

1 **Discussion of “Validation of a Novel Sensing Approach for Continuous**
2 **Pavement Monitoring Using Full-Scale APT Testing” by Mario Manosalvas-**
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20 The authors conducted a comprehensive full-scale accelerated pavement test (APT) on
21 conventional flexible pavement for the detection of bottom-up fatigue cracking. During the APT
22 loading process, the changes in pavement materials and structural conditions, such as structural
23 capacity (layer moduli) and functional performance (international roughness index-IRI), were
24 monitored using piezo-powered smart sensors to track the harvested variations in strain energy
25 rather than the traditional strain measurements. The comparison study between the newly
26 developed sensors and the conventional approach indicated that the performance of the
27 piezoelectric sensor was successfully validated by the conventional strain gauge in a full-scale
28 testing pavement system. In addition, the cumulative loading time of piezo-voltage could serve
29 as an indicator of the pavement damage progress, and the timing of the activation of sensor
30 thresholds with different voltage levels could reveal the pavement structural damage severity,
31 contributing to the pavement structural health monitoring (SHM) and maintenance. The
32 discussers highly appreciate the work of the authors and would like to provide some comments
33 regarding the experimental process, analysis, and results.

34 Design of the pavement structure (layers and thickness) plays a significant role in
35 controlling the structural responses of pavement under the repeated loading process, especially
36 during the full-scale APT testing (Cortes Avellaneda 2010; Jiang et al. 2022b; Li et al. 1999; Terrell et
37 al. 2003). The discussers recommend the authors explain why the thicknesses of the asphalt
38 concrete layer (AC), unbound aggregate base layer (UAB), and subgrade are set as 102 mm, 760
39 mm, and 1,600 mm, respectively. The relative layer thickness of the pavement among all layers
40 determines its performance (Jiang et al. 2021). Therefore, an explanation for the pavement design
41 could provide a good reference for future applications.

42 APT is considered a highly efficient and versatile testing approach to simulate the actual
43 wheel loading on the full-scale pavement structure, and evaluate pavement structural responses
44 and performance in a shorter duration (Jiang et al. 2022c; Leng et al. 2009; Ling et al.
45 2020). Compared with conventional on-site investigations, the full-scale APT test on the
46 pavement has the advantages of controlled loading and environmental conditions. In this study,
47 single-axle dual-wheels of 65 kN and an approximate velocity of 76 km/h of moving wheels
48 were chosen. It is worthwhile that the authors could explain how to set the parameters of the
49 APT facility. In addition, the authors could provide operation experiences of APT equipment,
50 serving as a good reference for other APT facility users.

51 Falling weight deflectometer (FWD) is a type of nondestructive road testing device for
52 pavement structural analysis (Horak 2008; Jiang et al. 2022a; c; Talvik and Aavik 2009). In this study,
53 the authors concluded that most deflection change was limited to the upper layers, and most
54 damage took place in the asphalt layer. Usually, the deformation of the subgrade accounts for the
55 largest proportion among all pavement layers (Gong et al. 2018; Maina and Matsui 2004; Vyas et al.
56 2020; Wang et al. 2022b). Thus, the authors are recommended to clarify this phenomenon. In
57 addition, the qualitative analysis of the FWD deflection could not reveal the actual layers'
58 condition. In this study, the layer moduli back-calculation using Dynatest ELMOD version 6
59 software was conducted to back-calculate the different layer moduli of the pavement. However,
60 in the back-calculation program, the solution's reliability depends on the seed moduli, making
61 the back-calculation an ill-posed process. Due to inaccurate results, the multi-layer linear elastic
62 theory and back-calculation procedures have come under scrutiny (Horak 2008). In addition, the
63 accuracy of any back-calculation method primarily depends on the accurate estimation of
64 individual layer thickness. Nevertheless, the actual layers' thicknesses were not validated in this

65 study. In previous studies, coring and ground penetration radar (GPR) are popular methods to
66 assess pavement layer thickness(Leng and Al-Qadi 2014; Wang et al. 2018, 2022a, 2023).
67 Coring operations are significantly time-consuming and resource-intensive. Traffic management
68 is required when coring is conducted, bringing burdens to daily traffic operations. GPR has not
69 been put into regular use by most transportation agencies, which hinders the evaluation progress.
70 Thus, a more convenient, safe, and cost-efficient method is needed to investigate the pavement
71 layers' performance (modulus) after repeated loadings. For the conventional flexible pavement, a
72 deflection basin parameters (DBPs)-based pavement evaluation was proposed by Horak et al
73 (Horak, E., Emery, S., and Maina 2015). The evaluation parameters such as Surface Curvature
74 Index (SCI), Base Damage Index (BDI), and Base Curvature Index (BCI) were used to
75 characterize the condition of the surface layer, base layer, and subgrade layer, respectively. SCI
76 represents the difference of deflections measured with load geo- phones located at the center of
77 the loading plate (D_0) and 300 mm (12 in.) from the center. BDI represents the difference of
78 deflections measured with load geophones located at a distance of 300 mm (12 in.) and 600 mm
79 (24 in.). BCI represents the difference of deflections measured with load geophones located at
80 600 mm (24 in.) and 900 mm (36 in.). The threshold values were also presented based on a load
81 of 40 kN or a contact pressure of 560 kPa by the FWD testing method. Therefore, the authors are
82 recommended to apply the DBPs-based evaluation system after loading calibration in the future
83 field investigation.

84 Environmental factors such as temperature and moisture are of great importance to the
85 performance of the pavement structure. In this study, the back-calculated asphalt modulus was
86 corrected to a reference temperature of 20°C following Highways England CS 229. The authors
87 are recommended to show how to control the testing temperatures during the APT test. In

88 addition, the IFSTTAR circular test track (CTT) is an outdoor APT facility. Moisture in the base
89 layers and subgrade soil may degrade the performance of the pavement structures. Could you
90 provide the experience for controlling the moisture or considering the moisture effects of the
91 base layers and subgrade of the testing pavements? The discussers believe the authors could
92 provide a reasonable explanation for these questions.

93 Visual observation based on the experience could provide a fast and direct evaluation of the
94 surface layer condition. In this study, the extent of cracking in the longitudinal and transverse
95 directions was investigated and calculated. However, the criteria (widths) for the cracking
96 detection were missing. According to the Federal Highway Administration (FHWA) standard
97 (*The Long-Term Pavement Performance Program 2017*), there are three severity levels according to
98 the crack width: low (≤ 6 mm), medium (6 to 19 mm), and high (≥ 19 mm). The authors are
99 recommended to provide the evaluation criteria for visual cracking detection. In this study, the
100 cracks were considered classical bottom-up fatigue cracking. Asphalt pavement may have both
101 bottom-up and top-down cracking. Could you provide your analysis for classifying these cracks?

102 The major contribution of this study is to develop a novel piezo-powered sensing system
103 combined with a new response-only-based approach for data reduction and interpretation for the
104 continuous monitoring of pavement conditions. In this study, the piezoelectric sensors were
105 placed at the bottom of the asphalt layer. The unbound aggregates were below the AC layer and
106 the potential slippery between the aggregates and the AC bottom would occur, influencing the
107 accuracy of the sensors. Thus, the authors are recommended to provide an explanation for the
108 sensor's installation and data accuracy acquired by the piezo-powered sensors. In addition, the
109 construction experience for the AC layer containing sensors is recommended to be provided.

110 Another interesting point we want to discuss with the authors is the further application of
111 the sensors. The structural responses of the UAB layer have attracted attention from academia
112 and industry for many years. The stress dependency of the granular materials plays an important
113 role in pavement performance. In this study, the selected piezo-transducers for this work were
114 designed to respond only to tension and not to compression. Nevertheless, the unbound
115 aggregates could not experience tension during the loading process. If this type of sensor is
116 applied to the UAB layer in the future, what improvements or modifications will you do?

117 The long-term monitoring of the pavement's structural health is critical to the pavement
118 industry and stakeholders. The authors' research is of great value to the technical advances and
119 promotion of pavement maintenance and rehabilitation. The discussers believe the authors would
120 complete more comprehensive and valuable studies after reviewing and answering the above
121 discussions and questions.

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