

39 **Abstract**

40 Seawater and sea-sand based ultra-high performance concrete (UHPC) is currently regarded as one of
41 the most innovative and promising construction materials. To provide design guidance for the usage of
42 silica fume (SF) during the production of seawater and sea-sand based UHPC, this work investigated
43 the effect of SF on the later-age macro-performances and micro/nanostructure of seawater ordinary
44 Portland cement (OPC) pastes prepared with a low water-to-binder (W/B) ratio. The results showed
45 that the optimal SF dosage in seawater OPC-SF system was 10 wt.% while that for the deionized (DI)
46 water OPC-SF system was 20 wt.%. This showed that the usage of SF could be reduced for the
47 production of the seawater based UHPC system. It was found that the later-age compressive strength of
48 seawater OPC-SF system was increasingly degraded with increasing SF dosage when compared with
49 the corresponding DI water OPC-SF system. The difference in the distribution of unhydrated clinkers
50 in the matrix as SF addition increased was the main cause. The seawater OPC-10%SF system exhibited
51 the highest compressive strength and the lowest pore content among all the seawater-OPC systems.
52 Additionally, with the usage of this ultra-low W/B ratio, Friedel's salt was only present in the seawater
53 OPC pastes prepared without SF, but absent in the seawater OPC-SF pastes.

54 **Key words:** Seawater UHPC; Silica fume; Compressive strength; Micro/nanostructure

55

56 **1. Introduction**

57 Seawater sea-sand concrete (SSC), as an innovative and sustainable construction material, has
58 attracted increasing research interest worldwide [1-3], especially for coastal cities and remote
59 islands [4]. The shortages of freshwater and river-sand for concrete production have posed
60 challenges to the concrete industry, and thus a novel concept of using seawater and sea-sand
61 instead of freshwater and river-sand together with the use of non-metallic reinforcement (e.g.,

62 fibre reinforced polymer) has been proposed [2, 5, 6]. Some SSC based infrastructures have been
63 successfully applied, for instance, in London's Olympic Stadium, and the Great Belt Bridge
64 located between Denmark and Sweden [3].

65

66 The macro-performance of SSC has been extensively studied in previous research. The usage of
67 seawater would improve the early-age compressive strength of hardened OPC pastes, mortar or
68 concrete [1, 7, 8]. However, the findings regarding the effect of seawater on the later-age
69 compressive strength remain controversial. Kaushik et al. [9] and Younis et al. [10] found that the
70 usage of seawater could slightly lower the later-age compressive strength, while Li et al. [8]
71 reported a similar compressive strength in seawater mixed specimens at 28 days compared with
72 the freshwater mixed specimen. Another study conducted by Wang et al. [5] observed that
73 seawater increased the later-age compressive strength. Generally, most findings have observed a
74 slightly lower long-term compressive strength of SSC compared to ordinary concrete [7, 9-12].
75 Additionally, the usage of seawater reduced the setting time and the initial slump flow, influencing
76 the workability of fresh cement pastes [9, 10]. Regarding the durability of SSC, Nishida et al. [13]
77 stated that the plain concrete structures using seawater as mixing water showed good durability.
78 Younis et al. [10] reported that the usage of seawater slightly increased shrinkage but had little
79 effect on the permeability and Cl^- ion resistance.

80

81 Concerning the microstructure of SSC, the accelerated hydration rates of OPC have been widely
82 observed when concrete was mixed with seawater [5, 7, 8, 14-16]. Wang et al. reported that
83 seawater OPC pastes produced about 1.12 times the cumulative heat generated from deionized

84 water (DI water) OPC pastes during the hydration of 3 days [5]. However, Sikora et al. [15],
85 Montanari et al. [16] and Younis et al. [10] found that the total amount of hydration heat at a
86 medium or later age in seawater OPC pastes was comparable to that in DI water OPC pastes. A
87 similar accelerating effect by seawater can also be seen in the hydration of C_3S [14]. In addition,
88 our previous work suggested that the effect of seawater on accelerating cement hydration was
89 more significant at a lower water-to-binder (W/B) ratio [7]. Previous research also examined the
90 mechanism concerning the accelerating effect of seawater. Edwards et al. demonstrated that the
91 Ca^{2+} , Na^+ , Mg^{2+} , Cl^- and SO_4^{2-} ions in seawater accelerated the hydration of C_3S [17]. Li et al.
92 suggested that the Cl^- ion in seawater was the main active ion promoting the precipitation and
93 resulting in the accelerating effect [8]. In addition, using seawater as the mixing water in concrete
94 has also modified the composition of hydration products in OPC pastes. Friedel's salt was
95 observed as the new hydration product in seawater OPC pastes due to the reaction between
96 monosulfate phase and Cl^- ion [7, 8]. Wang et al. stated that the formation of Friedel's salt can bind
97 40% of chloride of the seawater [5]. However, Montanari et al. observed that only ~0.47 wt.%
98 Friedel's salt was present in the seawater OPC paste, and most Cl^- ions were absorbed in the
99 C-S-H gel [16].

100
101 The accelerating effect of seawater at an early age provides a potential for the production of
102 seawater and sea-sand based ultra-high performance concrete (UHPC). UHPC, as an advanced
103 cement-based material, has a significantly high mechanical strength (typical compressive strength
104 of over 150 MPa and tensile strength of over 20 MPa) and excellent durability [18-20]. Generally,
105 UHPC comprises cement, fine supplementary cementitious materials, quartz sand, fibers, water at

106 a very low W/B ratio, and superplasticizers [21, 22]. Due to the use of a high dosage of
107 superplasticizer to assure workability at a low W/B, the early hydration and strength development
108 can be retarded. The usage of seawater can improve the retarded hydration and the early-age
109 mechanical strength of UHPC. Teng et al. produced seawater and sea-sand UHPC at a W/B ratio
110 of 0.18, which was prepared with a 52.5N OPC, silica fume (SF), quartz powder and
111 superplasticizer. The UHPC had a compressive strength of over 180 MPa [4]. Shi et al. examined
112 the effect of seawater on the early-age (3 days) hydration of cement with the incorporation of slag
113 and SF at a W/B of 0.2, and showed that seawater increased the early-age compressive strength of
114 the specimens, modified the interaction of slag and SF, and decreased the reactivity of SF [23].
115 However, few studies have investigated the influence of seawater on the later-age (≥ 28 days)
116 macro-performance and microstructure of OPC with SF at low W/B. Moreover, the research
117 regarding seawater OPC-SF pastes prepared with a very low W/B ratio is limited. Furthermore,
118 the evolution of macro-performances and microstructure of seawater OPC pastes with different SF
119 addition need to be investigated, which is beneficial to provide some recommendations for the
120 appropriate SF content in seawater and sea-sand based UHPC.

121

122 This work presents an experimental study on the influence of SF on the later-age compressive
123 strength, impedance modulus and micro/nanostructure of seawater OPC pastes prepared with an
124 ultra-low W/B ratio. The evolutions of macro-performances, hydration process and hydration
125 products of a cement paste prepared with a W/B ratio of 0.15 and different SF replacements
126 ranging from 0 wt.% to 30 wt.% **of the total binders** were investigated. The aim was to identify the
127 effect of seawater on the most optimal dosage of SF in the OPC-SF system.

128 2. Materials and experimental procedure

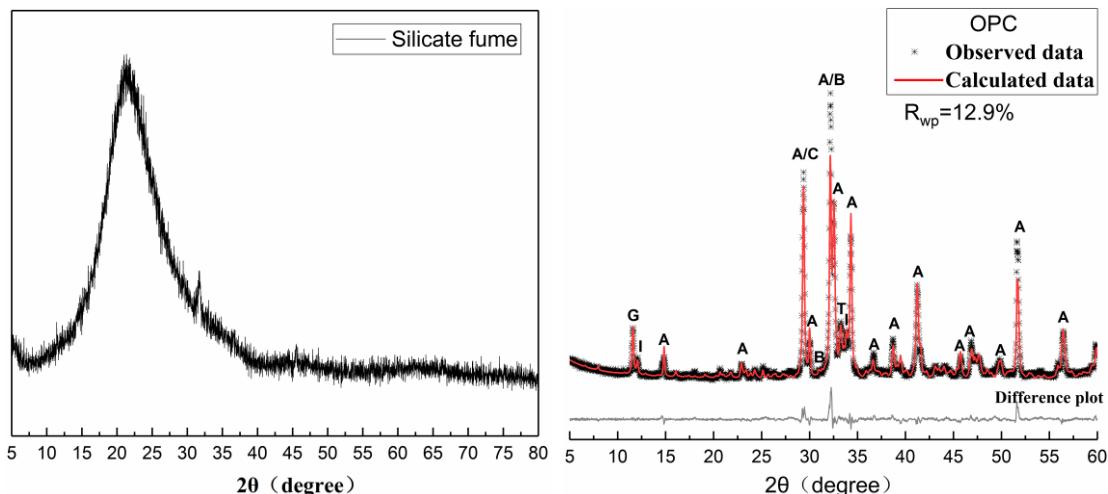
129 2.1 Materials

130 In this study, an ASTM Type I OPC and a SF produced by Elkem (China) were used. Table 1
 131 shows the chemical composition of SF and OPC, tested by X-ray fluorescence spectroscopy
 132 (Supermin200, Rangaku Corporation, Japan). Fig. 1 shows the XRD results of SF and OPC. The
 133 SF was amorphous, and the composition of OPC was 66.4 wt.% of C₃S, 11.9 wt.% of C₂S, 6.0
 134 wt.% of C₃A, 7.2 wt.% of C₄AF, 5.8 wt.% of CaCO₃ and 2.7 wt.% of gypsum. Table 2 shows the
 135 ICSD codes used for the Rietveld analysis. The polycarboxylate-based superplasticizer (with a
 136 solid content of 20%) produced by BASF was used. The artificial seawater was prepared
 137 according to ASTM D1141-98 (2013), as shown in Table 3.

138 **Table 1** Chemical composition of SF and OPC (wt%)

| Phase | CaO | SiO ₂ | Al ₂ O ₃ | Fe ₂ O ₃ | SO ₃ | MgO | K ₂ O | TiO ₂ | P ₂ O ₅ | ZnO | MnO | CuO | Cl | SrO | ZrO ₂ |
|-------|-------|------------------|--------------------------------|--------------------------------|-----------------|------|------------------|------------------|-------------------------------|------|------|------|------|------|------------------|
| SF | 0.73 | 95.7 | 0.45 | 0.07 | 0.27 | 0.71 | 1.74 | - | 0.09 | - | 0.03 | - | 0.22 | - | - |
| OPC | 66.10 | 20.00 | 5.13 | 3.24 | 2.99 | 1.09 | 0.60 | 0.27 | 0.24 | 0.10 | 0.08 | 0.05 | 0.04 | 0.04 | 0.01 |

139 -: not detected.



140
 141 **Fig. 1** XRD curve of SF and Rietveld quantitative analysis for OPC. Legend: A: C₃S; B: C₂S; T: C₃A; I: C₄AF; C:
 142 CaCO₃; G: gypsum.

143

144

Table 2 Phase compositions and ICSD codes used for quantitative analysis.

| Phase | ICSD Codes |
|--------------------------------|------------|
| C ₃ S | 4331 |
| C ₂ S | 81096 |
| C ₃ A | 1841 |
| C ₄ AF | 9197 |
| C _{SH} ₂ | 36186 |
| CaCO ₃ | 18165 |
| CH | 34241 |
| AFt | 155395 |
| Friedel's salt | 62363 |
| Al ₂ O ₃ | 10425 |

145

146

Table 3 Compositions of the artificial seawater (g/L)

| Type | NaCl | MgCl ₂ | Na ₂ SO ₄ | CaCl ₂ | KCl | NaHCO ₃ | KBr |
|---------------|-------|-------------------|---------------------------------|-------------------|-------|--------------------|-------|
| Concentration | 24.53 | 5.20 | 4.09 | 1.16 | 0.695 | 0.201 | 0.101 |

147

148 2.2 Specimen preparation

149 In order to investigate the effect of SF on the seawater OPC pastes prepared at a low W/B ratio,

150 different contents of SF, a W/B ratio of 0.15 and a superplasticizer dosage of 3 wt% **of the total**

151 **binders** were used to produce the specimens. The pastes were mixed with DI and seawater,

152 respectively. Table 4 shows the detailed mix proportions.

153

154 Based on the modified Andreasen & Andersen model [24], the grading curves of the different

155 cement paste mixes and the target curve were obtained to theoretically investigate the evolution of

156 the pore structures, shown in Fig. 2. The target curve was developed by using the Least Squares

157 Method to adjust the contents of OPC and SF and optimize the packing [24]. The most

158 theoretically favorable particle distribution among the mixes was observed when 20 wt% SF was

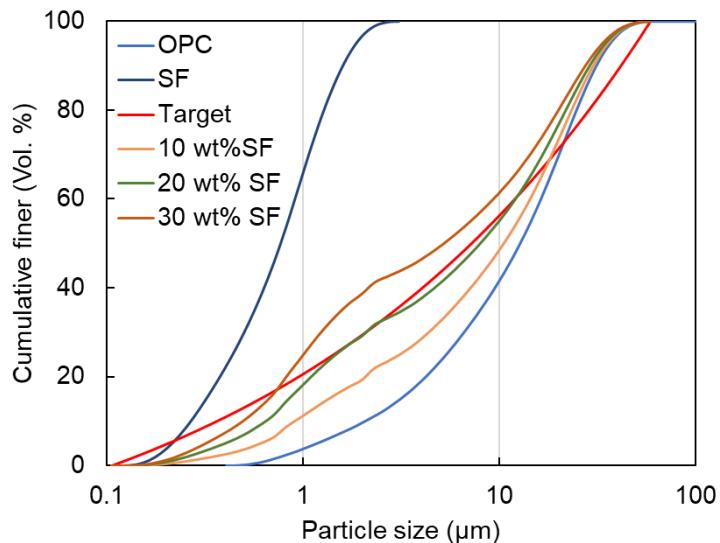
159 incorporated.

160

161 **Table 4** Mixture design of different pastes

| Sample | D0 | D10 | D20 | D30 | S0 | S10 | S20 | S30 |
|--------------|----------|----------|----------|----------|----------|----------|----------|----------|
| OPC (wt%) | 100 | 90 | 80 | 70 | 100 | 90 | 80 | 70 |
| SF (wt%) | 0 | 10 | 20 | 30 | 0 | 10 | 20 | 30 |
| Mixing water | DI water | DI water | DI water | DI water | seawater | seawater | seawater | seawater |

162



163

164 **Fig. 2** Particle size distributions of SF, OPC, target grading curve, and the grading curves with different SF
165 contents.

166

167 The cementitious materials (i.e., OPC and SF) were dry mixed for 1 min at a low speed before
168 adding DI water or seawater and the superplasticizer. The pastes were then mixed for 1 min at low
169 speed and 5 min at high speed. The well-mixed pastes were cast into 40 mm × 40 mm × 40 mm
170 steel cube molds, and vibrated on a mechanical vibration table for 1 min. Then, the samples were
171 covered with plastic films and cured at 23 ± 2 °C for 24 h. After demolding, the samples were
172 cured at 23 ± 2 °C and >95% relative humidity until testing. **Fig. 3** shows the picture of paste

173 specimens. The compressive strength test was then performed at the testing ages with a loading
174 rate of 0.96 kN/s. The broken samples after the compressive strength test were collected and
175 grounded into powder or small particles, and then immersed in isopropanol and vacuum dried for
176 microstructure analysis [25].

177



178

Fig. 3 Picture of paste specimens

180

181 2.3 Microstructure methods

182 Electrochemical impedance spectroscopy (EIS) was conducted using a Multi-AutolabM 204
183 instrument where its frequency ranged from 10 mHz to 1 MHz and its AC signal magnitude was
184 10 mV. The samples were dried with a towel, covered by two thin wet sponges, and immediately
185 measured using a two-electrode system. The resulting impedance spectra were analyzed by Nova
186 1.11 software.

187

188 X-ray diffraction (XRD) curves were acquired using a Bruker D8 Advance X-Ray Powder
189 Diffractometer (Germany) equipped with CuKa12 radiation ($\lambda_1 = 0.15406$ nm, $\lambda_2 = 0.15444$ nm)
190 under the conditions of 40 kV tube voltage and 40 mA tube current. The measurement was set
191 with the step size at 0.020° , the rate of 0.5 s per step and the 2θ ranging from 5.0° to 80.0° . The

192 overall measurement time was about 30 min to ensure the favorable signal-to-noise ratio. TOPAS
193 5.0 software was used for quantifying each mineral content of OPC and hydration products. To
194 prepare the samples for the experiment, the hydrated powder was well mixed with 10 wt%
195 α -Al₂O₃ (Aladdin, 99.99% purity) as an internal standard to calculate **the content of amorphous**
196 **(Acn) phases and crystalline phases**. The overall refined parameters included background
197 coefficients, zero-shift error, phase scale factors, Chebyshev polynomial correction, Lorentz
198 polarization factor, crystal structures and cell parameters. The ICSD codes used for quantitative
199 analysis were shown in Table 2.

200

201 Thermogravimetric (TG) curves were recorded using a Mettler Toledo TGA/DSC1 instrument.
202 The heating procedure was set from 30°C to 1000°C at a constant heating rate of 10°C/min. 10 ±
203 0.5 mg test powder was put into a corundum crucible and heated under N₂ atmosphere.

204

205 Nitrogen adsorption and desorption curves were acquired using an ASAP 2020 Micromeritics
206 Accelerated Surface Area and Porosimetry instrument. The test powder was firstly degassed and
207 preheated at 60 °C and at a pressure of 0.1 mmHg for 24 h. The bath temperature for analysis was
208 -196 °C. The pore volume distribution was obtained based on Barrett-Joyner-Halenda (BJH)
209 analysis.

210

211 Mercury intrusion porosimeter (MIP) analysis was conducted using a Micromeritics AutoPore IV
212 9500 Series instrument where the maximum mercury intrusion pressure was 207 MPa. The
213 specimens were broken into small particles with a size of ~5 mm and soaked in isopropanol as an

214 exchange solvent. Finally, the specimens were dried in a vacuum desiccator for 7d before the MIP
215 analysis.

216

217 ^{29}Si magic angle spinning nuclear magnetic resonance (^{29}Si MAS-NMR) curves were recorded
218 using a GEOL 500 MHz spectrometer with a 7-mm CP/MAS probe. The rotation rate and
219 resonance frequency for the measurement were set at 4500 Hz and 79.5 MHz, respectively. The
220 relaxation delay was 30 s and over 2000 scan was completed for each ^{29}Si MAS-NMR spectrum.

221

222 Backscattered electron (BSE) images were performed by a Tescan VEGA3 instrument with an
223 accelerating voltage of 20 kV. Firstly, the dried samples were mounted in the low-viscosity epoxy
224 resin (EPO-TEK 301) under a vacuum pressure of 10 mbar, ensuring that the air was removed and
225 the epoxy resin was infiltrated well. After the epoxy was solidified (~24h), the mounted specimens
226 were polished by a grinding & polishing equipment (Buehler AutoMet 300)/ the details of the
227 procedures were modified according to the previous work [26], and isopropanol was used at the
228 first step. The polished samples were coated with carbon prior to BSE measurements.

229

230 Nanoindentation was performed using a Hysitron Triboindenter 950 system (TI 950 TriboIndenter,
231 Bruker) equipped with a Berkovich diamond indenter probe. Firstly, the surface roughness of the
232 chosen area was determined to ensure it was lower than 70 nm for nanoindentation measurement
233 [27]. For the analysis, a 15×15 grid with a 10 μm interval was chosen, and therefore 225
234 indentations in total were determined. The peak load was set at 2000 μN , with a period of 10 s
235 loading time, 5s holding time, and 10s unloading time. The elastic modulus (Er) of each

236 indentation was determined based on the previous guidelines [28].

237

238 **3. Results**

239 **3.1 Compressive strength**

240 Fig. 4 shows the compressive strength of seawater and DI water OPC pastes with different SF
241 contents. Compared to DI water OPC pastes, the seawater OPC pastes with each SF addition
242 showed a higher compressive strength at 1 day due to the presence of active ions in the seawater.

243 The higher early-age compressive strength of seawater OPC pastes without SF was also confirmed
244 by previous works [1, 5, 7, 8]. In addition, the compressive strength of seawater was significantly
245 improved at an early age when the used SF content was 30 wt%. However, the compressive
246 strength of seawater OPC pastes was lower at 7 and 28 days compared to DI water OPC pastes.

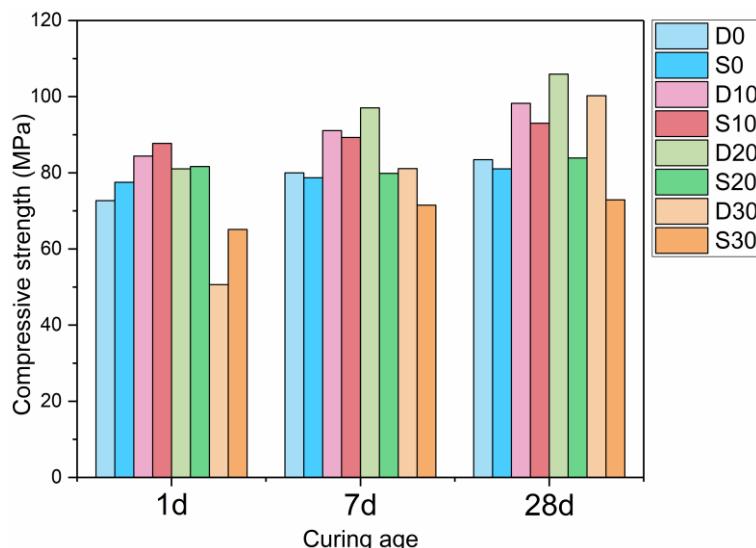
247 Furthermore, the decreasing rate in compressive strength was much higher as the increase in SF
248 content, i.e., 2.9% (0 wt% SF), 5.3% (10 wt% SF), 20.8% (20 wt% SF), 27.3% (30 wt% SF), and
249 thus the addition of SF would further decrease the later-age compressive strength of the seawater
250 OPC pastes.

251

252 In addition, the compressive strength evolution of DI water OPC pastes and seawater OPC pastes
253 with different SF contents was determined. In Fig. 4, the compressive strength of DI water and
254 seawater group increased firstly and then decreased with increasing SF contents. The highest
255 compressive strength (~105 MPa) of DI water OPC pastes was observed when incorporating 20
256 wt% SF, while the seawater OPC pastes with 10 wt% SF showed the highest compressive strength
257 (~98 MPa). Furthermore, with respect to the OPC-based construction materials used an ultra-low

258 W/B, the usage of seawater can reduce production costs as the seawater OPC-SF system with a
259 low SF addition would have the highest macro-strength, saving SF dosage.

260



261

Fig. 4 Compressive strength of different specimens at 1, 7 and 28 days

262

263

264

265 3.2 EIS measurements

266 Fig. 5 shows the results of the electrochemical impedance test of seawater and DI water OPC
267 pastes with different SF contents. The impedance modulus changes with the change of ion
268 transport properties in concrete. The ion transport rate would be slower as the impedance modulus
269 increases, and thus ions in the external environment would be harder to enter the interior of the
270 specimen, suggesting that the specimen would exhibit a favorable durability. In both DI water
271 pastes and seawater pastes, the addition of SF decreased the impedance modulus at 1day, and the
272 impedance modulus was further decreased with increasing SF content. However, after curing for 7
273 or 28 days, the impedance modulus of the specimens with SF was higher compared to that without
274 SF. In addition, the impedance modulus significantly increased with SF content or with prolonged
275 curing age, corresponding to the improvement in external ions corrosion resistance.

277 The comparison of impedance modulus of seawater OPC pastes and DI water OPC pastes is
 278 shown in Fig. 6. Specimens without SF in DI water and seawater group showed a similar
 279 impedance modulus. The scenario was different when incorporating SF. The impedance modulus
 280 of S10 and S20 were higher than that of D10 and D20, respectively. However, S30 had a lower
 281 impedance modulus compared to D30. Therefore, seawater can increase the impedance modulus
 282 when 10 wt.% and 20 wt.% SF were used, indicating that the durability of those two seawater
 283 OPC-SF systems was improved compared with the corresponding DI water systems.

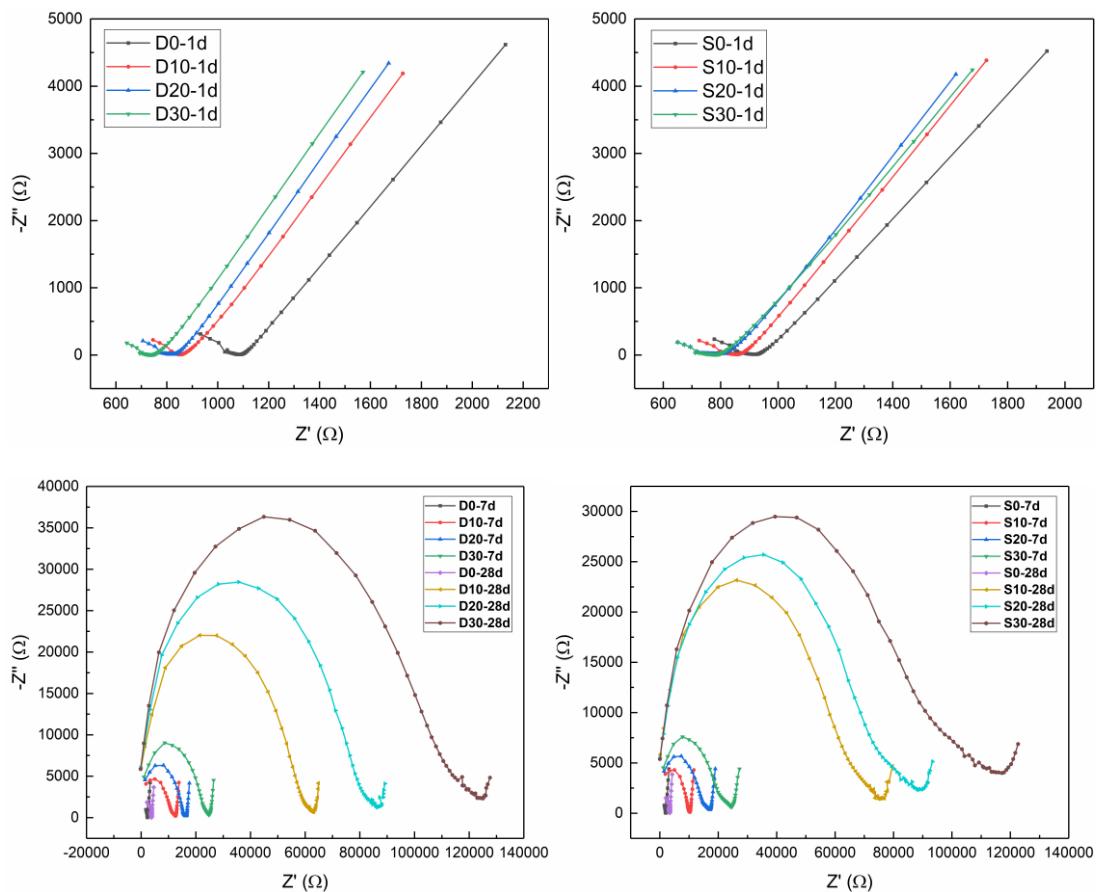
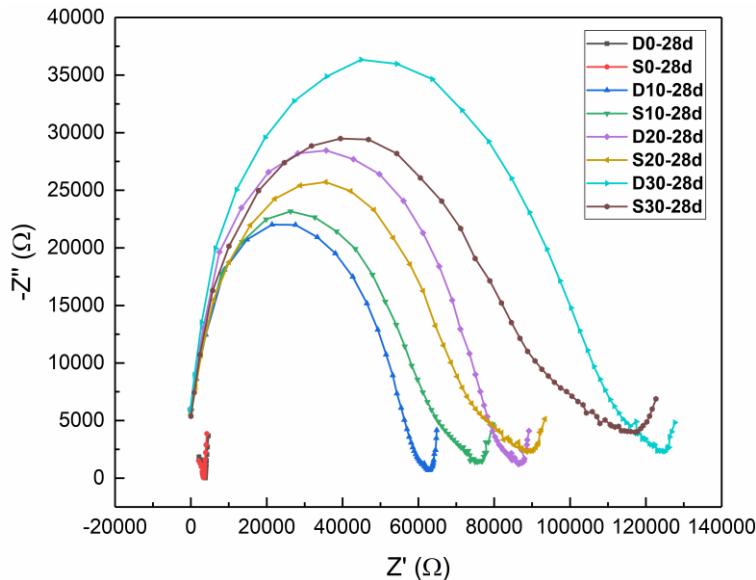


Fig. 5 EIS results of different samples at 1, 7 and 28 days



291

292

Fig. 6 Comparison of seawater and DI water OPC pastes at 28 days

293

294 3.3 Compositions and contents of hydration products

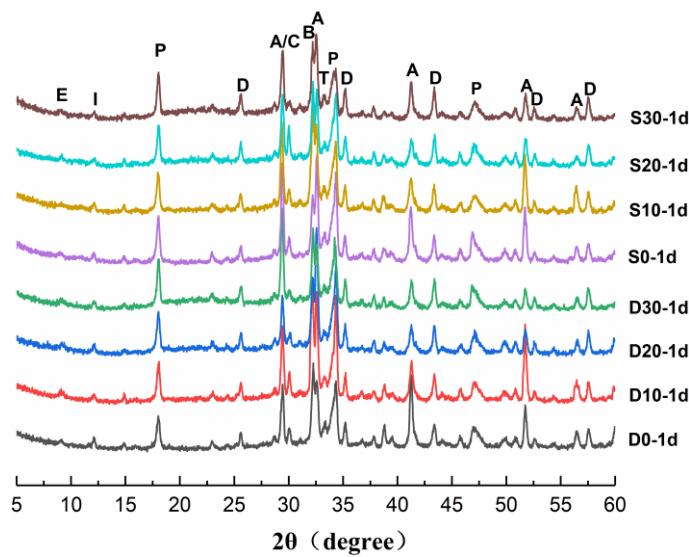
295 The compositions of hydration products were analysed by XRD using EVA software. Fig. 7 shows
 296 the XRD patterns of seawater OPC pastes and DI water OPC pastes at 1, 7 and 28 days. The
 297 unhydrated clinkers (C_3S , C_2S , C_3A , C_4AF and $CaCO_3$) could be clearly observed as their peaks
 298 were easily identified at the ultra-low W/B. The XRD peaks of main crystalline hydration products
 299 (i.e., AFt and CH) were identified in both seawater and DI water OPC pastes, which was also
 300 confirmed by the TGA-DTG curves in Fig. 8. Due to the presence of Cl^- in seawater, Friedel's salt
 301 ($Ca_4Al_2(OH)_{12}Cl_2 \cdot 4H_2O$), as a Cl^- -containing crystalline product, was obviously detected in S0
 302 sample at 7 or 28 days, which was consistent with previous works [5, 29, 30]. Gypsum peaks were
 303 not detected at both the early or later age and this might indicate that gypsum was easily dissolved
 304 and reacted with C_3A and Cl^- to form AFt and Friedel's salt.

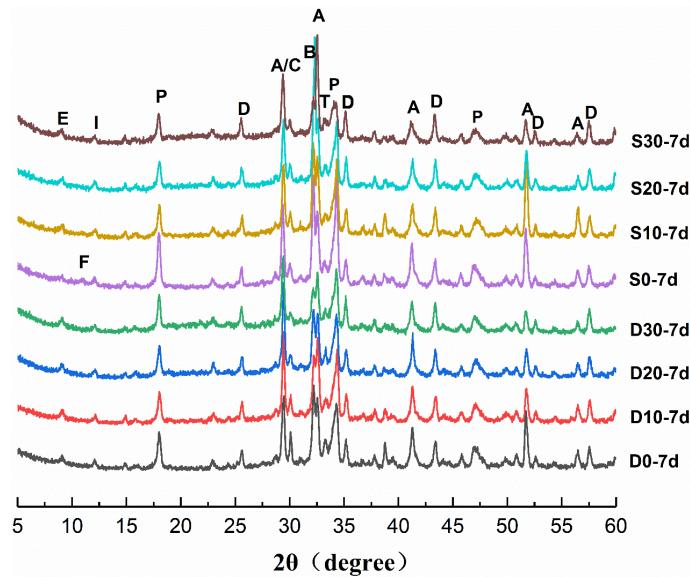
305

306

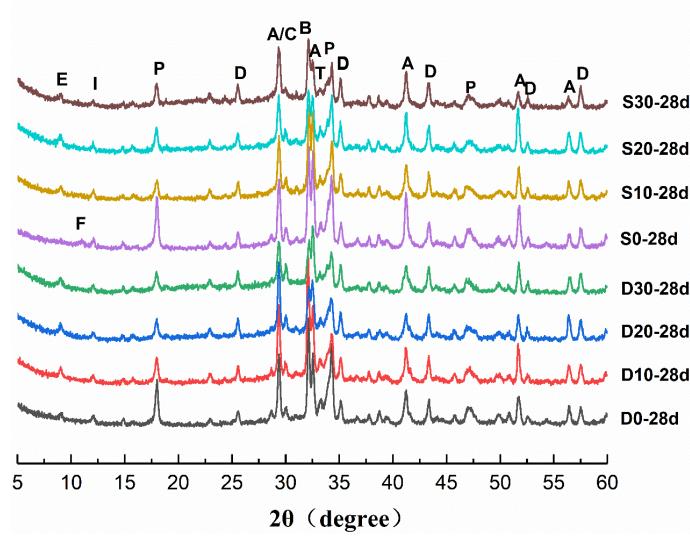
307 The incorporation of SF influenced the compositions of hydrated seawater OPC pastes. As shown
308 in Fig. 7, Friedel's salt was hardly detected at 7 or 28 days when SF was used, and can only be
309 found in the S0 sample. Previous works suggested that Cl^- ion can be physically absorbed in the
310 structure of C-S-H gel [16, 31-34], and Cl^- tended to attach to C-S-H gel due to high absorbability
311 instead of participating in the formation of Friedel's salt [16]. This might be due to with the
312 presence of SF, more C-S-H gel with a low Ca/Si ratio was formed. This type of C-S-H gel and
313 SiO_2 gel exhibited a high specific surface area for physical absorption of Cl^- ion, and thus less free
314 Cl^- ion participated in the formation of Friedel's salt.

315
316





318



319

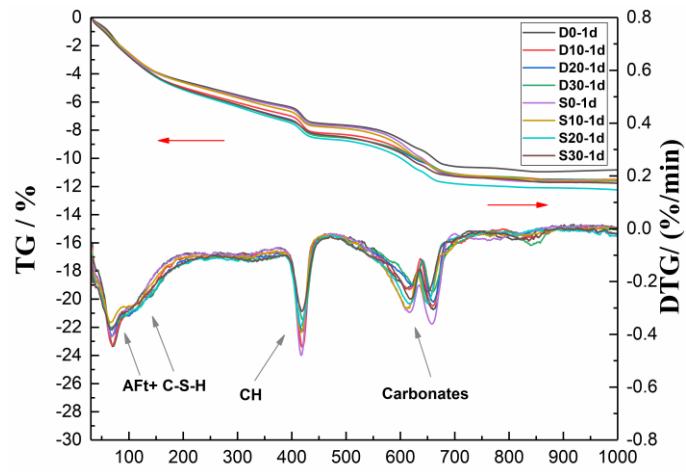
320

Fig. 7 XRD patterns of samples at 1, 7 and 28 days.

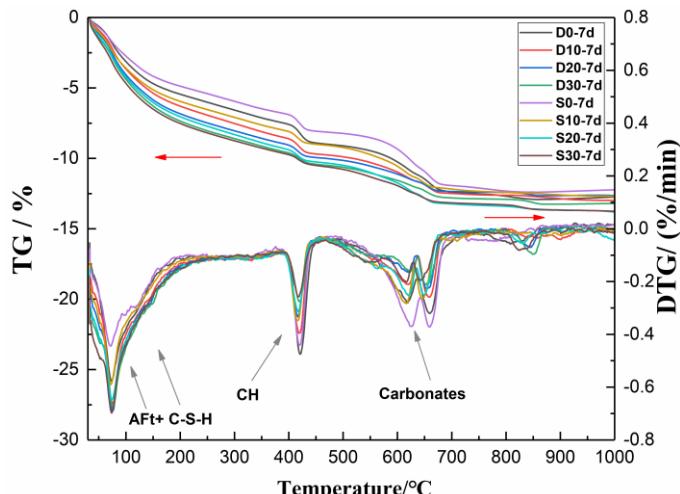
321

Legend: E: ettringite (AFt); F: Friedel's salt; P: CH; A: C_3S ; B: C_2S ; T: C_3A ; I: C_4AF ; C: Calcite; D: Al_2O_3 .

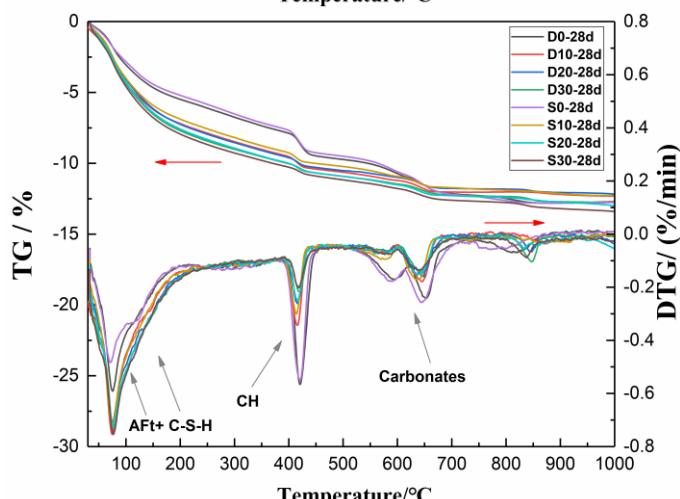
322



323



324



325

326

Fig. 8 TG-DTG curves of different samples at 1, 7 and 28 days

327

328 The contents of unhydrated clinkers and hydration products were determined by the Rietveld
 329 method. Table 5 shows the quantitative XRD results of the samples at 1 and 28 days. All R_{wp}
 330 (weighted-profile R value) values were lower than 15%, corresponding to the good accuracy of
 331 quantitative results. Additionally, the Rietveld plots of D30 and S30 samples at 28 days, as two
 332 typical examples, were shown in Fig. 9. Firstly, seawater accelerated the early-age hydration rate
 333 of OPC-SF system. The hydration degree of C_3S (the principal early-hydration clinker phase) was
 334 increased at 1 day in all the seawater OPC-SF systems. The higher CH content (8.6 wt%) at 1 day
 335 in S0 specimen (Table 5) and the higher mass loss of CH (Fig. 8) in S0 supported the observation.
 336 In addition, the seawater OPC pastes prepared with SF incorporation exhibited a lower CH content

337 at 1 day compared to the DI water OPC pastes with SF shown in Table 5, indicating that seawater
338 also accelerated the pozzolanic reaction between CH and SF at an early age. However, the
339 accelerating effect of seawater was not obvious in the OPC pastes prepared without SF or with
340 only 10 wt% SF at a later age. The C₃S content and the mass loss of CH at 28 days were similar in
341 S0 and D0, and S10 and D10 samples (Table 5). However, S30 showed a lower C₃S content and
342 higher Acn content compared to D30, indicating that the acceleration effect of seawater was still
343 observed when 30 wt% SF was used. **The acceleration effect of seawater mainly occurred at an**
344 **early age, while the cumulative heat release of the seawater mixed paste was comparable to that of**
345 **the freshwater mixed paste after 7 days [5, 16, 35, 36]. Also, the low W/B used in the UHPC might**
346 **have also reduced the acceleration effect at a later age. But this study showed that seawater**
347 **accelerated the pozzolanic reaction, and this acceleration effect might be more obvious with a**
348 **higher SF dosage. Thus, the acceleration effect of seawater was still found in S30 sample at a later**
349 **age.**

350

351 The change of Friedel's salt content was further quantitatively determined in the seawater OPC-SF
352 system (Table 5). Friedel's salt was absent in all OPC-SF systems at 1 day, while it was present in
353 the S0 specimen at 28 days. However, the AFt content was significantly increased in the seawater
354 OPC pastes prepared with SF. The above observation was consistent with the fact that C-S-H gel
355 with a low Ca/Si ratio and SiO₂ gel would absorb Cl⁻ ion, and the aluminate ions (Al(OH)₄⁻) would
356 react with sulfate ions (SO₄²⁻), forming more AFt.

357

358

Table 5 Quantitative XRD results at 1 and 28 days. (wt%)

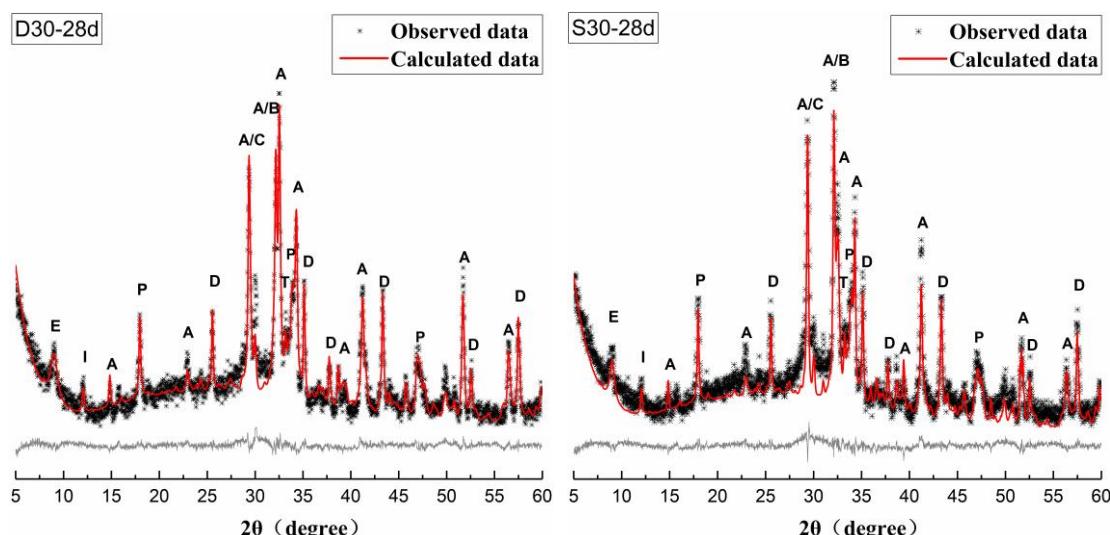
| Sample | C ₃ S | C ₂ S | C ₃ A | C ₄ AF | Calcite | CH | AFt | F salt ^a | Acn | R _{wp} | Clinkers ^b | Hydrates ^c |
|---------|------------------|------------------|------------------|-------------------|---------|-----|-----|---------------------|------|-----------------|-----------------------|-----------------------|
| D0-1d | 39.2 | 12.5 | 3.8 | 4.9 | 4.9 | 7.3 | 2.9 | - | 24.5 | 11.0 | 65.3 | 34.7 |
| D10-1d | 36.3 | 11.2 | 2.9 | 5.4 | 4.4 | 6.6 | 2.5 | - | 30.7 | 10.3 | 60.2 | 39.8 |
| D20-1d | 22.4 | 12.7 | 2.2 | 5.5 | 3.9 | 5.4 | 2.4 | - | 45.5 | 10.6 | 46.7 | 53.3 |
| D30-1d | 21.0 | 10.5 | 2.5 | 3.4 | 3.7 | 4.6 | 2.1 | - | 52.2 | 10.1 | 41.1 | 58.9 |
| S0-1d | 33.1 | 11.1 | 3.7 | 4.9 | 5.4 | 8.6 | 2.4 | - | 30.8 | 11.1 | 58.2 | 41.8 |
| S10-1d | 32.7 | 11.3 | 3.0 | 4.2 | 4.1 | 5.4 | 1.0 | - | 38.3 | 10.0 | 55.3 | 44.7 |
| S20-1d | 20.3 | 11.4 | 1.3 | 5.1 | 4.2 | 5.2 | 1.9 | - | 50.6 | 10.6 | 42.3 | 57.7 |
| S30-1d | 16.0 | 10.6 | 1.8 | 4.7 | 3.5 | 3.5 | 2.1 | - | 57.8 | 10.1 | 36.6 | 63.4 |
| D0-28d | 27.9 | 12.1 | 2.9 | 6.5 | 4.7 | 7.8 | 2.1 | - | 36.0 | 9.9 | 54.1 | 45.9 |
| D10-28d | 26.8 | 11.1 | 2.5 | 4.9 | 4.5 | 4.3 | 4.0 | - | 41.9 | 10.2 | 49.8 | 50.2 |
| D20-28d | 20.4 | 10.3 | 1.8 | 4.0 | 4.2 | 3.4 | 3.3 | - | 52.6 | 10.5 | 40.7 | 59.3 |
| D30-28d | 20.7 | 8.9 | 1.4 | 3.8 | 4.2 | 2.0 | 3.4 | - | 55.6 | 11.3 | 39.0 | 61.0 |
| S0-28d | 28.0 | 11.2 | 2.3 | 6.5 | 4.8 | 8.4 | 1.0 | 2.0 | 35.8 | 10.4 | 52.8 | 47.2 |
| S10-28d | 26.3 | 11.2 | 1.4 | 5.1 | 4.2 | 3.9 | 3.2 | - | 44.7 | 10.5 | 48.2 | 51.8 |
| S20-28d | 17.7 | 10.2 | 2.4 | 3.9 | 3.6 | 3.3 | 3.6 | - | 55.3 | 10.9 | 37.8 | 62.2 |
| S30-28d | 15.1 | 8.3 | 2.0 | 4.1 | 3.5 | 1.7 | 4.3 | - | 61.0 | 11.2 | 33.0 | 67.0 |

360 -: not present.

361 ^a: Friedel's salt362 ^b: content of total unhydrated clinkers363 ^c: content of total hydration products

364

365



366

367 **Fig. 9** Rietveld quantitative analysis for D30-28d and S30-28d. Legend: E: ettringite (AFt); P: CH; A: C₃S; B: C₂S;

368

T: C₃A; I: C₄AF; C: Calcite; D: Al₂O₃.

369

370 3.4 ^{29}Si MAS-NMR measurements

371 The nanostructures of C-S-H gel and SiO_2 gel were analyzed using the ^{29}Si MAS-NMR spectra.
372 The usage of seawater and SF changed the structure of the C-S-H gel. Fig. 10 shows the ^{29}Si
373 MAS-NMR spectra of seawater and DI water OPC pastes prepared without SF and with 30 wt.%
374 SF at 28 days, including the raw experimental, deconvoluted, and different Q^n (where n ranges
375 from 0 to 4) structural unit curves. Q^n represents different Si-O tetrahedron structures, and n
376 denotes the number of oxygen bridges. Generally, the ^{29}Si MAS-NMR spectra exhibit five signals
377 of Q^n Si-O tetrahedron structures, including (a) Q^0 located at -68 to -75 ppm that represents the
378 uncondensed silicate group (monomer), (b) Q^1 located at -76 to -82 ppm that represents the
379 chain-end silicate group of C-S-H gel, (c) Q^2 located at -82 to -88 ppm that represents the
380 chain-middle silicate group of C-S-H gel, (d) Q^3 located at -88 to -98 ppm that represents the
381 layers and chain branching sites of the silicate group of amorphous SiO_2 gel, and (e) Q^4 located at
382 -98 to -120 ppm that represents the three-dimensional networks layers of the silicate group of
383 amorphous SiO_2 gel [37-39].

384

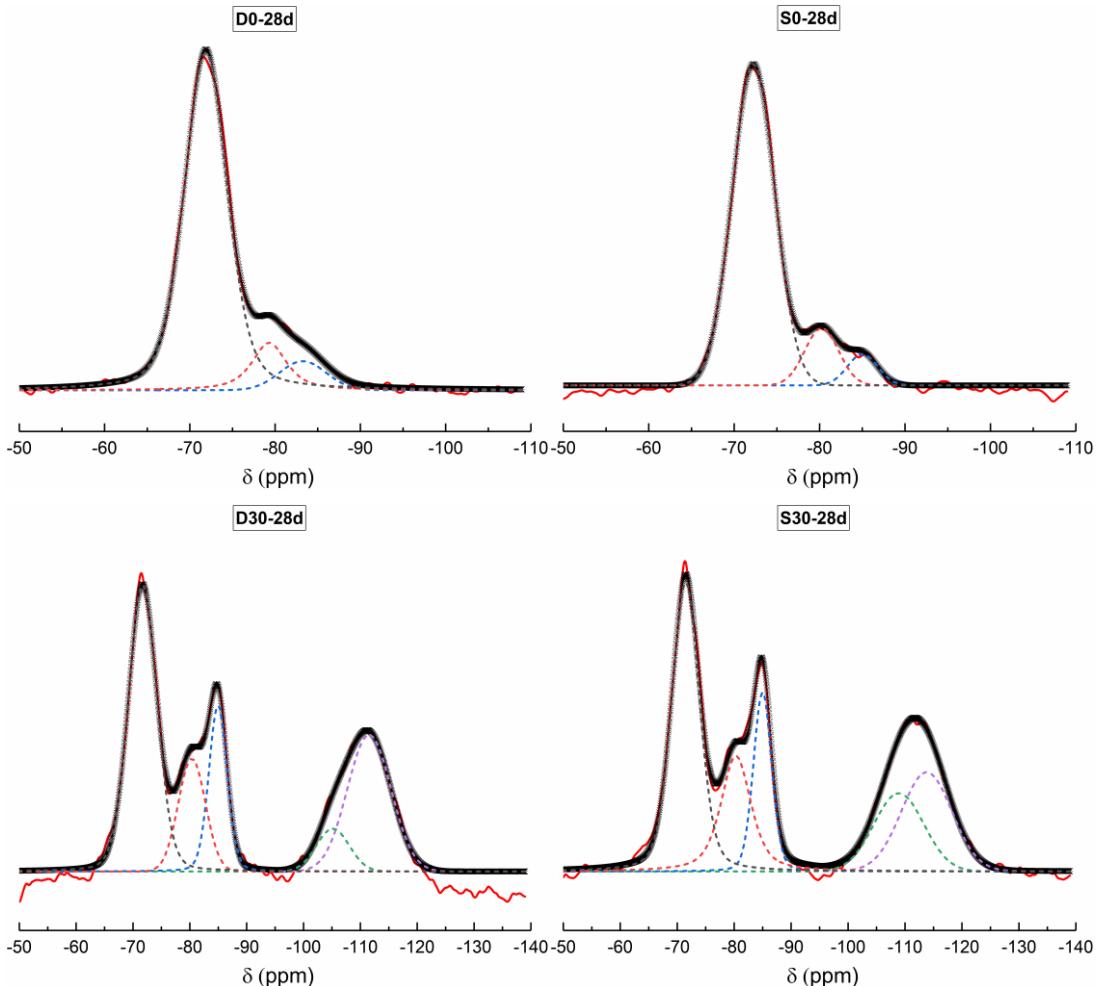
385 The deconvoluted curves were derived from the Gauss and Lorentz type component convolutions
386 using Peakfit 4.12 software, shown in Fig. 10. Table 6 summarizes Q^n distributions, and calculated
387 polymerization degree, mean molecular chain length and fraction of vacant tetrahedral sites of the
388 C-S-H gel. In Fig. 10, the D0 and S0 samples exhibited three ^{29}Si MAS-NMR signals (viz. Q^0 , Q^1
389 and Q^2), and Q^3 or Q^4 signals were not present, further confirming that no supplementary
390 cementitious materials were present in this OPC paste. Also, Q^0 , the signal for siliceous clinkers,
391 accounted for over 80%, because of the low hydration degree at this low W/B. S0 sample had a

392 similar percentage of Q^0 compared to D0 sample, suggesting the similar hydration degrees of the
393 two samples. S30 exhibited an evidently lower percentage of Q^0 compared to D30 sample,
394 demonstrating a higher hydration degree than D30 sample. The sum of Q^1 and Q^2 , as two
395 characteristic signals of C-S-H gel, further confirmed the change of hydration degree in the
396 seawater and DI water OPC pastes, which was also consistent with the Rietveld quantitative
397 analysis results (Table 5).

398

399 The structure of C-S-H gel changed with the usage of SF and seawater as shown in Table 6. In
400 both seawater OPC pastes and DI water OPC pastes, the polymerization degree and mean
401 molecular chain length of C-S-H gel increased with the presence of SF, and an opposite trend was
402 observed regarding the fraction of vacant tetrahedral sites of C-S-H gel. The observation was
403 attributed to the formation of the C-S-H gel with a low Ca/Si ratio by the reaction of SF and CH,
404 and this C-S-H gel had a higher polymerization degree and a longer molecular chain length. In
405 addition, the polymerization degree and mean molecular chain length of the C-S-H gel in the
406 seawater OPC and OPC-30%SF systems were lower than those in DI water OPC and OPC-30%SF
407 systems, indicating that the usage of seawater changed the Ca/Si ratio of C-S-H. The difference in
408 the C-S-H structure was due to the presence of sodium ions in seawater, which influenced the
409 C-S-H structure. Sodium ions substituted the calcium ions in C-S-H structure and were
410 incorporated on the C-S-H surface or in the C-S-H interlayer. Thus, the C-S-H structure was
411 reorganized, resulting in a decrease of mean molecular chain length [7, 40].

412



415 **Fig. 10** ^{29}Si MAS-NMR spectra of different hydrated pastes at 28 days.

416

417 **Table 6** Percentages of Q^0 , Q^1 , Q^2 , Q^3 and Q^4 resonances and characterization parameters of C-S-H structure

| Sample | $\text{Q}^0(\%)$ | $\text{Q}^1(\%)$ | $\text{Q}^2(\%)$ | $\text{Q}^3(\%)$ | $\text{Q}^4(\%)$ | PD ^a | MCL ^b | ν^c |
|---------|------------------|------------------|------------------|------------------|------------------|-----------------|------------------|---------|
| D0-28d | 82.42 | 10.90 | 6.68 | - | - | 0.61 | 3.22 | 0.24 |
| S0-28d | 81.60 | 12.71 | 5.69 | - | - | 0.45 | 2.90 | 0.26 |
| D30-28d | 39.08 | 13.62 | 14.58 | 6.58 | 26.44 | 1.07 | 4.14 | 0.19 |
| S30-28d | 34.73 | 16.91 | 13.62 | 15.02 | 19.72 | 0.81 | 3.62 | 0.22 |

418 -: not present.

419 ^a Polymerization degree of C-S-H, $\text{PD} = \text{Q}^2/\text{Q}^1$. [37]

420 ^b Mean molecular chain length of C-S-H, $\text{MCL} = 2(\text{Q}^1 + \text{Q}^2)/\text{Q}^1$. [37, 38]

421 ^c Fraction of vacant tetrahedral sites, $\nu = 1/(\text{MCL} + 1)$. [41]

422

423 3.5 Pore structure

424 Nitrogen adsorption and desorption measurements were used for the analysis of the pore structure

425 of the cement pastes ranging from 2 nm to 20 nm. MIP measurements were used for analyzing the

426 pore structure ranging from 20 nm to 1000 nm. Fig. 11 shows the BJH adsorption pore volume
427 and dV/dD pore volume of D0, S0, D30 and S30 samples at 28 days. S0 sample exhibited a
428 smaller nanopore size between 2 nm and 20 nm compared to D0 sample since seawater
429 accelerated the hydration reaction. However, S30 sample shows a similar but slightly smaller pore
430 size distribution compared to D30 sample. Thus, the effect of seawater on the nanopore size was
431 reduced when SF was used.

432

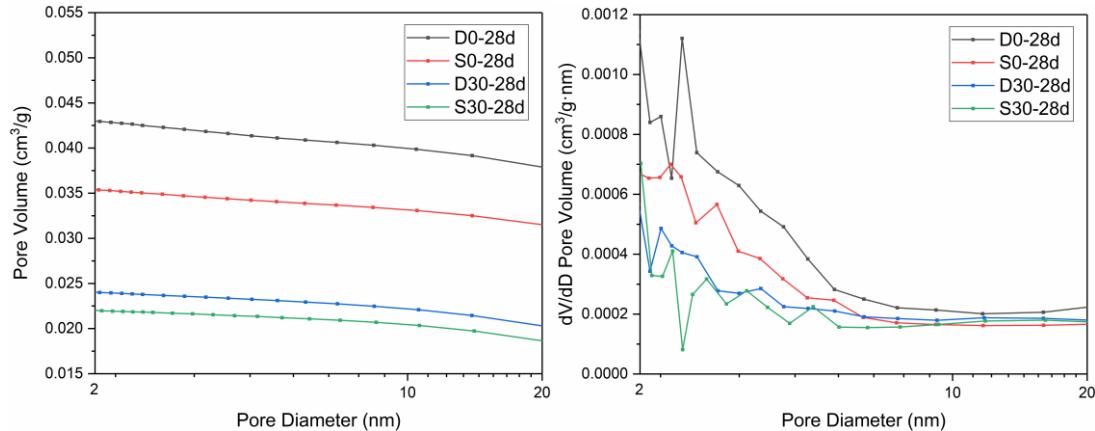
433 Fig. 12 shows the MIP results of D0, S0, D30 and S30 samples at 28 days. It is obvious that the
434 cumulative pore **area** with a pore diameter of 35 nm or more in S0 sample was invariably higher
435 than that of D0 sample, but the cumulative pore **area** with a pore diameter between 20 nm - 35 nm
436 of D0 sample was substantially increased, and was higher than that of the S0 sample. Additionally,
437 the major pore size fraction of D0 was located at ~40 nm, which was smaller compared with S0
438 sample (its peak was located at ~45 nm). Also, the average pore diameter and median pore
439 diameter of S0 sample were 53.7 nm and 49.1 nm, respectively, which were higher than those of
440 the D0 sample with an average pore diameter of 43 nm and a median pore diameter of 41.8 nm.
441 With the usage of 30 wt% SF, the seawater OPC pastes and DI water OPC pastes exhibited a very
442 fine pore structure, e.g., the total cumulative pore areas of D30 and S30 samples were extremely
443 low (less than 0.5 m³/g).

444

445 Combining the results of nitrogen adsorption and desorption measurements and MIP provided the
446 information of the pore structure in the size range of 2 nm – 1000 nm. The usage of seawater
447 decreased the amount of pore between 2 nm and 20 nm, but increased the content of pore ranging

448 from 20 nm to 1000 nm when SF was not used. It was due to the accelerated hydration of seawater,
 449 leading to pore coarsening (decrease in finer pores and increase in larger pores). This resulted in
 450 an inferior pore size distribution. With SF addition, the effect of seawater on the pore structure
 451 was weakened, and both seawater OPC pastes and DI water OPC pastes exhibited better pore
 452 structures.

453

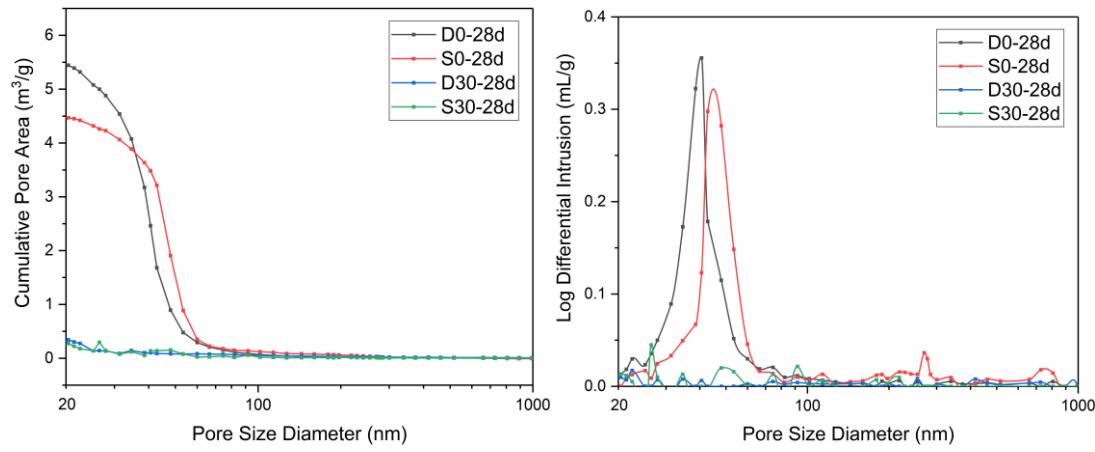


454

455 **Fig. 11** BJH adsorption curves of different hydrated pastes at 28 days.

456

457



458

459 **Fig. 12** MIP curves of different hydrated pastes at 28 days.

460

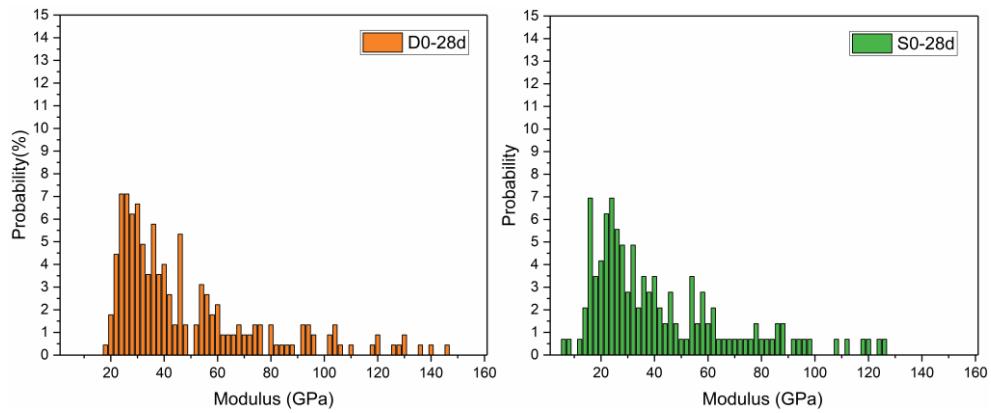
461 3.6 Nanoindentation

462 Nanoindentation analysis was conducted to analyze the micromechanical properties [42-47]. Fig.

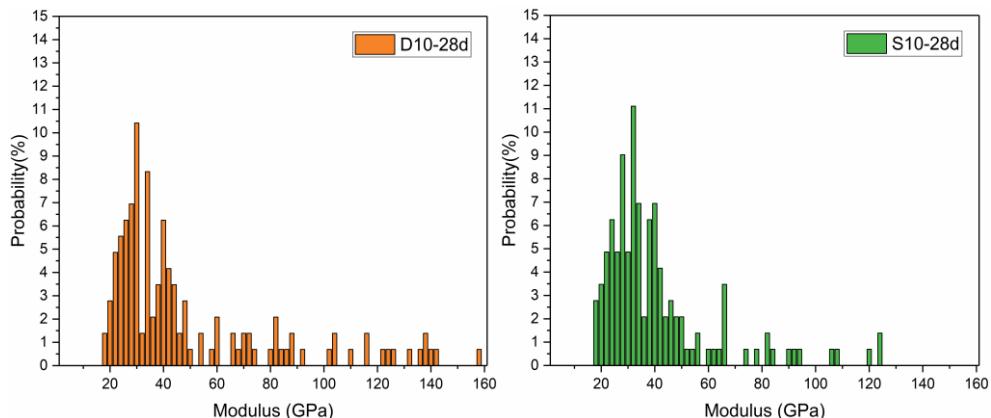
463 13 shows the statistical distribution of indentation modulus in the seawater and DI water OPC-SF
464 systems at 28 days. For OPC pastes with a low W/B, four major phases, namely, LD (low density)
465 C-S-H, HD (high density) C-S-H, UHD (ultra-high density) C-S-H and unhydrated clinkers, can
466 be identified. Generally, the LD C-S-H and HD C-S-H exhibit their intrinsic elastic moduli, which
467 are not dependent on the mix proportions of the specimens [42]. LD C-S-H has been known as an
468 inner product of cement clinker hydration [44], a low stiffness C-S-H [45], or equivalent spheres
469 with random packing (C-S-H globules) [43], with the modulus value ranging from about 17.5 GPa
470 to 27.5 GPa [47]. HD C-S-H has been considered as an outer product [44], a medium stiffness
471 C-S-H [45], or equivalent spheres with more ordered packing [43], with the modulus value ranging
472 from about 27.5 GPa to 33.3 GPa [47]. UHD C-S-H has been reported as a composite of HD
473 C-S-H and nano-CH present in the gel pores of C-S-H [42, 43] with the modulus ranging from
474 about 33.3 GPa to 48.6 GPa [47] and exhibits a relatively high scatter in micromechanical
475 properties due to the CH crystals with various sizes and the possible presence of C₄AF clinkers
476 [43]. Unhydrated cement clinkers exhibits a significantly scattered modulus data (i.e., 48.6 GPa
477 and above [47]) owing to the various sizes of clinker crystals.

478
479 The results in Fig. 13 show the seawater OPC pastes and DI water OPC pastes prepared with
480 different SF contents exhibited different distributions of LD C-S-H, HD C-S-H, UHD C-S-H and
481 unhydrated clinkers.

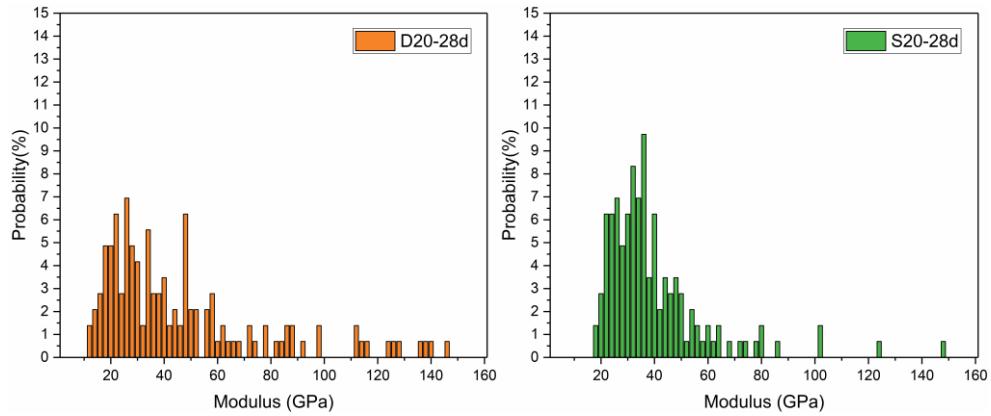
482



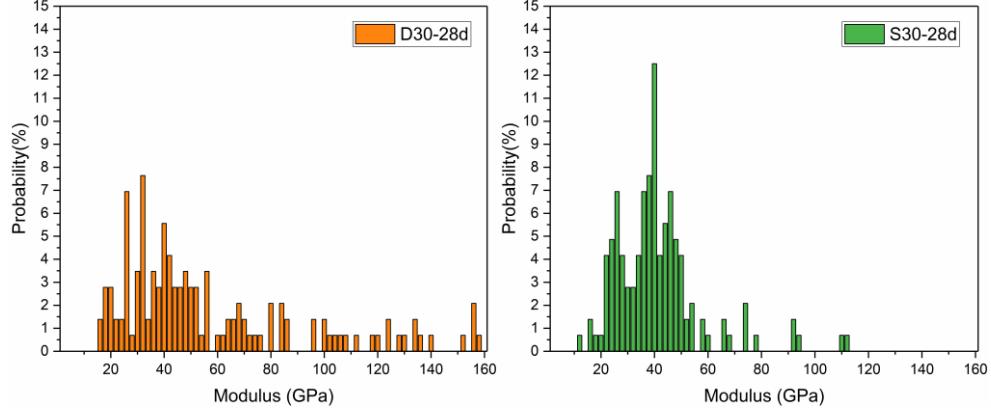
483



484



485



486

487

488

489

Fig. 13 Statistical distribution of indentation modulus for different hydrated pastes at 28 days.

490 4. Discussion

491

492 The usage of SF changed the influence of seawater on macro-performances and microstructure of
493 OPC pastes prepared with an ultra-low W/B. On the one hand, the compressive strength of
494 seawater OPC-SF system with an ultra-low W/B was increasingly degraded at 28 days as the SF
495 content increased, compared to the DI water OPC-SF system with the same SF addition. The
496 macro-mechanical strength of specimens was closely related to the microstructural evolution.

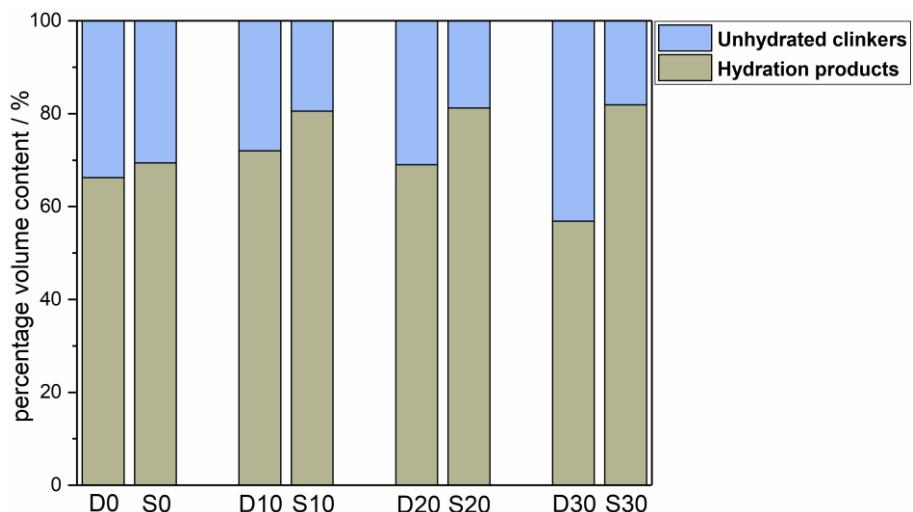
497 Firstly, the pore structure was not the key factor resulting in the increasing degradation in
498 compressive strength of seawater OPC-SF system. As shown in Figs. 11 and 12, S0 specimen
499 exhibited a poorer pore size distribution compared to the D0 specimen, i.e., larger pore; and thus,
500 the seawater OPC pastes without SF had a lower compressive strength compared to the DI water
501 OPC pastes without SF. However, S30 specimen had a similar pore structure to D30, but the
502 compressive strength of S30 was significantly lower than that of D30. This suggested that the
503 strength reduction in seawater OPC-SF system was not mainly caused by the pore structure.

504

505 Fig. 14 shows the volume percentages of unhydrated clinkers and hydration products, based on the
506 difference in indentation modulus, in the hydrated pastes at 28 days. Without SF addition, S0 had
507 a comparable volume percentage of unhydrated clinkers to that of D0; however, the difference in
508 clinker volume between the seawater OPC pastes and DI water OPC pastes became larger as the
509 SF addition increased. Correspondingly, S0 exhibited a slightly low compressive strength
510 compared with D0, but the compressive strength of S30 specimen was significantly degraded
511 compared with D30 specimen, as shown in Fig. 4. Therefore, the change of the amount of

512 unhydrated clinker phases was the main factor resulting in the macro-strength change, which was
513 because the unhydrated clinkers had a high modulus and exhibited a significantly large scatter in
514 micromechanical properties. Similarly, the Rietveld results and ^{29}Si MAS-NMR results also
515 confirmed the changes in the unhydrated clinkers. As shown in Table 3, the content of unhydrated
516 clinkers in S0 at 28 days was slightly lower than that in D0, however, S30 had a significantly low
517 content of unhydrated clinkers compared with D30. Additionally, as shown in Table 6, S0 sample
518 exhibited a slightly low content of Q^0 distribution compared with D0 sample, but the Q^0 content in
519 S30 was notably lower than that in D30, and correspondingly the content of unhydrated siliceous
520 clinkers showed the same trend.

521



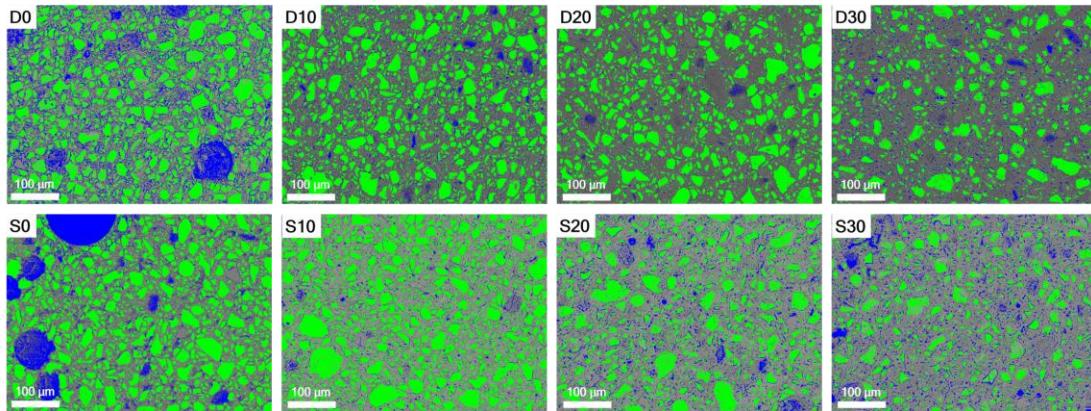
522

Fig. 14 Volume percentages of clinkers and products at 28 days based on the nanoindentation results

523

524 On the other hand, the compressive strength of both the seawater OPC pastes and DI water OPC
525 pastes increased firstly but then decreased with an increase in SF dosage (Fig. 4), which also
526 corresponded to the closest packing results (Fig. 2). The optimal SF dosage was 20% in the DI
527 water OPC pastes. However, for the seawater, the optimal was 10 wt.% SF addition. BSE was
528 used to investigate the pore evolution. Fig. 15 shows the pore size distribution of different
529

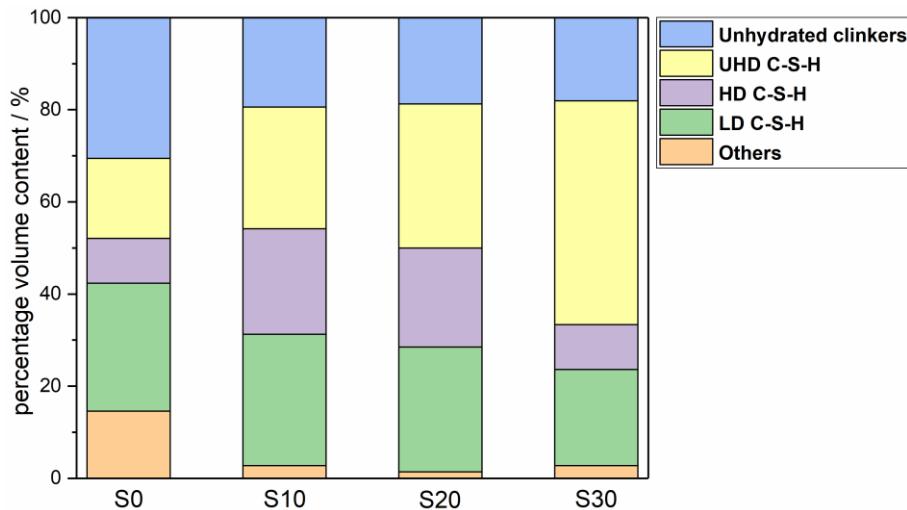
530 specimens. Image analysis on these hardened pastes was conducted by ImageJ software. The pores
531 were identified by the difference in gray levels from the solid matrix. Pores, unhydrated clinkers
532 and hydrated pastes are shown in different colors in order to clearly distinguish the phases. OPC
533 pastes prepared without SF, regardless of the mixing water used, had more larger pores. The larger
534 pores disappeared and only smaller pores were present when SF was added, suggesting that SF
535 incorporation improved the porosity. The EIS results showed consistent results as the impedance
536 moduli of seawater or DI water OPC-SF systems were notably higher than that of OPC systems
537 prepared without SF, indicating that the OPC-SF system had a denser pore structure and higher
538 external ions corrosion resistance. The average detectable pore contents of DI water OPC pastes
539 were found to be 11.0% (D0), 3.4% (D10), 1.7% (D20) and 4.1% (D30) respectively and D20 had
540 the lowest pore content. D20 also exhibited the highest compressive strength among all the DI
541 water pastes. However, the pore contents of seawater OPC pastes were 9.6% (S0), 2.1% (S10),
542 3.2% (S20) and 4.5% (S30), respectively, and S10 had the lowest pore content. Thus, the lowest
543 pore content of seawater OPC pastes appeared at a lower SF addition (10 wt%), compared with DI
544 water OPC pastes. A novel concept is proposed to explain this result, i.e., a favorable combination
545 of unhydrated clinkers and different C-S-H gel with varying densities might optimally contribute
546 to the high compressive strength. As shown in Fig. 16, the percentage volume contents of LD
547 C-S-H, HD C-S-H, UHD C-S-H and unhydrated clinkers in S10 sample exhibited a
548 well-proportioned distribution, which is believed to render a high macro-strength. However, the
549 distribution of the different phases in the S30 sample was significantly degraded, i.e., UHD C-S-H
550 accounted for a high percentage and the percentage of HD C-S-H was low.
551



552

553 **Fig. 15** Pore evolution of DI water OPC pastes and seawater OPC pastes at 28 days based on BSE images. Blue
554 region denoted pore, green region denoted unhydrated clinkers, and grey region denoted hydrated pastes.

555



556

557 **Fig. 16** Volume percentages of different phases in the seawater OPC systems at 28 days

558

559 5. Conclusion

560 The present work investigated the effect of SF on the macro-performances and
561 micro/nanostructure of seawater OPC pastes at an ultra-low W/B ratio. Based on the results of this
562 work, the main conclusions drawn are summarized as follows:

563

564 1. With respect to seawater OPC pastes at an ultra-low W/B, the usage of SF was found to enhance

565 the compressive strength at a low SF content. In the seawater OPC-SF system, 10 wt.% SF
566 incorporation exhibited the highest compressive strength while for the DI water OPC-SF system, a
567 20 wt.% SF was needed, and thus the amount of SF dosage might be saved in the seawater based
568 UHPC system.

569

570 2. The durability of the OPC-SF system, regardless of the type of mixing water, was improved
571 with an increase of SF addition. The later-age impedance modulus and external ions corrosion
572 resistance of seawater OPC pastes and DI water OPC pastes increased with the SF content. The
573 seawater OPC pastes exhibited better ions corrosion resistance compared to DI water OPC pastes
574 when 10 wt.% and 20 wt.% SF were used.

575

576 3. The accelerating effect of seawater on cement hydration was small at a later age for the OPC
577 pastes prepared with less than 10 wt% SF addition at an ultra-low W/B. The accelerating effect
578 was still obvious when 30 wt% SF was used. Friedel's salt was not found in the seawater OPC
579 pastes prepared with SF addition. In contrast, the AFt content significantly increased in the
580 seawater OPC-SF system. The polymerization degree and mean molecular chain length of C-S-H
581 gel in both of OPC system and OPC-30%SF system was reduced when seawater was used as the
582 mixing water.

583

584 4. The pore size distribution in the seawater OPC pastes prepared without SF was degraded with
585 more larger pores. However, OPC pastes prepared with SF, regardless of the type of mixing water,
586 exhibited excellent pore structures.

587

588 5. The compressive strength of seawater OPC-SF system with an ultra-low W/B was increasingly
589 degraded at a later age with increasing SF addition, compared with the corresponding DI water
590 OPC-SF system, mainly caused by the difference in the distribution of unhydrated clinkers
591 between those two systems. The seawater OPC-10% SF exhibited the highest compressive
592 strength and the lowest pore content among these seawater samples.

593

594 **Acknowledgments**

595 We wish to thank the financial supports of the Research Grants Council Theme Based Research
596 Scheme (T22-502/18-R), National Natural Science Foundation of China (52108252), Natural
597 Science Foundation of Hebei Province (E2021203147, E2019203413), Key Project of Hebei
598 Education Department (ZD2019096), and Hebei Science &Technology Program (21375401D).

599

600 **References**

- 601 [1] J. Xiao, C. Qiang, A. Nanni, K. Zhang, Use of sea-sand and seawater in concrete construction:
602 Current status and future opportunities, *Constr. Build. Mater.* 155 (2017) 1101-1111.
- 603 [2] J.G. Teng, Performance enhancement of structures through the use of fibrereinforced polymer
604 (FRP) composites, *Proceedings of 23rd Australasian Conference on the Mechanics of Structures
605 and Materials (ACMSM23)*, keynote presentation, Lismore, Australia, 2014.
- 606 [3] M. Guo, B. Hu, F. Xing, et al., Characterization of the mechanical properties of eco-friendly
607 concrete made with untreated sea sand and seawater based on statistical analysis, *Constr. Build.
608 Mater.* 234 (2020) 117339.
- 609 [4] J. Teng, Y. Xiang, T. Yu, Z. Fang, Development and mechanical behaviour of
610 ultra-high-performance seawater sea-sand concrete, *Adv. Struct. Eng.* 22 (14) (2019) 3100-3120.
- 611 [5] J. Wang, E. Liu, L. Li, Multiscale investigations on hydration mechanisms in seawater OPC paste,
612 *Constr. Build. Mater.* 191 (2018) 891-903.
- 613 [6] H. Zheng, W. Li, F. Ma, Q. Kong, The performance of a surface-applied corrosion inhibitor for
614 the carbon steel in saturated $\text{Ca}(\text{OH})_2$ solutions, *Cem. Concr. Res.* 55 (2014) 102-108.
- 615 [7] Y. Zhang, Y. Sun, H. Zheng, Y. Cai, W.L. Lam, C.S. Poon, Mechanism of strength evolution of
616 seawater OPC pastes, *Adv. Struct. Eng.* 24 (6) (2021) 1256-1266.
- 617 [8] P. Li, W. Li, T. Yu, F. Qu, V.W.Y. Tam, Investigation on early-age hydration, mechanical
618 properties and microstructure of seawater sea sand cement mortar, *Constr. Build. Mater.* 249
619 (2020) 118776.

620 [9] S.K. Kaushik, S. Islam, Suitability of sea water for mixing structural concrete exposed to a marine
621 environment, *Cem. Concr. Compos.* 17 (3) (1995) 177-185.

622 [10] A. Younis, U. Ebead, P. Suraneni, A. Nanni, Fresh and hardened properties of seawater-mixed
623 concrete, *Constr. Build. Mater.* 190 (2018) 276-286.

624 [11] C.G. Girish, D. Tensing, K.L. Priya, Dredged offshore sand as a replacement for fine aggregate in
625 concrete, *Int. J. Eng. Sci. Emerg. Technol.* 8 (3) (2015) 88-95.

626 [12] M.M. Islam, M.S. Islam, M. Al-Amin, M.M. Islam, Suitability of sea water on curing and
627 compressive strength of structural concrete, *J. Civ. Eng. (the Institution of Engineers, Bangladesh)*
628 40 (1) (2012) 37-45.

629 [13] T. Nishida, N. Otsuki, H. Ohara, et al. Some considerations for applicability of seawater as
630 mixing water in concrete, *J. Mater. Civ. Eng.* 27 (7) (2015). B4014004-1-7.

631 [14] S.A. Yaseen, G.A. Yiseen, C.S. Poon, Z. Li, Influence of seawater on the morphological evolution
632 and the microchemistry of hydration products of tricalcium silicates (C₃S). *Acs Sustain. Chem.*
633 Eng. 8 (2020) 15875-15887.

634 [15] P. Sikora, K. Cendrowski, M. Abd Elrahman, S. Chung, E. Mijowska, D. Stephan, The effects of
635 seawater on the hydration, microstructure and strength development of Portland cement pastes
636 incorporating colloidal silica, *Appl. Nanosci.* 10 (2020) 2627-2638

637 [16] L. Montanari, P. Suraneni, M. Tsui-Chang, et al., Hydration, pore solution, and porosity of
638 cementitious pastes made with seawater. *J. Mater. Civ. Eng.* 31(8) (2019) 04019154.

639 [17] G.C. Edwards, R.L. Angstadt, The effect of some soluble inorganic admixtures on the early
640 hydration of portland cement, *J. Appl. Chem.* 16 (5) (1966) 166-168.

641 [18] M. Alkaysi, S. El-Tawil, Z. Liu, et al., Effects of silica powder and cement type on durability of
642 ultra high performance concrete (UHPC), *Cem. Concr. Compos.* 66 (2016) 47-56.

643 [19] K. Wille, S. El-Tawil, A.E. Naaman, Properties of strain hardening ultra high performance fiber
644 reinforced concrete (UHP-FRC) under direct tensile loading, *Cem. Concr. Compos.* 48 (2014)
645 53-66.

646 [20] K. Wille, A.E. Naaman, G.J. Parra-Montesinos, Ultra-High Performance Concrete with
647 Compressive Strength Exceeding 150 MPa (22 ksi): A Simpler Way, *Aci Mater. J.* 108 (1) (2011)
648 46-54.

649 [21] C. Shi, Z. Wu, J. Xiao, D. Wang, Z. Huang, Z. Fang, A review on ultra high performance concrete:
650 Part I. Raw materials and mixture design, *Constr. Build. Mater.* 101 (2015) 741-751.

651 [22] K. Wille, C. Boisvert-Cotulio, Material efficiency in the design of ultra-high performance
652 concrete, *Constr. Build. Mater.* 86 (2015) 33-43.

653 [23] H. Li, N. Farzadnia, C. Shi, The role of seawater in interaction of slag and silica fume with
654 cement in low water-to-binder ratio pastes at the early age of hydration, *Constr. Build. Mater.* 185
655 (2018) 508-518.

656 [24] R. Yu, P. Spiesz, H.J.H. Brouwers, Mix design and properties assessment of Ultra-High
657 Performance Fibre Reinforced Concrete (UHPFRC), *Cem. Concr. Res.* 56 (2014) 29-39.

658 [25] J. Zhang, G.W. Scherer, Comparison of methods for arresting hydration of cement, *Cem. Concr.*
659 *Res.* 41 (10) (2011) 1024-1036.

660 [26] B.J. Zhan, D.X. Xuan, C.S. Poon, Enhancement of recycled aggregate properties by accelerated
661 CO₂ curing coupled with limewater soaking process, *Cem. Concr. Compos.* 89 (2018) 230-237.

662 [27] W. Ashraf, N. Tian, Nanoindentation assisted investigation on the viscoelastic behavior of
663 carbonated cementitious matrix: Influence of loading function, *Constr. Build. Mater.* 127 (2016)

664 904-917.

665 [28] W. Ashraf, J. Olek, J. Jain, Microscopic features of non-hydraulic calcium silicate cement paste
666 and mortar, *Cem. Concr. Res.* 100 (2017) 361-372.

667 [29] M. Santhanam, M. Cohen, J. Olek, Differentiating seawater and groundwater sulfate attack in
668 Portland cement mortars, *Cem. Concr. Res.* 36 (12) (2006) 2132-2137.

669 [30] G. Li, A. Zhang, Z. Song, C. Shi, Y. Wang, J. Zhang, Study on the resistance to seawater
670 corrosion of the cementitious systems containing ordinary Portland cement or/and calcium
671 aluminate cement, *Constr. Build. Mater.* 157 (2017) 852-859.

672 [31] C. Qiao, P. Suraneni, T. Nathalene Wei Ying, A. Choudhary, J. Weiss, Chloride binding of
673 cement pastes with fly ash exposed to CaCl_2 solutions at 5 and 23 °C, *Cem. Concr. Compos.* 97
674 (2019) 43-53.

675 [32] H. Hirao, K. Yamada, H. Takahashi, H. Zibara, Chloride binding of cement estimated by binding
676 isotherms of hydrates, *J. Adv. Concr. Technol.* 3 (1) (2005) 77-84.

677 [33] M.V.A. Florea, H.J.H. Brouwers, Chloride binding related to hydration products, *Cem. Concr.*
678 *Res.* 42 (2) (2012) 282-290.

679 [34] K. De Weerdt, A. Colombo, L. Coppola, H. Justnes, M.R. Geiker, Impact of the associated cation
680 on chloride binding of Portland cement paste, *Cem. Concr. Res.* 68 (2015) 196-202.

681 [35] U. Ebead, D. Lau, F. Lollini, A. Nanni, P. Suraneni, T. Yu, A review of recent advances in the
682 science and technology of seawater-mixed concrete, *Cem. Concr. Res.* 152 (2022) 106666.

683 [36] P. Li, W. Li, Z. Sun, L. Shen, D. Sheng, Development of sustainable concrete incorporating
684 seawater: A critical review on cement hydration, microstructure and mechanical strength, *Cem.*
685 *Concr. Compos.* 121 (2021) 104100.

686 [37] J. Wang, B. Han, Z. Li, et al., Effect investigation of nanofillers on CSH gel structure with Si
687 NMR, *J. Mater. Civ. Eng.* 31(2018) 04018352.

688 [38] D. Wang, Y. Fang, Y. Zhang, J. Chang, Changes in mineral composition, growth of calcite crystal,
689 and promotion of physico-chemical properties induced by carbonation of β -C₂S, *J. CO₂ Util.* 34
690 (2019) 149-162.

691 [39] D. Da Silva Andrade, J.H. Da Silva Rêgo, P.C. Moraes, A.N. de Mendonça Lopes, M.F. Rojas,
692 Investigation of C-S-H in ternary cement pastes containing nanosilica and highly-reactive
693 supplementary cementitious materials (SCMs): Microstructure and strength, *Constr. Build. Mater.*
694 198 (2019) 445-455.

695 [40] B. Lothenbach, A. Nonat, Calcium silicate hydrates: Solid and liquid phase composition, *Cem.*
696 *Concr. Res.* 78 (2015) 57-70.

697 [41] I.G. Richardson, Model structures for C-(A)-S-H(I), *Acta Crystallogr. Sect. B* 70 (6) (2014)
698 903-923.

699 [42] C. Hu, Z. Li, A review on the mechanical properties of cement-based materials measured by
700 nanoindentation, *Constr. Build. Mater.* 90 (2015) 80-90.

701 [43] J. Tatar, C.R. Taylor, H.R. Hamilton, A multiscale micromechanical model of adhesive interphase
702 between cement paste and epoxy supported by nanomechanical evidence, *Compos. B Eng.* 172
703 (2019) 679-689.

704 [44] G. Constantinides, F. Ulm, The nanogranular nature of C-S-H, *J. Mech. Phys. Solids.* 55 (1) (2007)
705 64-90.

706 [45] P. Mondal, S.P. Shah, L. Marks, A reliable technique to determine the local mechanical properties
707 at the nanoscale for cementitious materials, *Cem. Concr. Res.* 37 (10) (2007) 1440-1444.

708 [46] L. Li, Z. Li, M. Cao, et al. Nanoindentation and porosity fractal dimension of calcium carbonate
709 whisker reinforced cement paste after elevated temperatures (up to 900°C). *Fractals*, 29 (2) (2021)
710 2140001

711 [47] M. Vandamme, F. Ulm, P. Fonollosa, Nanogranular packing of C-S-H at substochiometric
712 conditions, *Cem. Concr. Res.* 40 (1) (2010) 14-26

713