 25 °C	Cm	D 113	>100
Viscosity at 135 °C	mPa·s	D 4402	477.5
Flash point	°C	D 92	>232

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26 2.2 Aggregate gradation

27 Asphalt mixtures with four different gradations, namely WC-10, WC-20, BC-28, and SMA-20, were
 28 designed following the Asphalt Institute Manual Series (31). WC, BC, and SMA are the abbreviations for
 29 wearing course, base (binder) course, and stone mastic asphalt, respectively. Fig. 1 shows the aggregate
 30 gradations of the mixtures, and the binder content for WC-10, WC-20, BC-28, and SMA-20 is designed as
 31 6%, 5%, 4.5%, and 6.3%, respectively.

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Figure 1. Aggregate gradations. (a) WC-10, (b) WC-20, (c) BC-28, (d) SMA-20.

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5 2.3 Specimens

6 The schematic diagrams of the preparation process of the test specimens are shown in Figure 2. For each
7 type of gradation, eight cylindrical specimens with a diameter of 150 mm and a height of 80 mm were
8 compacted using the Superpave[®] gyratory compactor. The AVs of those eight specimens were purposely
9 controlled to range from 3% to 10% to investigate the potential effects of AV content on the D_s of mixtures
10 and the effects of AV content and D_s on mixtures' aging susceptibility. To reduce the effects of possible
11 uneven compaction, the top and bottom portions of the specimens of 20 mm height were cut off and
12 discarded. The middle part of the specimens was used for the oxygen diffusion coefficient test and
13 subsequent aging treatment. In laboratory aging treatment, the heating temperature was set at 60 °C to
14 reduce the risk of binder drain down. Petersen et al. (32) recommended a dual oxidation mechanism: The
15 oxygen-containing functional groups initially increase rapidly ('initial jump') and then increase slowly at a
16 nearly constant rate (long-term aging reaction stage). In a parallel study, we have identified changes in the
17 aging state of different asphalt binders at 60 °C and under atmospheric pressure. The results indicate that
18 the maximum time for the carbonyl area to exceed the 'initial jump' phase is about 20 days. Therefore, the
19 21-day period at 60 °C in oven is applied in this study to simulate the long-term aging process for asphalt
20 mixtures. The aged specimens were cut into five layers for the extraction and recovery of asphalt binders.
21 The layers were marked as layer one to layer five from top to bottom, and the location of each layer along
22 the vertical axis starting from the top was used to designate the layer position in the specimen, as shown in
23 the bottom left corner of Fig. 2.

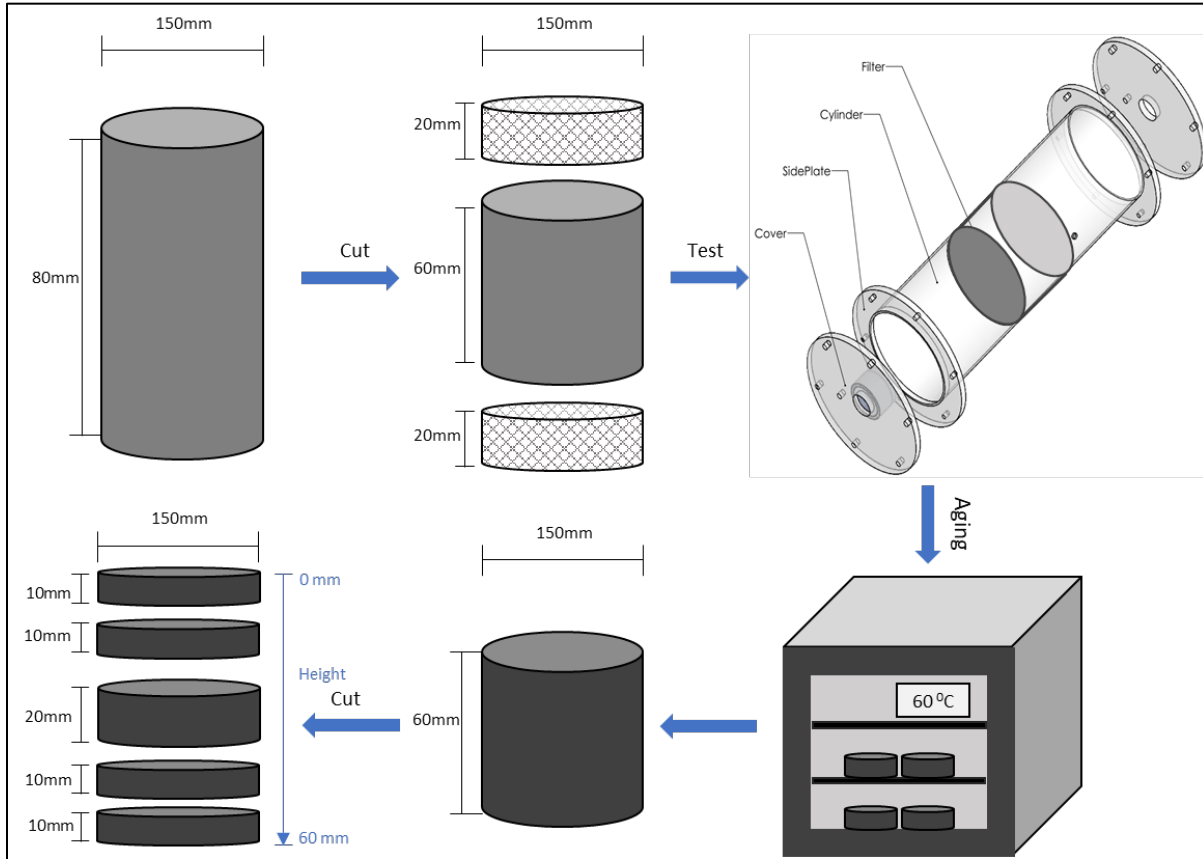


Figure 2. The schematic diagrams of specimen preparation.

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3. Experimental Tests and Analysis

3.1 Experimental tests

3.1.1 Tests for the oxygen diffusion coefficient (D_s)

A diffusion testing system was used to test the D_s of compacted asphalt mixtures (28). The diffusion testing system consists of two components: One is the diffusion chamber, and the other is the oxygen concentration recording system. A mixture specimen is placed in the middle of the diffusion chamber. The difference in oxygen concentration drives oxygen to diffuse from one side of the specimen to the other. Time taken for the bottom and upper chambers to achieve balance in oxygen content can be used to back calculate the diffusion coefficient of the specimen. Based on previous work, the mounting system of the diffusion apparatus was redesigned in this study to provide better sealing performance. In addition, the oxygen sensor was substituted with a more stable commercial product, Apogee SO-210. The updated testing system is shown in Fig. 3.

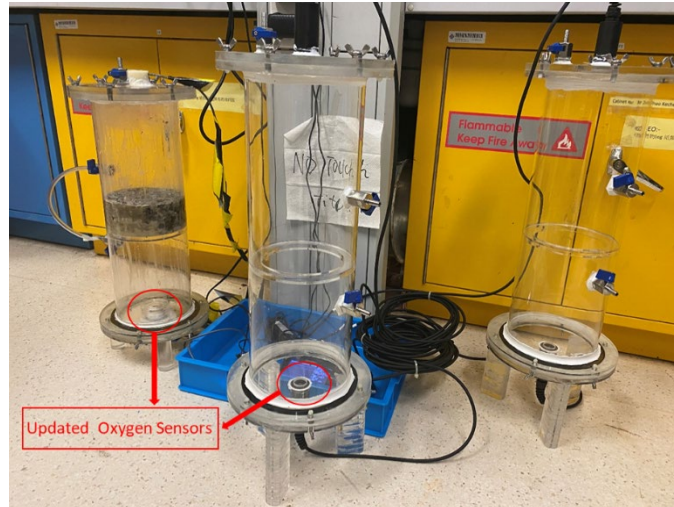


Figure 3. The diffusion testing system

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The testing procedure for the determination of D_s is summarized as follows:

(1) Firstly, assemble the equipment and supply nitrogen to the chamber until the oxygen content approaches zero. To avoid nitrogen leakage, all the air valves and screws should be tightened. The oxygen content in the chamber is recorded continuously for 24 hours to check the sealing performance.

(2) After checking the sealing performance, open the upper cover and place the asphalt mixture specimen of a diameter of 150 mm and a height of 60 mm on the middle plate. It has been recommended to use Vaseline to seal the gap between the specimen and the chamber wall (28), but it is not adopted in this study since the melting point for Vaseline is lower than 60 °C. For the convenience of implementing one-dimensional aging treatment, wax with a melting point of 80 °C can be applied to seal the gap.

(3) The sealing of the diffusion system should be rechecked before testing. Firstly, tighten up all the screws and close the two air valves in the walls. Then, open the air valves in the upper and lower cover and blast nitrogen throughout the two opening air valves to expel oxygen in chambers until the oxygen content approaches zero. Then, close the two air valves and record the oxygen content in the chamber. If the oxygen content keeps stable in the next one hour, the apparatus is deemed to be well sealed. Otherwise, all the screws and air valves should be checked and tightened again.

(4) After checking the sealing conditions, open the two upper air valves and use an air blower to expel the nitrogen in the upper chamber until the oxygen content recovers to the same level in the air. Then, tighten the two upper air valves.

(5) During the diffusion process, the oxygen contents in the upper and lower chambers are recorded using a data logger (GL240). When the oxygen content in the upper chamber tends to be the same as the oxygen content in the lower chamber, the diffusion process is completed. However, one may also interrupt the diffusion process in the middle and use the collected data to calculate the diffusion coefficient.

Based on the test results, the D_s of the mixture is calculated with the proposed analytical solution, as will be introduced in section 3.2.1.

3.1.2 One-dimensional aging treatment

The oxygen diffusion coefficient was obtained based on the diffusion test in one dimension. Therefore, one-dimensional aging treatment was designed to investigate the relationship between the oxygen diffusion

1 coefficient and the aging susceptibility of asphalt mixtures. To achieve one-dimensional oven aging, the
2 sides of the asphalt mixtures should be well sealed. For convenience, specimens covered with solidified
3 wax used in the diffusion test were taken out for one-dimensional aging treatment. The specimens were
4 aged in the oven for 21 days to simulate the long-term aging in the field, and the aging temperature was set
5 as 60 °C to avoid wax melting since the melting point of the wax product is 80 °C. Furthermore, elastic
6 gauze was wrapped around the mixture to prevent the wax from peeling off the specimen. Under this
7 condition, oxygen can only transport in the mixture samples through the vertical direction, and the aging
8 treatment setup in the oven is shown in Fig. 4. After aging treatment, the specimen was immediately
9 wrapped in a vacuum bag to prevent further aging in the air, as shown in the left corner of Fig.4.

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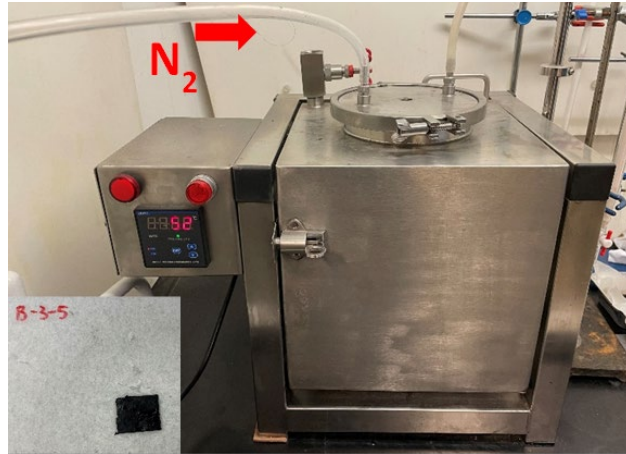
Figure 4. One-dimensional oven aging

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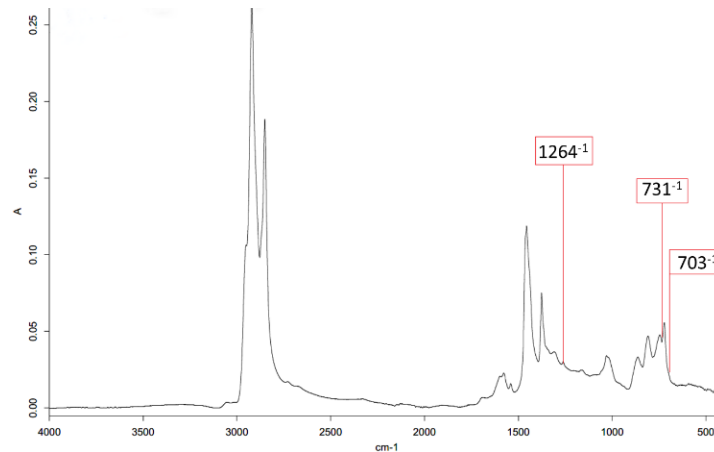
14 3.1.3 Binder extraction and recovery

15 Dichloromethane was used as the extraction solvent. Asphalt binders were extracted and recovered from
16 loose mixtures before compaction. The aging states of binders in the loose mixtures serve as a baseline to
17 evaluate binder changes after long-term aging treatment. As introduced in Fig. 2, the aged, compacted
18 mixture specimen was cut into five layers. The surrounded part of around 50 mm was discarded for each
19 layer, and the central part was crushed and soaked in dichloromethane for one hour to dissolve the asphalt
20 binder. The filtered solution of 100 ml was placed into a tube for centrifugation at 3,500 rpm for 20 minutes
21 to remove the fine particles further. Generally, the centrifuged solution is processed in a rotary evaporator
22 to distill the dichloromethane solvent. However, this method is not suitable in this study because the
23 extracted asphalt binder from each layer is too little to be poured out from the flask of a rotary evaporator.
24 Therefore, a novel approach was developed to recover the asphalt binder. As shown in Fig. 5, a customized
25 oven equipped with a chamber filled with nitrogen was used to evaporate the dichloromethane. The
26 centrifuged solution was placed into the chamber covered with silicon paper and slightly heated at 40 °C
27 for one hour to evaporate most of the dichloromethane. Then, the chamber temperature was raised to 60 °C
28 for 30 mins to recover the asphalt binders, and the recovered binder is shown in the corner of Fig. 5. To
29 check whether the dichloromethane was thoroughly removed, the infrared spectrum of the recovered binder
30 was examined, as shown in Fig. 6. No obvious characteristic peaks correspond to dichloromethane at 703⁻¹,
31 731⁻¹, and 1264⁻¹, indicating that dichloromethane is thoroughly removed.

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2 **Figure 5.** Customized oven and the recovered asphalt binder
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5 **Figure 6.** The infrared spectra of the extracted asphalt binder
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7 3.1.4 Examination of the samples using FTIR

8 The aging states of the specimens were examined using a PerkinElmer FTIR spectrometer. The measured
9 wavelength was set from 600 to 4000 cm^{-1} at a resolution of 4 cm^{-1} , and a baseline correction was performed
10 for each specimen when interpreting the FTIR results. All the specimens were scanned 32 times under the
11 attenuated total reflectance (ATR) mode, and the average results of the 32 scanings were used for analysis.
12 Furthermore, five replicate samples were tested for each specimen, and the average result was used to
13 represent the aging state of the recovered asphalt binder. Therefore, the actual aging states of the binders
14 can be confidently obtained.

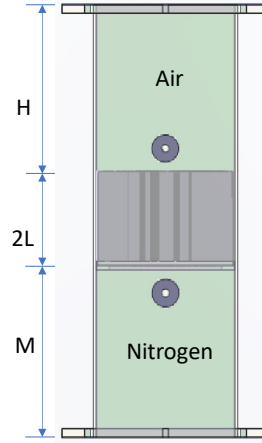
15 3.2 Analysis of experimental results

16 3.2.1 Determination of the oxygen diffusion coefficient (D_s)

17 Wen and Wang (28) introduced the computational methods to identify the oxygen diffusion coefficient D_s
18 using the recorded oxygen contents in the two chambers, and the detailed methodology was not repeated in
19 this article. Fig. 7 shows the sketch of the diffusion system. For easy reference, the analytical solution

1 without considering oxygen consumption in compacted asphalt mixtures is shown as follows, and D_s can
 2 be identified using the Solver Tool in Excel.

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5 **Figure 7.** Dimensions in the diffusion system

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$$\gamma = \frac{H}{M}, \beta = \frac{H}{L\varepsilon}, \tau = \frac{D_s t}{L^2 \varepsilon} \quad (1)$$

8
$$\alpha_1 = \left[\frac{1}{2\beta} (1 + \gamma) - \frac{1}{3\beta^2} (\gamma^2 - \gamma + 1) + \frac{2}{45\beta^3} (4\gamma^3 - 3\gamma^2 - 3\gamma + 4) \right]^{1/2} \quad (2)$$

9 For the upper chamber,
$$A = -\frac{\gamma}{\beta^2} - \frac{\alpha_1^2}{\gamma} \quad (3)$$

10 For the lower chamber,
$$A = \left[\alpha_1^4 + \frac{\gamma^2}{\beta^4} + (1 + \gamma^2) \left(\frac{\alpha_1}{\beta} \right)^2 \right]^{1/2} \quad (4)$$

11
$$B = \alpha_1^4 \frac{\beta}{\gamma} + \alpha_1^2 \left(\frac{1}{\beta\gamma} + \frac{\gamma}{\beta} + \frac{1}{2\gamma} + \frac{1}{2} \right) + \frac{\gamma}{\beta^3} + \frac{\gamma}{2\beta^2} + \frac{1}{2\beta^2} \quad (5)$$

12
$$C(t) = C_{s0} \left[1 / \left(1 + \frac{1}{\gamma} + \frac{2}{\beta} \right) - \frac{A}{B} e^{-\alpha_1^2 \tau} \right] - C_{r0} \quad (6)$$

13 Where γ , β , and τ are intermediate variables, D_s is the diffusion coefficient to be determined, α_1 , A and B
 14 are the calculation factors, H is the height of the upper chamber, M is the height of the lower chamber, ε is
 15 the porosity of compacted asphalt mixtures, C_{s0} is the initial oxygen concentration of the upper chamber,
 16 C_{r0} is the initial oxygen concentration of the lower chamber, and $C(t)$ is the analytical solution of the
 17 oxygen concentration changed with diffusion time.

18 **3.2.2 Determination of the chemical aging indicator**

19 FTIR results help reveal changes in the aging state of asphalt binders through changes in characteristic
 20 functional groups such as carboxylic acids, ketones, and sulfoxides (33). Two methods are commonly
 21 recommended to analyze the FTIR results in existing asphalt binder aging studies. One is to use the peak
 22 absorbance of ketone at around 1695 cm^{-1} (34) as an indicator of aging, and the other one is to use the
 23 carbonyl (C=O) index (34, 35, 36) as an aging indicator, as shown in Eq. 7 and Eq. 8. Considering the
 24 variation of experiments, the carbonyl index based on peak areas is more robust than the aging index only

1 using the peak value of ketone. Therefore, the carbonyl index is chosen in this study to represent the aging
2 state of asphalt binders.

3 Carbonyl index: $I_{C=O} = AR_{1,700}/\sum AR$ (7)

4 $\sum AR = AR_{1,700} + AR_{1,600} + AR_{1,460} + AR_{1,376} + AR_{1,030} + AR_{864} + AR_{814} + AR_{743} + AR_{724} +$
5 $AR_{(2953,2923,2862)}$ (8)

6 Where $AR_{1,700}$ is the integrated area for carbonyl groups from 1,650 to 1,820 cm^{-1} , and $AR_{1,600}$, $AR_{1,460}$,
7 $AR_{1,376}$, AR_{864} , AR_{814} , AR_{743} , AR_{724} , $AR_{(2953,2923,2862)}$ are the band areas for those functional groups that remain
8 relatively stable during the aging process.

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10 4. Results and discussion

11 4.1 Oxygen diffusion coefficients (D_s)

12 Fig. 8 shows changes in the measured and analytical solution of oxygen concentrations with diffusion time.
13 The upper data serials in the figures represent oxygen concentrations in the upper air chamber, and the
14 lower data serials refer to oxygen concentrations in the lower air chamber. Driven by the difference in
15 oxygen concentration in the upper and lower chambers, oxygen diffuses from the upper chamber into the
16 lower one and finally tends to achieve a balance. The longer time for the diffusion system to achieve a
17 steady state, the smaller the D_s is. Testing results indicate that most specimens can arrive at a steady state
18 in a few days, but several specimens need more than one month to reach a steady state. Time to reach the
19 steady state is apparently dependent on the volumetric properties of asphalt mixtures.

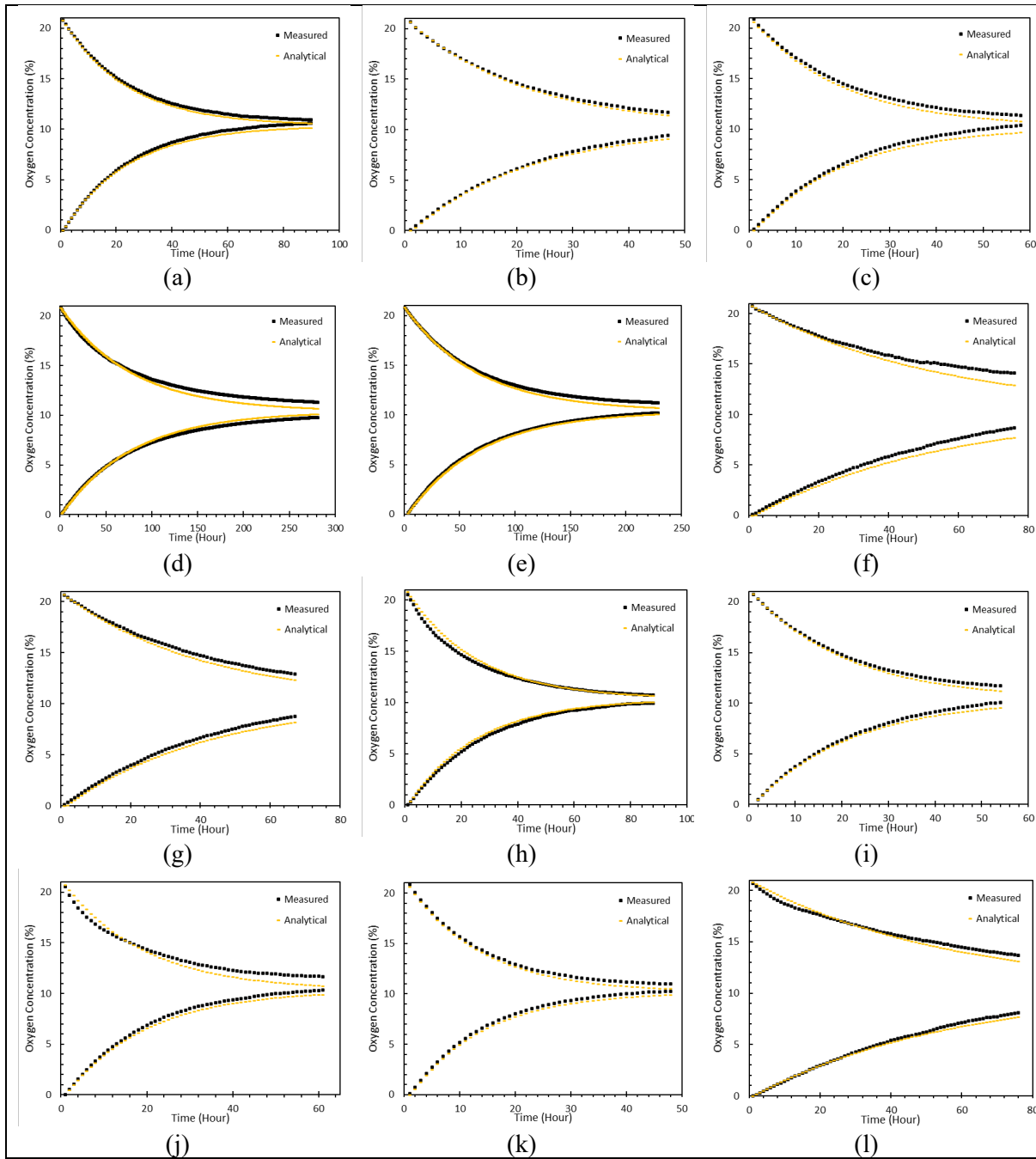
20 When the testing period is too long, the leaking effect of the equipment can no longer be ignored. An
21 example is shown in Fig. 8 (w), where the oxygen concentration in the upper air chamber is nearly
22 unchanged whereas the oxygen concentration in the lower chamber is gradually increased. This is caused
23 by the extremely low oxygen transportation efficiency of the sample. As a result, the rate of oxygen
24 diffusing from the upper chamber into the lower one is even lower than the rate of oxygen leaking into the
25 diffusion chambers. Therefore, to control the leaking effects on the computational accuracy of D_s , a failure
26 criteria (C_f) for the diffusion test is proposed, as shown in Eq. 9. In the equation, C_f reflects the leaking
27 level of the diffusion test, and a higher C_f means that leaking is more severe. Based on the test results of
28 this study, it is recommended to limit the value of C_f to be within 2 %.

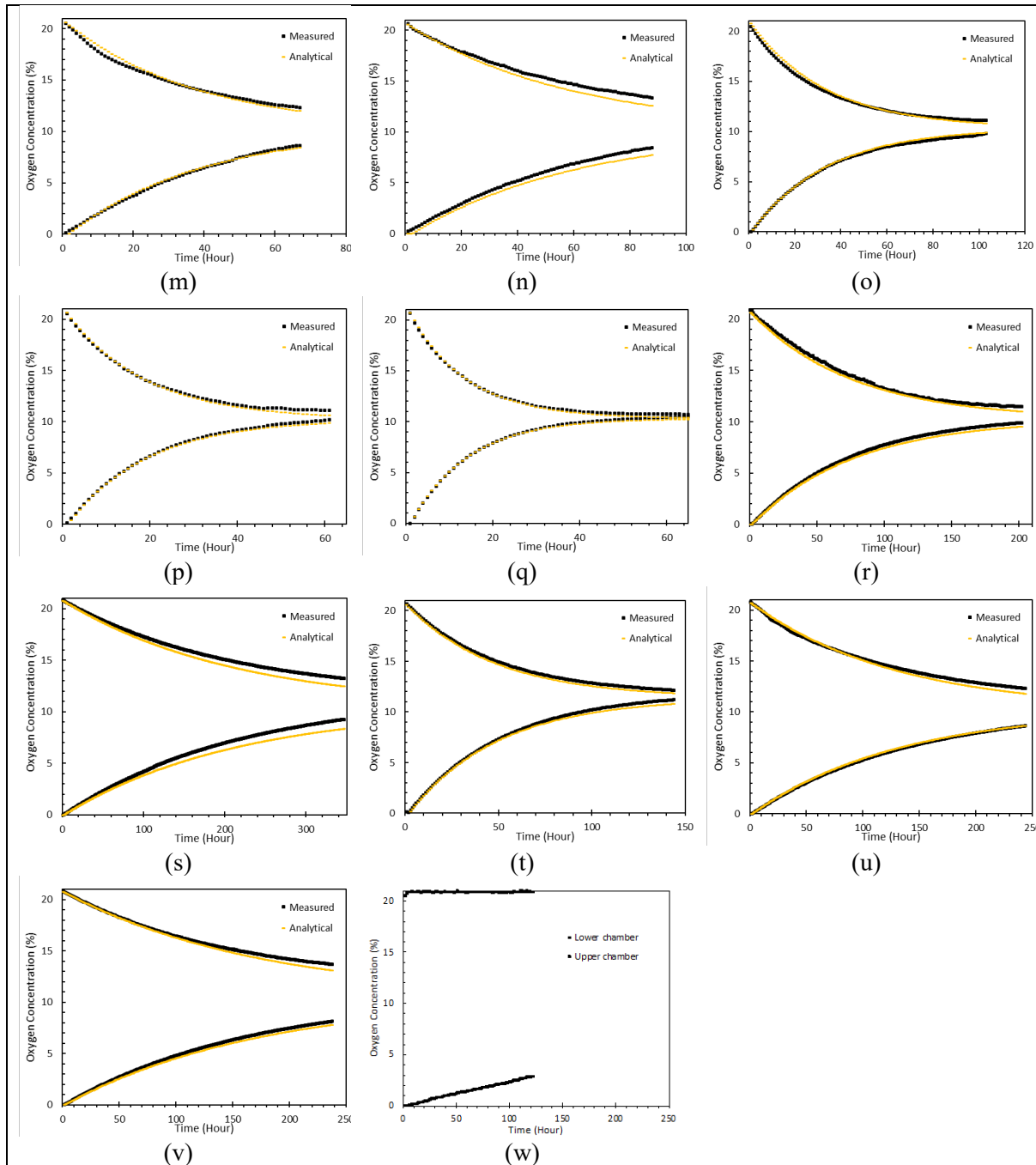
29 $C_f = (C_{sf} + C_{rf}) - (C_{s0} + C_{r0})$ (9)

30 Where, C_f is the indicator of the system leaking, C_{s0} is the initial oxygen concentration of the upper
31 chamber, C_{r0} is the initial oxygen concentration of the lower chamber, C_{sf} is final oxygen concentration of
32 the upper chamber, and C_{rf} is the final oxygen concentration of the lower chamber.

33 Based on the evaluation of leaking, three specimens for WC-10 and SMA-20 and two specimens for WC-
34 20 and BC-28 were abandoned. The common characterization of those specimens is their very low D_s . At
35 very low D_s , because it takes a long time for the two chambers to reach equilibrium and leaking problem
36 may interfere the results, it is not recommended to directly provide a D_s value for such a mixture. Instead,
37 it is suggested to label a mixture with D_s less than a certain value instead of giving an accurate number. On

- 1 the other hand, the diffusion testing system may be further upgraded to control leaking so that mixture
- 2 samples with very low air voids can be tested as well.





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2 **Figure 8.** Results for diffusion test: (a) WC-10-1; (b) WC-10-2; (c) WC-10-3; (d) WC-10-4; (e) WC-10-5;
 3 (f) WC-20-1; (g) WC-20-2; (h) WC-20-3; (i) WC-20-4; (j) WC-20-5; (k) WC-20-6; (l) BC-28-1; (m) BC-
 4 28-2; (n) BC-28-3; (o) BC-28-4; (p) BC-28-5; (q) BC-28-6; (r) SMA-20-1; (s) SMA-20-2; (t) SMA-20-3;
 5 (u) SMA-20-4; (v) SMA-20-5; (w) SMA-20-6.

6 Based on data from the oxygen diffusion process shown in Fig. 8, the D_s of each specimen can be calculated.
 7 Table 2 presents the calculated D_s for each specimen and the corresponding AV content. It is noteworthy
 8 that no direct relationship exists between the D_s value and the AV content for different asphalt mixtures.
 9 For instance, the AV contents for WC-20-4, SMA-20-2, and BC-28-5 are all around 6%, but differences in

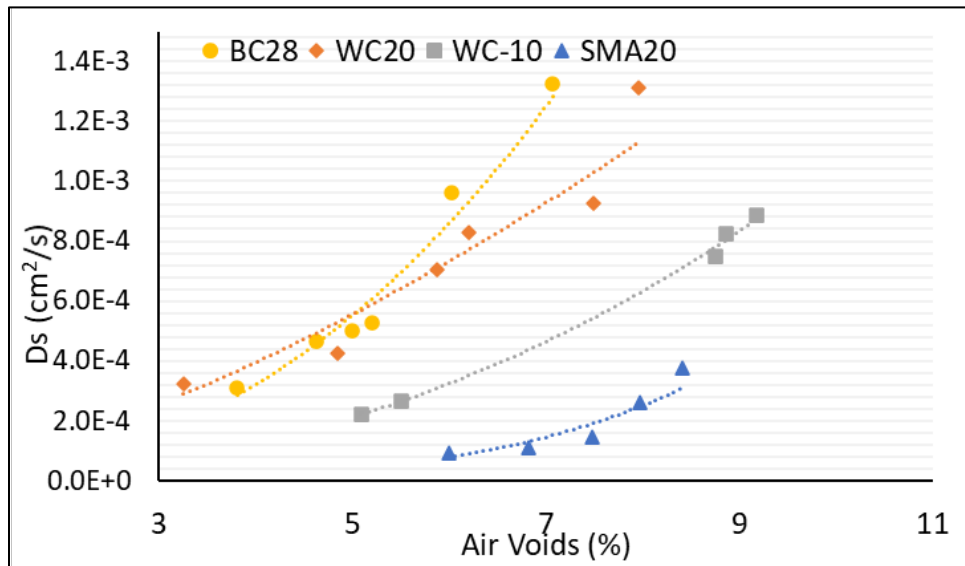
1 their D_s values are close to ten times. This implies that mix design plays an important role in oxygen
 2 transport in asphalt mixtures. On the other hand, for mixtures of the same gradation, the trend is quite
 3 clear— D_s increases with rising AV content. The relationships between D_s and AV content are shown in
 4 Fig. 9. Power functions are used to fit the D_s and AV content for mixtures with the same gradation, and the
 5 results are shown as Eq. 10 to Eq. 13. Satisfactory regression results support using a general function (Eq.
 6 14) to depict the relationship between the D_s value and AV content of a compacted asphalt mixture.

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9 **TABLE 2.** The air voids and D_s for each specimen

Specimen	Air voids (%)	D_s (cm ² /s)	Specimen	Air voids (%)	D_s (cm ² /s)
WC-10-1	8.8	7.48E-04	SMA-20-1	8.0	2.60E-04
WC-10-2	8.9	8.25E-04	SMA-20-2	6.0	9.27E-05
WC-10-3	9.2	8.87E-04	SMA-20-3	8.4	3.75E-04
WC-10-4	5.1	2.22E-04	SMA-20-4	7.5	1.47E-04
WC-10-5	5.5	2.64E-04	SMA-20-5	6.8	1.09E-04
WC-20-1	3.2	3.23E-04	BC-28-1	3.8	3.10E-04
WC-20-2	4.9	4.23E-04	BC-28-2	4.6	4.65E-04
WC-20-3	5.9	7.04E-04	BC-28-3	5.0	5.00E-04
WC-20-4	6.1	8.30E-04	BC-28-4	5.2	5.29E-04
WC-20-5	7.5	9.26E-04	BC-28-5	6.0	9.60E-04
WC-20-6	8.0	1.31E-03	BC-28-6	7.1	1.33E-03

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12 **Figure 9.** Relationship between air voids and D_s

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WC-10: $D_s = 5E-6 \varepsilon^{2.3289}$ $R^2 = 0.9956$ (10)

15

WC-20: $D_s = 5E-5 \varepsilon^{1.5279}$ $R^2 = 0.9143$ (11)

16

BC-28: $D_s = 1E-5 \varepsilon^{2.4338}$ $R^2 = 0.9774$ (12)

1 SMA-20: $D_s = 5E-8 \epsilon^{4.1285} R^2 = 0.9133$ (13)

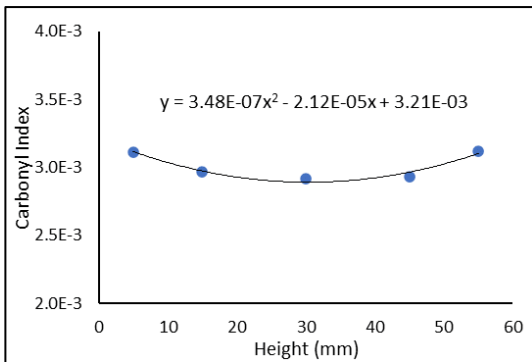
2 $D_s = K_1 \cdot \epsilon^{k_2}$ (14)

3 Where D_s is the oxygen diffusion coefficient, ϵ is the AV content, and K_1 and K_2 are the fitting parameters.

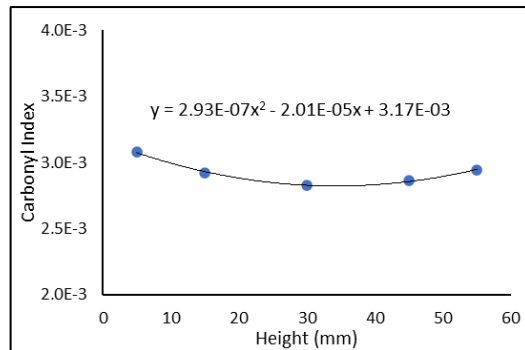
4 Moreover, the unique curves for different asphalt mixture types in Fig. 9 suggest that the oxygen diffusion
 5 coefficient is also related to other volumetric properties, including the gradation and the nominal maximum
 6 aggregate size of the mixture. For instance, both WC-10 and WC-20 belong to the wearing course, but the
 7 oxygen diffusion coefficient of WC-20 is quite higher than that for WC-10 at the same AV level. The
 8 nominal maximum aggregate size seemingly plays a positive role in determining the oxygen diffusion
 9 coefficient. Conversely, SMA-20 and WC-20 have the same nominal maximum aggregate size, but the
 10 oxygen diffusion coefficient of SMA-20 is much smaller than that of WC-20 and even smaller than that of
 11 WC-10. Apparently, difference in D_s is affected by gradation characteristics. The gap gradation with high
 12 binder content, such as SMA-20, reduces the inter-connected pores in asphalt mixtures; therefore, oxygen
 13 transport in the SMA mixtures is relatively slow.

14 4.2 D_s and the aging gradient in asphalt mixtures

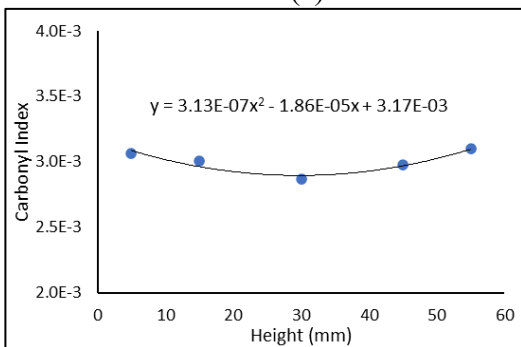
15 Does D_s affect the aging susceptibility of compacted asphalt mixtures? To answer this question,
 16 relationships between D_s and the aging gradients of the asphalt mixture samples were investigated. The
 17 band area of each functional group listed in Eq. 8 was measured using a commercial FTIR analysis software,
 18 OPUS. After deducting the integrated area of carbonyl formed in the mixing process (i.e., excluding the
 19 short-term aging effect), the carbonyl index of binder in each layer due to simulated long-term aging was
 20 calculated. Fig. 10 reveals the distribution of the carbonyl index from the top to the bottom layer for
 21 different mixtures. In the figure, the horizontal axis refers to the position of each layer in Fig. 2. For instance,
 22 the heights for layer-one and layer-five are 5 mm and 55 mm, respectively.



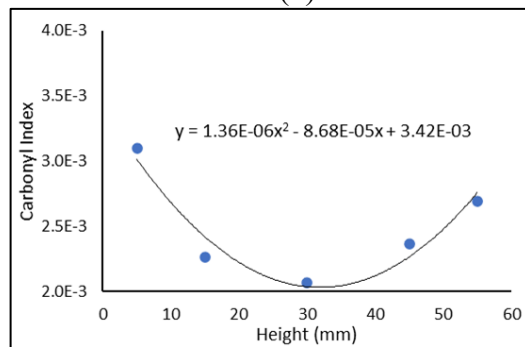
(a)



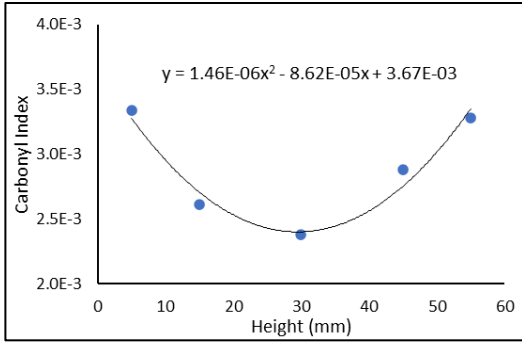
(b)



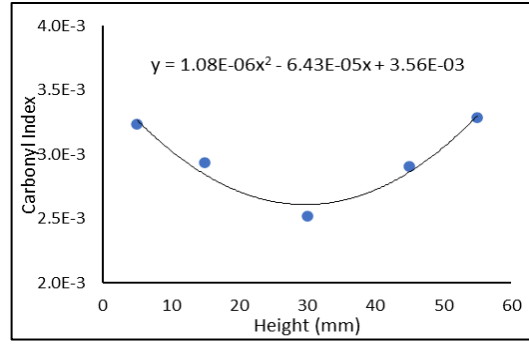
(c)



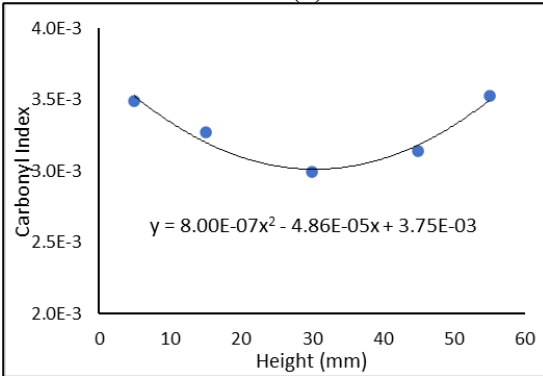
(d)



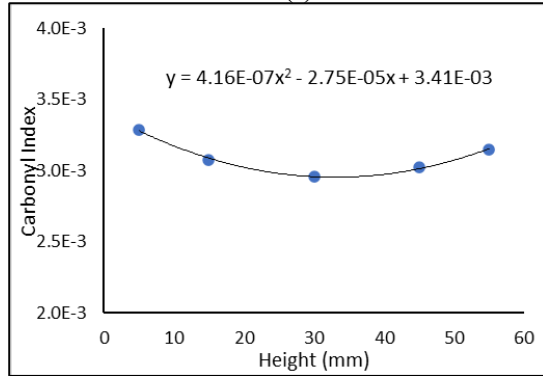
(e)



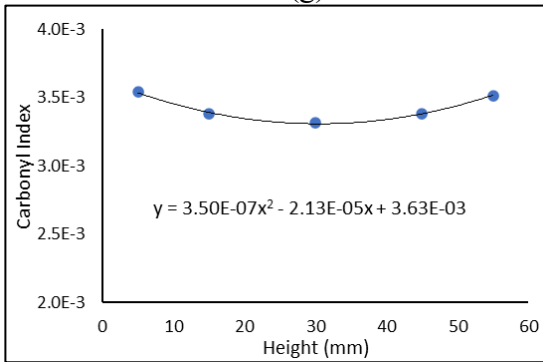
(f)



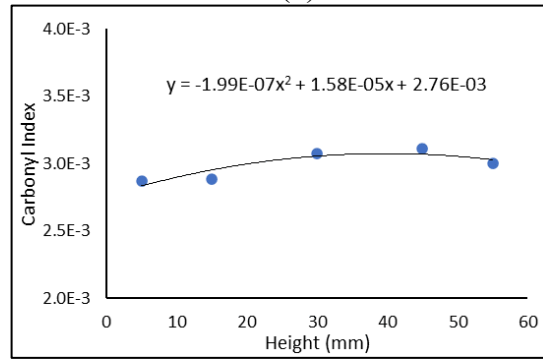
(g)



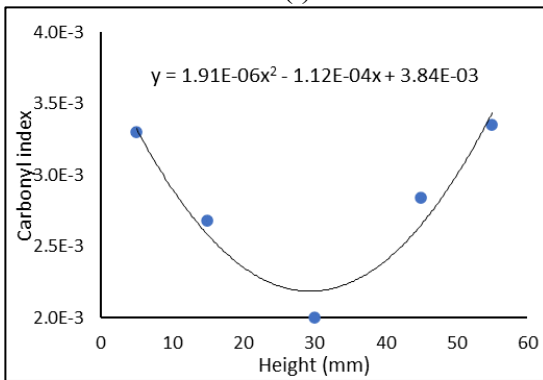
(h)



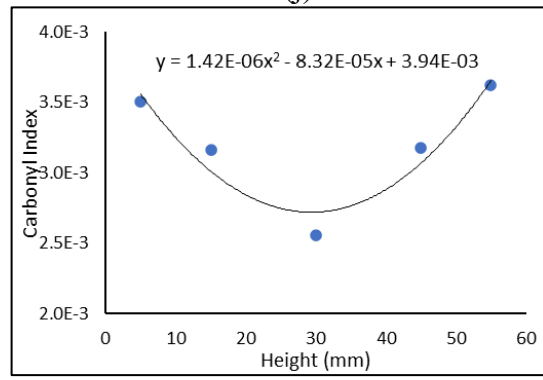
(i)



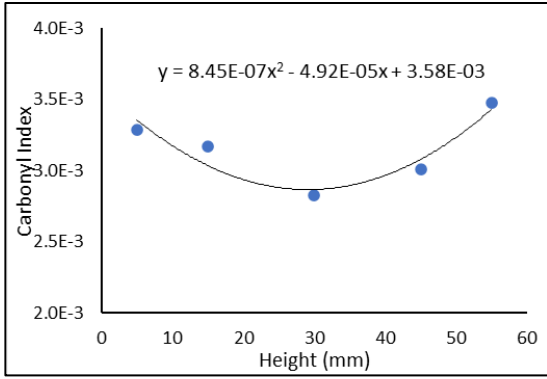
(j)



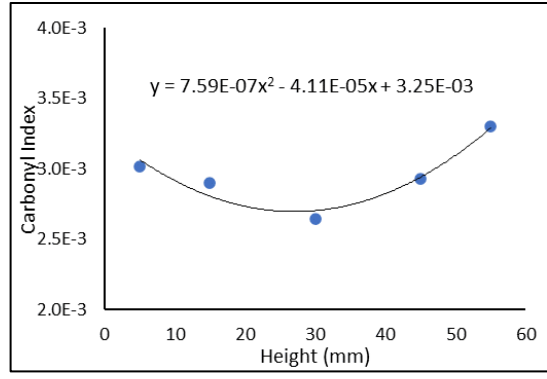
(k)



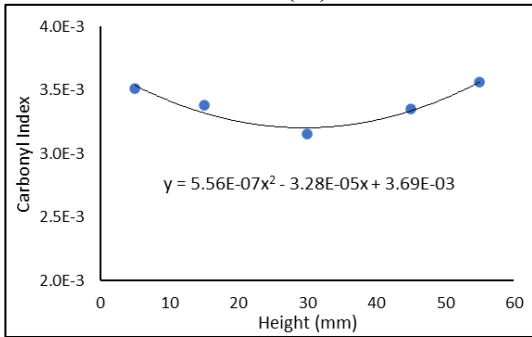
(l)



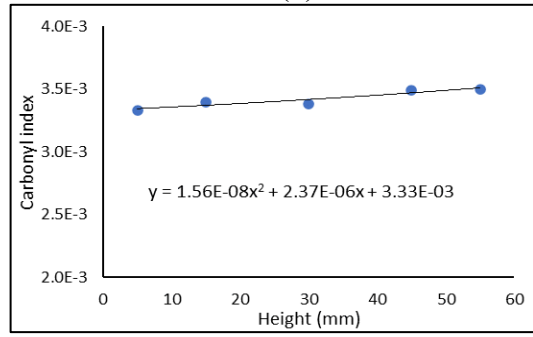
(m)



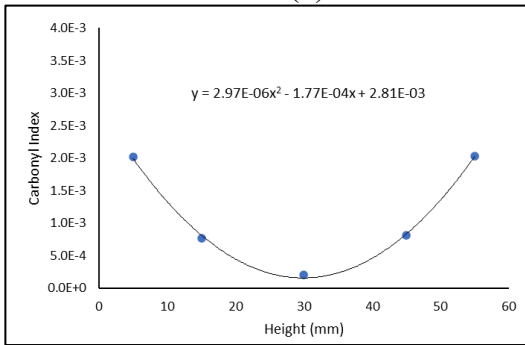
(n)



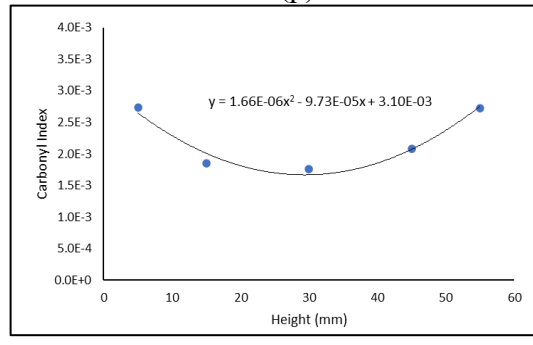
(o)



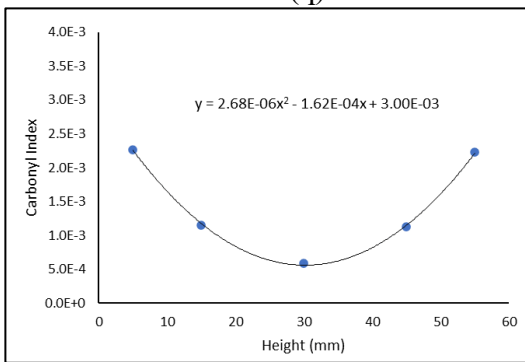
(p)



(q)



(r)



(s)

1

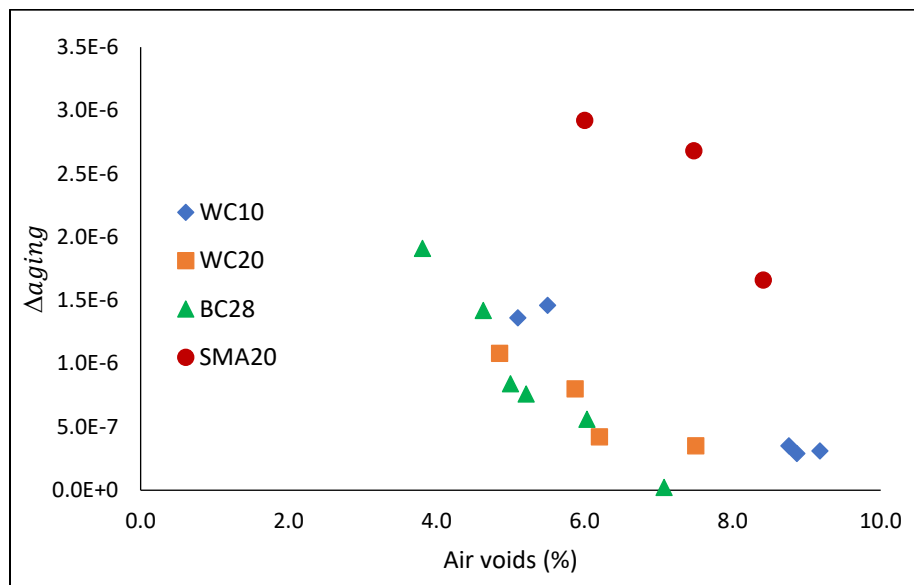
2 **Figure. 10** Distribution of carbonyl index in different layers for WC-10: (a) WC-10-1; (b) WC-10-2; (c)
 3 WC-10-3; (d) WC-10-4; (e) WC-10-5; (f) WC-20-2; (g) WC-20-3; (h) WC-20-4; (i) WC-20-5; (j) WC-

1 20-6; (k) BC-28-1; (l) BC-28-2; (m) BC-28-3; (n) BC-28-4; (o) BC-28-5; (p) BC-28-6; (q) SMA-20-2; (r)
 2 SMA-20-3; (s) SMA-20-4

3 In Fig. 10, most of the specimens exhibit an approximately symmetrical distribution of the carbonyl index.
 4 A quadratic function appears to fit the data very well. The coefficient of the second-degree terms in the
 5 quadratic function controls the degree of curvature of the graph. Steeper curvature corresponds to greater
 6 aging gradient along the mixture specimen, indicating the capacity of the mixture against oxygen diffusion
 7 (or aging resistant capacity). Therefore, the coefficient of the second-degree term in the quadratic function
 8 is designated as Δ_{aging} to represent the aging gradient of the asphalt mixture specimens. Δ_{aging} indicates
 9 the aging susceptibility of an asphalt mixture. The surface of a mixture sample can be more easily oxidized
 10 because it is close to the atmosphere. If a mixture has low aging susceptibility, the inner part of the mixture
 11 would be better “protected” away from oxidation. Conversely, a mixture with high aging susceptibility
 12 would age more in the inner part. In the extreme case, the most aging-susceptible mixture would age
 13 uniformly throughout the mixture. Therefore, Δ_{aging} is a good indicator to represent the aging
 14 susceptibility of the mixture.

15 Fig. 11 shows the relationship between Δ_{aging} and the AV content. As shown in the figure, for asphalt
 16 mixture samples of the same gradation, the aging gradient decreases with the increase of AV content. When
 17 AV content exceeds a certain value, however, the aging difference in different layers of the mixture
 18 specimens is much reduced. For example, specimen BC-28-6 has the highest AV content among the BC-
 19 28 specimens, and the aging gradient in BC-28-6 is negligible. However, for different asphalt mixtures,
 20 there is no direct relationship between Δ_{aging} and the AV content. For instance, SMA-20-2 has the same
 21 AV content as BC-28-5, but the aging gradient of SMA-20-2 is much greater than that of BC-28-5.

22



23

24

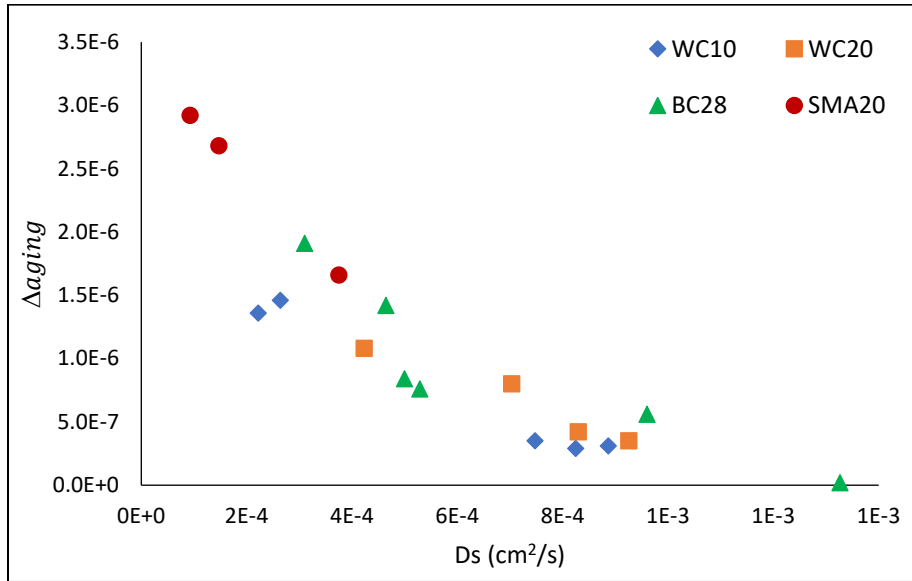
Figure 11. Δ_{aging} and the AV content.

25

26 The relationship between Δ_{aging} and D_s is shown in Fig. 12. Clearly, as compared to air voids, D_s is more
 27 closely related to Δ_{aging} , which decreases with the increase of D_s . The overall trend can be well captured
 28 by a log-linear model, as shown in Fig. 13. The high R^2 value indicates the relationship between Δ_{aging}
 29 and D_s can be universally applied to different mixture types, unlike the relationship between Δ_{aging} and

1 AV content. Therefore, D_s may serve as a valuable and convenient index to evaluate and predict the aging
 2 gradient of an asphalt mixture, which is related to the mixture's aging susceptibility. The well-fit
 3 relationship between Δ_{aging} and D_s is not surprising, as D_s fundamentally determines the efficiency of
 4 oxygen transport into inner asphalt mixtures.

5

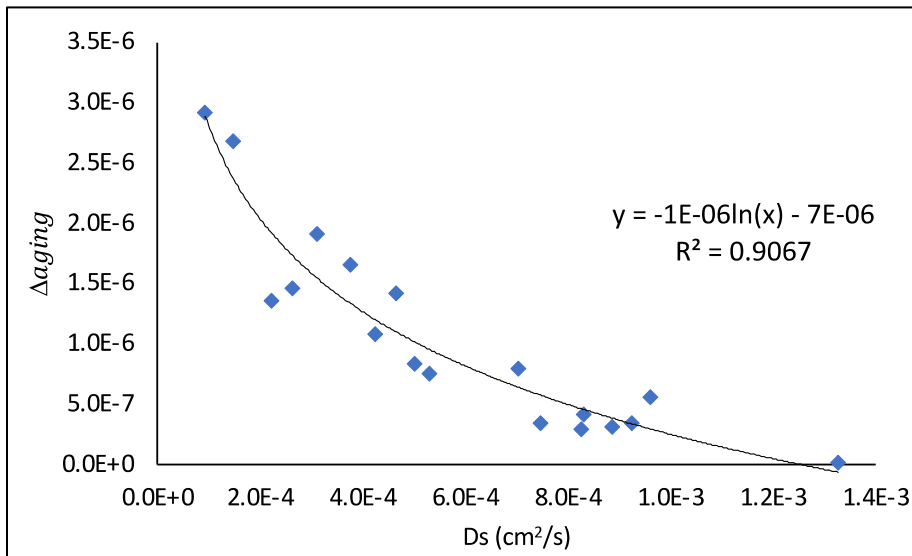


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7

Figure 12. Δ_{aging} and the D_s

8



9

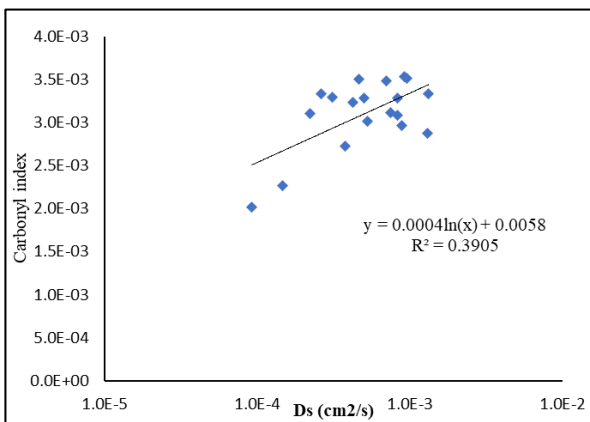
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Figure 13. Regression results for Δ_{aging} and D_s

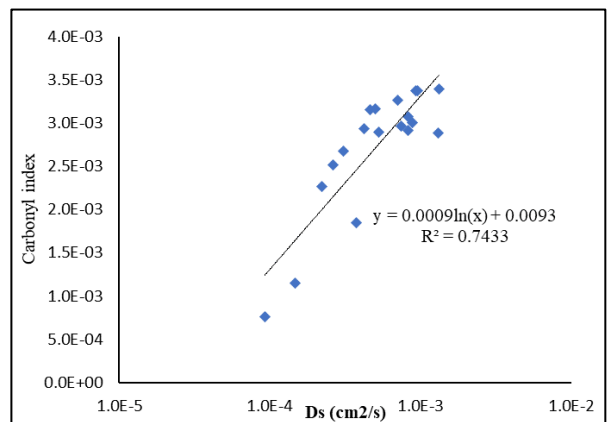
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12 4.3 D_s and the aging severity of asphalt mixtures

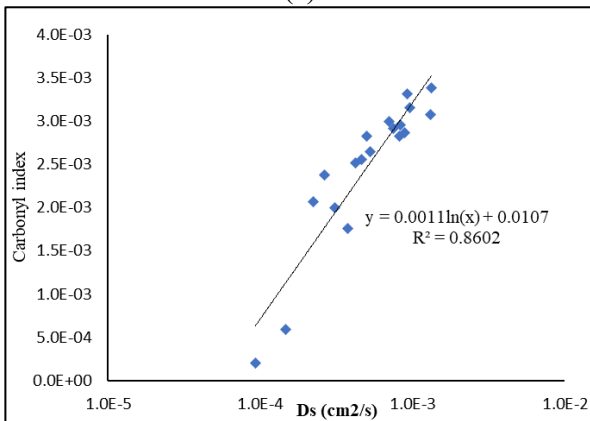
1 Figure 14 (a)~(e) shows the relationships between D_s and the aging severity of asphalt binders extracted
 2 from the different layers of the mixture samples. Note that the thickness of each layer is 10 mm except for
 3 the middle layer, which is 20 mm. The figure indicates that correlations between D_s and aging severity for
 4 binders from layer-two, layer-three, and layer-four range are quite strong, but the correlations are not as
 5 strong for layer-one and layer-five. This is reasonable because the top and bottom layers have close contact
 6 with oxygen in the air, while oxidation in the middle layers highly depends on the efficiency of oxygen
 7 transport of the mixtures. This further proves that the oxygen transportation efficiency can be well
 8 characterized by D_s . The closer the asphalt binder approaches the middle layer of the compacted mixtures,
 9 the higher the correlations between D_s and binder aging severity. Moreover, the aging of asphalt even 10
 10 mm beneath the surface is significantly affected by D_s . To exclude the influences of the surface (both top
 11 and bottom) layers, the average carbonyl index for binders from layer-two, layer-three, layer-four is
 12 calculated and used to represent the overall aging severity of the mixtures. The relationship between D_s and
 13 the overall aging severity is shown in Figure 14 (f). The high R^2 value implies that D_s is indeed appropriate
 14 to characterize the overall aging susceptibility of asphalt mixtures.



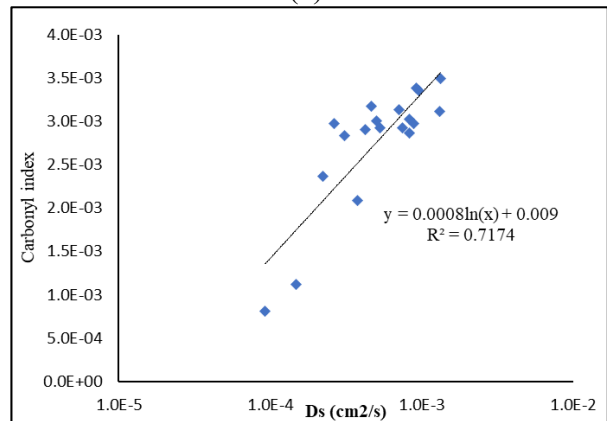
(a)



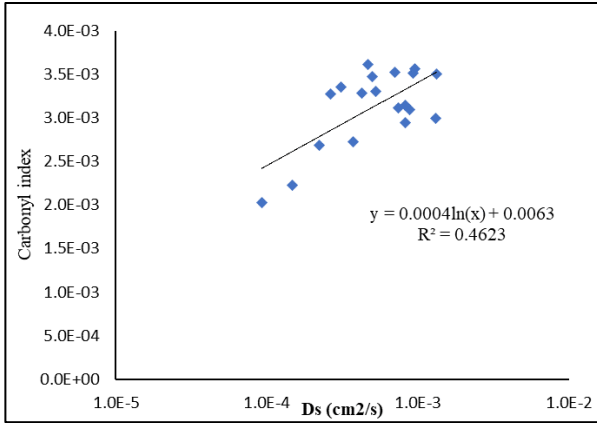
(b)



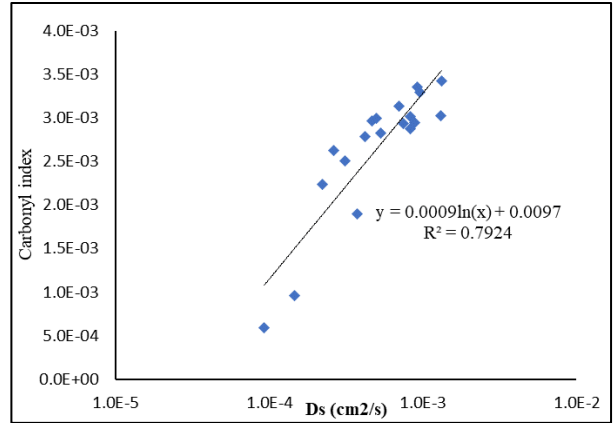
(c)



(d)



(e)

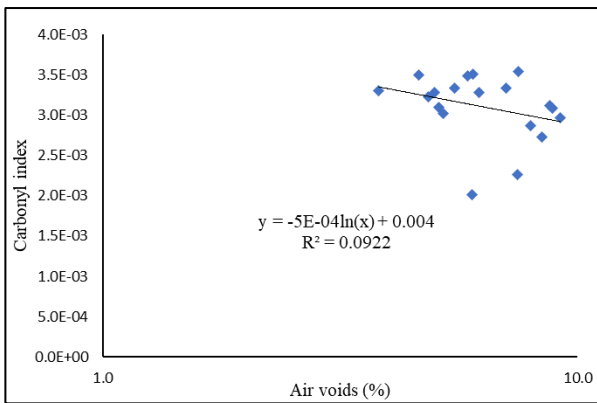


(f)

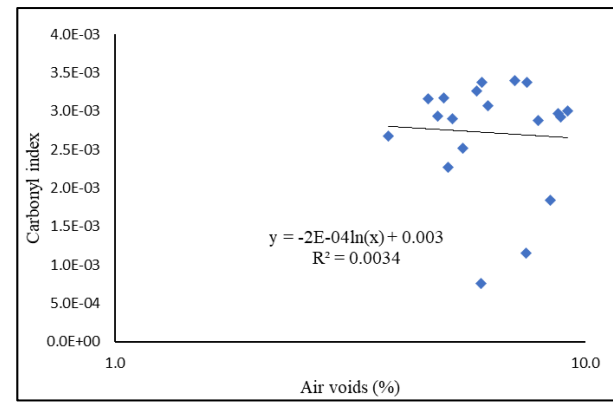
1 **Figure 14.** D_s and aging severity in different layers: (a) layer-one; (b) layer-two; (c) layer-three; (d) layer-
 2 four; (e) layer-five; (f) D_s and average aging severity for layer-two, layer-three, and layer-four

3 For comparison purposes, Figure 15 shows the effect of AVs on the aging of asphalt mixtures. Small R^2
 4 values in Fig. 15 indicate that there are no direct relationships between AVs and aging for asphalt mixtures.

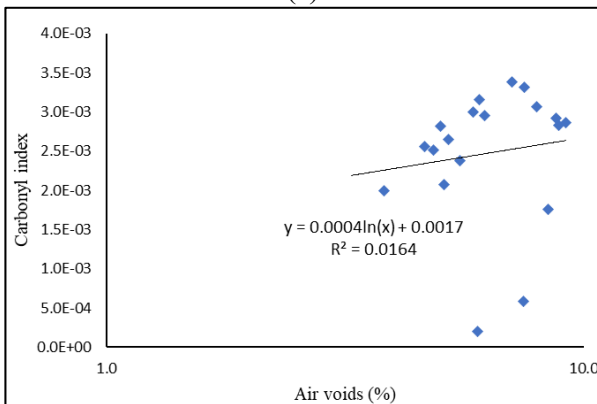
5 Therefore, D_s is a more suitable parameter than AVs to characterize the aging of asphalt mixtures.



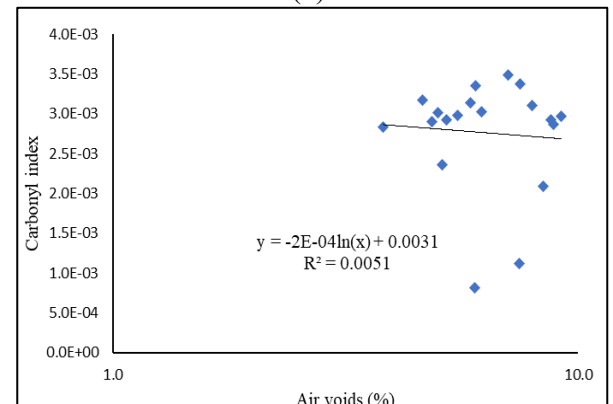
(a)



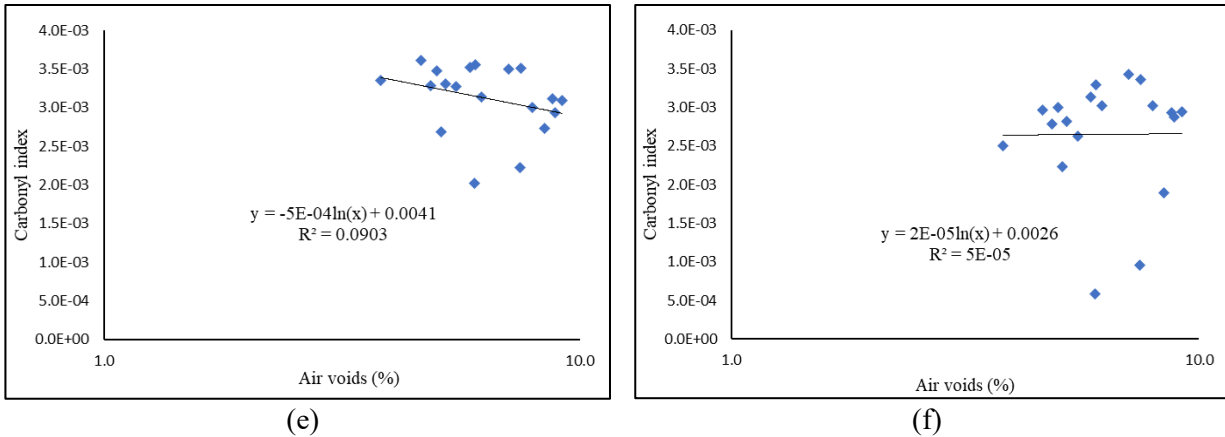
(b)



(c)



(d)



1 **Figure 15.** AVs and the aging severity in different layers: (a) layer-one; (b) layer-two; (c) layer-three; (d)
 2 layer-four; (e) layer-five; (f) D_s and average aging severity for layer-two, layer-three, and layer-four

3

4 **5. Conclusion and Recommendation**

5 An upgraded diffusion test apparatus was developed to determine the diffusion coefficient (D_s) of asphalt
 6 mixtures with different gradations and air voids (AVs). The same asphalt mixture samples were
 7 subsequently subjected to one-dimensional long-term aging treatments. Asphalt binders from different
 8 slices of the aged specimens were extracted and recovered, and their aging states were determined using
 9 FTIR. Influences of D_s and AV content on binder aging gradients in the asphalt mixture were systematically
 10 examined. The following conclusions are drawn from the test and analysis results.

- 11 (1) The D_s values of asphalt mixtures are influenced by mixtures' volumetric characteristics including
 12 gradation, AV content, and the nominal maximum aggregate size. It is a comprehensive indicator
 13 of the oxygen transportation efficiency of compacted asphalt mixtures.
- 14 (2) The majority of the asphalt mixture samples subjected to aging treatment demonstrate a similar
 15 pattern of the aging gradient along the depth profile. Asphalt binders in the top and bottom surface
 16 layers of the mixture samples are more severely aged than those in the middle layers.
- 17 (3) For each mixture type, the aging gradients of asphalt binders in the mixture samples are closely
 18 related to AV contents. Such a relationship, however, is specific to mixture type. On the other hand,
 19 the D_s values are closely and universally related to the aging gradients of all asphalt mixture types.
- 20 (4) The aging extent of asphalt binder in the inner portion of an asphalt mixture is closely related to
 21 the D_s value of the mixture. In view of the close relationships between D_s and aging gradient and
 22 between D_s and aging severity of asphalt mixtures, the D_s value of an asphalt mixture may serve
 23 as a useful surrogate to assess the aging susceptibility of the asphalt mixture. The determination of
 24 D_s does not demand expensive equipment and complicated test procedure. Therefore, it can serve
 25 as a promising parameter in designing more aging-resistant mixtures, modeling and predicting field
 26 pavement aging.

27 Since testing the D_s of asphalt mixture to evaluate its aging susceptibility is newly proposed, additional
 28 studies may be performed in the future to gain more understanding on this parameter. In particular, the
 29 following research directions are recommended:

- 30 (1) The rheological properties of the extracted asphalt binders at different depth may be further tested
 31 to evaluate the influences of aging difference on binder rheology. This will facilitate easier
 32 interpretation on the test results and better connection to industry practices.

- 1 (2) Asphalt binder aging is also dependent on binder type. The aging susceptibility of an asphalt binder
2 is governed by the oxidation kinetics of the specific binder. The relative impacts of binder's aging
3 susceptibility and D_s may be evaluated and compared.
- 4 (3) In this study, only air voids content is used as the independent variable to model D_s for each type
5 of mixture of a same gradation. Other volumetric properties such as the VMA and VFA may be
6 also considered in modeling D_s ."
- 7 (4) The concept and method of determining D_s may be applied to the preventative maintenance of
8 asphalt pavements. Impeding oxygen diffusion is one of the strategies in preventative maintenance
9 to retard pavement aging. Since D_s reflects the oxygen transport efficiency of asphalt mixtures, it
10 may be used to evaluate the effectiveness of preventative maintenance measures on reducing
11 oxygen diffusion.

12

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18

19 Author Contributions

20 The authors confirm contribution to the papers as follows: study conception and design: Yuhong Wang;
21 data collection: Xingyu Chen; analysis and interpretation of results: Xingyu Chen, Yong Wen; draft
22 manuscript preparation: Xingyu Chen, Yuhong Wang, Huailei Cheng, Gengren Hao. All authors reviewed
23 the results and approved the final version of the manuscript.

24

25 Declaration of Conflicting Interests

26 The authors declared no potential conflicts of interest with respect to the research, authorship, and/or
27 publication of this article.

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