

# 1     **Recycling of red mud and flue gas residues in geopolymer** 2                    **aggregates (GPA) for sustainable concrete**

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## 4     **Highlights**

- 5        • Red mud (RM) and flue gas residue (FGR) partially replaced precursor and  
6           activator
- 7        • Replacement rate of fly ash by RM and Na<sub>2</sub>O in activator by FGR reached 40%  
8           and 32%
- 9        • RM and FGR had minimal impacts on compressive strength of GPA pastes
- 10      • Concrete strength declined by 13.4% upon 100% replacement of NA by GPA

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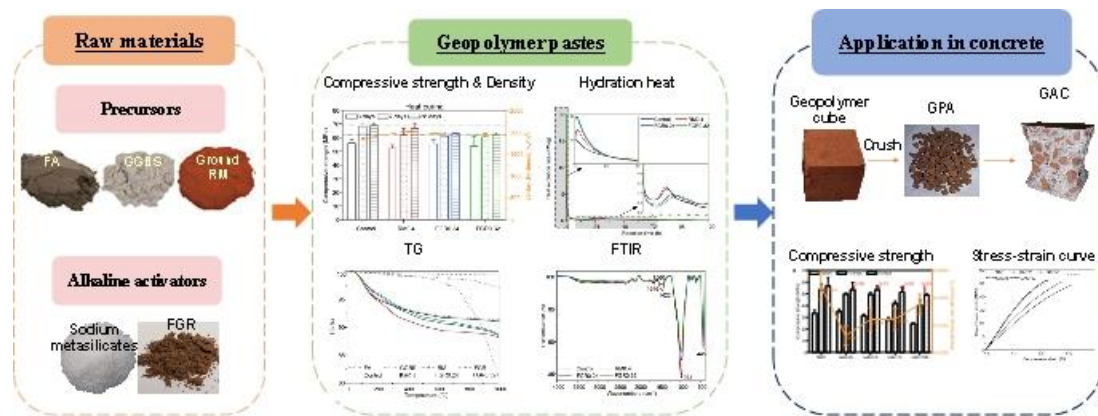
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11    **Abstract**

12    This study proposed simultaneous utilization of red mud (RM) and flue gas residues  
13    (FGR) as a partial replacement of traditional precursors and energy-intensive activator.  
14    Reaction mechanism of geopolymer pastes were investigated through isothermal  
15    calorimetry, X-ray diffraction, thermogravimetry and infrared spectroscopy, by varying  
16    the dosage of RM (40%) and FGR (24%, 32%). Results showed that use of RM and  
17    FGR as a replacement of FA and commercial activator had a limited influence on the  
18    compressive strength. RM and FGR participated in reaction at a slower rate. After  
19    optimization, heat-cured FGR0.32 mix was selected to manufacture GPA. Engineering  
20    properties, cost analysis and environmental impact of GPA were evaluated. Natural  
21    aggregates (NA) were replaced by GPA at replacement levels of 25%, 50%, 75% and  
22    100% by volume to prepare concrete. GPA concrete with 25% and 100% replacement  
23    level could achieve the 94.6% and 86.6% of NA concrete strength respectively at 28  
24    days age.

25    **Keywords:** Geopolymer aggregates (GPA); Red mud (RM); Flue gas residues (FGR);  
26    Geopolymer aggregate concrete (GAC); Mechanical properties

## 27 Graphical Abstract



## 1 Introduction

Red mud (RM) is an industrial waste discharged during alumina extraction by Bayer process. The refining of 1 ton of alumina generates approximately 1.5-2.5 tonnes of RM [1]. The global production of RM is estimated at 70 to 120 million tonnes annually [2] due to the growing demand for alumina. China and Australia account for around 80% of total production of alumina [3]. The characteristics of RM, e.g., strong alkalinity, high fineness, and concentrated heavy metals make its reclamation further difficult [4]. The significant production of alumina has prompted a number of studies to focus on the efficient recycling of RM [5-8]. The most promising practical application of recycling RM seems to be in the construction industry which consumes a significant amount of building materials. Since RM contains Si and Al, many researchers have studied the RM-based geopolymer or alkali-activated materials which exhibited comparable mechanical characteristics and stabilization of toxic elements to other cementitious materials [9]. However, RM is not recommended as the sole precursor for preparing geopolymer due to its relatively poor reactivity and low Si/Al molar ratio, thus it is usually pretreated (e.g., alkali-thermal treatment or mechanical milling) or mixed with other Si-rich materials [10]. Furthermore, RM has been reported as the sole Na source to make low-strength blocks [11] while the  $D_{50}$  (average particle size) of RM was only 3  $\mu\text{m}$ , which was much finer than the values in other studies. According to the findings in [12] where the sizes of RM were under 300  $\mu\text{m}$ , most of Na in the raw RM was insoluble.

Flue gas residues (FGR) refer to an industrial waste after the cleaning procedures of the flue gases generated by the incineration of municipal solid wastes, clinical wastes or sewage sludge [13]. Hong Kong has one of the world's largest sewage sludge incinerators with a design capacity of 2000 tonnes/day. Besides, the first municipal solid waste incineration plant (IWMF) with a capacity of 3000 tonnes/day is under construction in Hong Kong and will soon start to operate in 2025. It can be expected that solid wastes will increase significantly as the economy is growing and the FGR production will rise accordingly. FGR produced in the Hong Kong incineration plant contains high contents of alkaline oxides and heavy metals, posing significant

challenges to deal with this waste. FGR is mostly landfilled after stabilization/solidification with hydraulic binders (usually cement) due to its high content of heavy metals and high alkalinity. However, this is not a long-term solution due to the limited landfill capacity and wasting of recoverable resources. Few studies focused on the recycling of FGR as construction materials, e.g., sintered artificial aggregates [14, 15], blended cement [16, 17] and geopolymer [18].

Recycling wastes into artificial aggregates has been demonstrated to be an effective method of turning wastes into commercial products in the research area of green building materials [19-22]. Generally, aggregates occupy around 60% to 80% volume of concrete, hence preparing artificial aggregates from industrial wastes could significantly ease the disposal burden on landfills. Artificial aggregates currently include sintered aggregates and cold-bonded aggregates. Sintered aggregates require high temperatures over 1000 °C to harden the pellets by fusing raw material particles, which are energy-intensive and costly. Cold-bonded aggregates typically utilize cementitious materials to bind the waste materials at curing temperatures below 105°C. Geopolymer as a kind of cement-free binder have been reported to have good mechanical properties and superior durability [23-25]. Hence, one newly proposed type of cold-bonded aggregates is geopolymer aggregates (GPA) and it has been successfully applied in concrete or engineered composites in recent years [26-31]. Although research in the geopolymer and alkali-activated materials has sought consider attention of scientists, the raw materials utilized in this field are mostly limited to fly ash, GGBS and commercial alkaline activators. However, fly ash and GGBS are now considered as commercial products and they are slightly cheaper than cement. Besides, the greatest concern of geopolymer in terms of cost and carbon emission is the commercial alkaline activator. GPA produced in such case have higher cost and not very environmental-friendly. Hence some studies started using some low-activity wastes as precursors, whereas the quality of the produced GPA decreased [26]. Most studies of artificial aggregates focused on the pelletized aggregates but few studies explored aggregates production through crushing technology [32]. The common practice requires geopolymer concrete to be cured at elevated temperatures to achieve a reasonable strength

development rate [25]. Concrete wastes have been reported to have a more than 12% share in total solid waste in some countries [33]. A significant amount of geopolymer concrete waste can be expected in the future due to the increase in use of geopolymer concrete in construction industry. The recycled GPA as coarse aggregates have been successfully used in Portland cement concrete and geopolymer concrete [34]. Hence, the investigations on crushed GPA not only can directly recycle waste materials as GPA, but also can be a reference to the application of recycled GPA in the future.

This study aims to evaluate the feasibility of recycling both RM and FGR into synthesizing GPA. RM and FGR were designed to partially replace the main precursor and alkaline activator, respectively. Reduction of CO<sub>2</sub> emissions and cost from the commercial alkaline activator can be achieved by using FGR as a partial alkali source. In addition, replacing FA with RM is not only a timely response for the decreased supply of FA in the future but is also capable of consuming a considerable amount of RM and converting it to valuable products. FGR was selected as the waste-derived activator based on its high Na<sub>2</sub>O content from the material characterization results. Coal fly ash (FA) was the main precursor and ground granulated blast-furnace slag (GGBS) was added to improve the strength. First, the compressive strength and the reaction mechanism of the geopolymer pastes was investigated through isothermal calorimetry, X-ray diffraction (XRD), thermogravimetric analysis (TGA) and infrared spectroscopy measurement. Additionally, the leaching behavior of geopolymer pastes was also studied to evaluate if the leaching values of toxic elements are within the regulatory limits. Second, one paste mix with appropriate performance was crushed to produce GPA and then prepare concrete. The effects of different volume substitution of GPA with natural aggregates (NA) on the performance of concrete were discussed. This work is expected to provide a useful practice for large-scale recycling of RM and FGR through the manufacturing of GPA.

## 2 Materials and mix design

### 2.1 Materials

#### 2.1.1 Raw materials for geopolymer pastes and GPA

The powder materials for production of geopolymer pastes and GPA in this study included coal fly ash (FA), ground granulated blast-furnace slag (GGBS), red mud (RM) and flue gas residues (FGR). FA was locally provided by Green Island Cement Co. Ltd (Hong Kong) and GGBS was purchased from Mainland China. RM was a by-product of the bauxite refining into alumina from a plant in Henan Province, China. FGR was collected from the local sewage sludge incineration plant (T-park) in Hong Kong. In this study, some large lumps were found in the as-received RM. Hence the original RM was ground using a ball mill and then sieved to below 300  $\mu\text{m}$  prior to use. From Fig. 1(a), the particle sizes of FA, GGBS and FGR were all smaller than 120  $\mu\text{m}$  and the  $D_{50}$  of FA, GGBS and FGR was 18.66  $\mu\text{m}$ , 12.72  $\mu\text{m}$ , and 6.36  $\mu\text{m}$ , respectively, which were determined by a laser particle size analyzer (Malvern Mastersizer 3000). It can be found that FGR had the lowest average particle size. However, the particle sizes of RM were relatively coarser and the  $D_{50}$  particle size was 47.90  $\mu\text{m}$ .

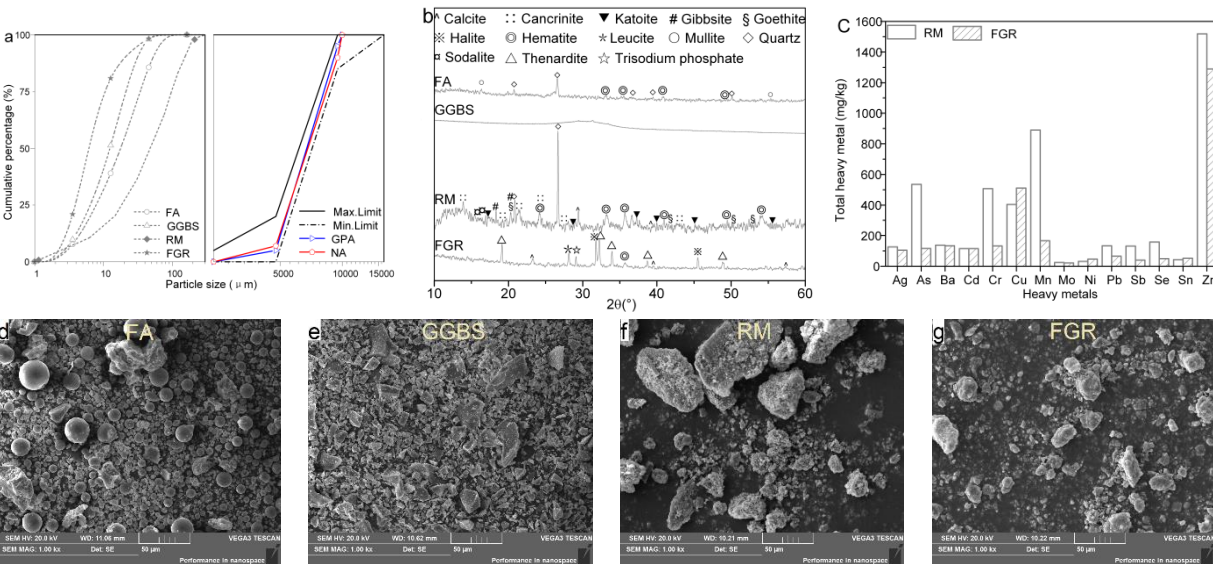


Fig. 1 Characterization of raw materials: (a) Particle size distribution of waste materials and coarse aggregates for concrete, (b) XRD pattern of waste materials, (c) Total concentration of heavy metals in RM and FGR, (d)-(g) SEM images of FA, GGBS, RM, and FGR, respectively.

Table 1 Chemical components of raw materials

Oxides (%)	FA	GGBS	RM	FGR
SiO <sub>2</sub>	49.90	31.80	17.90	6.80
Al <sub>2</sub> O <sub>3</sub>	24.80	14.00	21.80	3.80
Fe <sub>2</sub> O <sub>3</sub>	8.41	0.27	36.90	4.81
CaO	6.36	43.0	3.92	3.42
MgO	2.83	6.66	0.53	1.72
Na <sub>2</sub> O	1.84	1.27	10.20	38.10
K <sub>2</sub> O	1.72	0.41	0.85	0.56
TiO <sub>2</sub>	1.24	0.54	3.81	0.20
P <sub>2</sub> O <sub>5</sub>	0.68	0.11	0.48	2.88
SO <sub>3</sub>	1.66	1.69	3.06	31.40
MnO	0.08	0.21	0.07	0.02
Cl	0.11	-	-	5.98
Others	0.38	0.05	0.47	0.30
Specific gravity	2.28	2.92	3.03	2.97
Moisture content (%)	0.20	0.10	2.91	0.40

Note: The chemical compositions were determined by Rigaku Supermini200. The specific gravity was tested by water pycnometer method following the ISO 17892-3 and moisture content was determined by drying the samples at 105 °C in an oven for 24 hours.

According to Table 1, as the sum content (83.1%) of SiO<sub>2</sub>, Al<sub>2</sub>O<sub>3</sub> and Fe<sub>2</sub>O<sub>3</sub> in FA was higher than 50%, the CaO content (6.36%) was less than 18%, and the LOI value (3.33%) measured at 950 °C for 30 minutes was less than 6%, FA can be classified as Class F as per ASTM C618-19 [35]. The main components of GGBS were SiO<sub>2</sub> (31.8%), Al<sub>2</sub>O<sub>3</sub> (14.0%), and CaO (43.0%). The main constituents in RM were SiO<sub>2</sub> (17.9%), Al<sub>2</sub>O<sub>3</sub> (21.8%), Fe<sub>2</sub>O<sub>3</sub> (36.9%) and Na<sub>2</sub>O (10.2%). The major elements in FGR were Na<sub>2</sub>O (38.1%) and SO<sub>3</sub> (31.4%). The high content of SO<sub>3</sub>, which are soluble in water, highly affects the environment. Other contents such as SiO<sub>2</sub>, Al<sub>2</sub>O<sub>3</sub>, Fe<sub>2</sub>O<sub>3</sub> and CaO in FGR were low, which may indicate limited potential as precursor for geopolymerization. Besides, FA, GGBS, RM and FGR had a specific gravity of 2.28, 2.92, 3.03 and 2.97, respectively, which showed that FA was the lightest raw material of them. Additionally, the moisture contents of FA, GGBS and FGR were all less than 0.5% while the moisture content of RM was relatively higher (2.91%).

Fig. 1b presents the X-ray diffraction (XRD) patterns of the four types of powder materials. Quartz (SiO<sub>2</sub>) and mullite (3Al<sub>2</sub>O<sub>3</sub>-2SiO<sub>2</sub>) were the most common crystals in FA. GGBS had a



highly amorphous hump around 25-35°. RM contained many kinds of crystal phases including calcite ( $\text{CaCO}_3$ ), cancrinite ( $\text{Na}_6\text{Ca}_{1.5}\text{Al}_6\text{Si}_6\text{O}_{24}(\text{CO}_3)_{1.6}$ ), katoite ( $\text{Ca}_3\text{Al}_2(\text{SiO}_4)(\text{OH})_8$ ), gibbsite ( $\text{Al}(\text{OH})_3$ ), goethite ( $\text{FeO}(\text{OH})$ ), hematite ( $\text{Fe}_2\text{O}_3$ ), quartz and sodalite ( $\text{Na}_8[\text{AlSiO}_4]6\text{Cl}_2$ ). FGR consisted of calcite ( $\text{CaCO}_3$ ), thenardite ( $\text{Na}_2\text{SO}_4$ ), leucite ( $\text{KAlSi}_2\text{O}_6$ ), hematite ( $\text{Fe}_2\text{O}_3$ ), halite ( $\text{NaCl}$ ) and trisodium phosphate ( $\text{Na}_3\text{PO}_4$ ).

The results of total concentrations in RM and FGR are provided in Fig. 1c. The major heavy metals (>200 mg/kg) presented in RM were As (535 mg/kg), Cr (507 mg/kg), Cu (404 mg/kg), Mn (890 mg/kg) and Zn (1519 mg/kg) with the highest concentration. The major heavy metals presented in FGR were Cu (511 mg/kg) and Zn (1289 mg/kg). Other heavy metals, such as Ag, Ba, Cd, Pb, Sb and Se, were also detected in very low concentrations in RM and FGR. Hence, it is highly suggested that RM and FGR require pre-treatment when they are aimed to be recycled as construction materials.

Knowing that the morphology of phases in the final products are inherited from the parent materials, it is necessary to test the morphological patterns of these four materials through scanning electron microscopic (SEM) observed by Tescan VEGA3, as displayed in Fig. 1(d-g). Round particles were predominant in FA with several irregular fragments, which could be some impurities such as unburnt carbon. GGBS had angular-shaped particles. Majority of the RM particles were porous with irregular shape and comprised of many micrometer-sized agglomerates. Similarly, FGR particles were also irregular with fluffy appearance while the sizes were much smaller than that of RM.

Industrial-grade anhydrous sodium metasilicate ( $\text{Na}_2\text{SiO}_3$ -anhydrous) purchased from Qingdao Haiwan Chemical Co. Ltd, China, was employed as the alkali activator. The chemical compositions of the sodium metasilicate particles were 55.6%  $\text{Na}_2\text{O}$ , 43.7%  $\text{SiO}_2$  and 0.7% impurities by weight, as tested by X-ray fluorescence. Therefore, the calculated silica modulus (the molar ratio of  $\text{SiO}_2/\text{Na}_2\text{O}$ ) was 0.812 for the sodium metasilicate. The particle size range of the sodium metasilicate was between 0.3-1.0 mm with  $D_{50}$  of 706  $\mu\text{m}$ , as measured by a laser particle size analyzer. The solid activator particles were dissolved in tap water and cooled down

at ambient temperature before mixing with powder materials.

### **2.1.2 Materials for concrete**

Type I 52.5 N OPC was locally purchased from Green Island Cement Co. Ltd (Hong Kong) as a binder to prepare concrete. River sand ( $<2.36\text{mm}$ ) with a fineness modulus of 2.70 was used as fine aggregates. The specific gravity and the loose bulk density of the river sand were 2.60 and  $1495.8\text{ kg/m}^3$ , respectively. The water absorption of river sand was measured as 1.02%. The size of coarse NA (granite) was in the range of 5-10 mm. The specific gravity and the loose bulk density of the coarse NA were 2.67 and  $1538.0\text{ kg/m}^3$ , respectively. The water absorption of coarse NA was measured as 0.8%. NA were washed and dried before their use in concrete. Polycarboxylate superplasticizer was adopted in mixing and tap water was used in concrete.

## **2.2 Mix design**

### **2.2.1 Mix design for geopolymer pastes and GPA**

The mix proportion for geopolymer pastes is provided in Table 2. The control group was the group without RM and FGR. Mass ratio of GGBS/FA (20/80) and sodium metasilicate/precursors (12%) as well as a lower water-to-binder ratio were adopted based on a previous study [32], which reported the highest compressive strength of GAC compared with other mixes. It was observed that the addition of GGBS in GPA resulted in stronger geopolymer-matrix interfaces in GAC [36]. A lower water-to-binder ratio aimed to further increase the strength. RM0.4 represented the group using RM as partial substitute of FA and 0.4 denoted the substitution rate by mass (40%). In Mix FGR0.24 and FGR0.32, 40% mass of FA was still replaced by RM and sodium metasilicate was further partially replaced by FGR. The number after FGR represented the mass replacement ratio of  $\text{Na}_2\text{O}$  in sodium metasilicate by FGR (24% and 32%). The substitution rate of RM was determined by the molar ratio of  $\text{SiO}_2/\text{Al}_2\text{O}_3$ , which was between 3.3-4.5 as suggested in previous studies [37]. The range of molar ratio of  $\text{SiO}_2/\text{Al}_2\text{O}_3$  in this study was 3.53-4.15. The replacement ratios of FGR were determined based on trial tests and overall considerations of performance and heavy metals. The powder materials were mixed first and then blended with prepared alkaline activator solution. The

water/precursors ratios were consistent in all mixes. A total of 48 cubes of 40 mm were mixed and cast (12 cubes for each mix). The mixing of raw materials and casting of fresh pastes were conducted at room temperature and a relative humidity of 65%. All paste specimens were demolded 1 day after casting. To investigate the effects of curing method on the paste properties, 6 specimens of each group were sealed in plastic bags under ambient temperature while the other 6 specimens were sealed and placed in an oven at 80°C for 24 hours (heat curing) and then sealed in plastic bags till testing. The optimal mix balancing the utilization ratios of RM and FGR as well as the performance assessment was selected for the GPA manufacturing. The crushing technique was employed as the production method of GPA referring to a previous study [32].

Table 2 Mix proportions of geopolymer paste and concrete (kg/m<sup>3</sup>)

<b>Geopolymer paste mix</b>	<b>GGBS</b>	<b>FA</b>	<b>RM</b>	<b>FGR</b>	<b>Sodium metasilicate</b>	<b>Water</b>
Control	300	1200	n/a	n/a	180	417
RM0.4	300	720	480	n/a	180	417
FGR0.24	300	720	480	63	136.8	417
FGR0.32	300	720	480	84	122.4	417
<b>Concrete group</b>	<b>Cement</b>	<b>Sand</b>	<b>GPA</b>	<b>NA</b>	<b>Water</b>	<b>Superplasticizer (in solid)</b>
NAC	420	756	n/a	1069	168	2.1
GAC25	420	756	200	802	168	2.1
GAC50	420	756	400	535	168	2.1
GAC75	420	756	600	267	168	2.1
GAC100	420	756	800	n/a	168	2.1

### 2.2.2 Mix design for concrete

The NA and crushed GPA of 5-10 mm size were employed in the preparation of concrete. As shown in Table 2, five concrete groups were cast in total. The control group was prepared by using NA and designated as NAC. The other four groups were geopolymer aggregate concrete (GAC) incorporating 25%, 50%, 75% and 100% GPA by volume, respectively. The mass of GPA was calculated based on the saturated-surface-dried (SSD) particle density (Section 3.2.1 and Section 4.2). Considering its high water absorption, GPA were pre-saturated and sealed prior to use. The mixing and casting of concrete were conducted at the same temperature and

relative humidity with that of geopolymer. For each group, nine cylindrical specimens were cast in total to test the mechanical properties at the curing age of 3, 7 and 28 days. The diameter and height of the cylindrical specimen were 75 mm and 150 mm, respectively. All the concrete specimens were demolded 1 day after casting and cured in lime water till the testing day.

### **3 Testing procedures**

#### **3.1 Test methods for geopolymer pastes**

##### **3.1.1 Density and Compressive strength**

To understand the strength development, the unconfined compressive strength (UCS) of paste cubes was tested at 3, 7 and 28 days, respectively. For each group of geopolymer paste, six samples were used to test the density through the Archimedes method in water. Then these samples were dried and tested under compression at a loading rate of 0.6 MPa/s in accordance with ASTM C109/C109M [38]. The density and UCS reported in this study were the average values of six specimens.

##### **3.1.2 Isothermal calorimetry**

Isothermal calorimetry was used to measure the hydration heat of four geopolymer paste groups according to ASTM C1679 [38]. To figure out the role of RM and FGR, the test was continued for 120 hours. Due to the equipment limitation, mixing could only be conducted outside the instrument and the mixed pastes were then quickly introduced into the calorimeter. The starting time on the plots represented the time when the samples were placed in the instrument, which was around 2 minutes after the start of mixing.

##### **3.1.3 XRD test**

The broken pieces of the heat-cured geopolymer pastes after compression test at 28 days were immersed in isopropanol for 7 days to stop further hydration of pastes. After that samples were dried in a vacuum freezer for 6 hours. The dried fractions were ground into powder form with sizes less than 45  $\mu\text{m}$  using a mortar and pestle. The equipment used in the XRD test for both waste materials and geopolymer pastes was Rigaku SmartLab 9kW-Advance. The XRD test

was conducted in a  $2\theta$  range from  $10^\circ$  to  $60^\circ$  with a scanning step of  $0.02^\circ$ .

#### **3.1.4 Thermogravimetric analysis**

The sample preparation for Thermogravimetric analysis (TGA) test was the same as that for XRD. A dried powder sample (around 10 mg) with particle size under  $45\ \mu\text{m}$  was used for each mix. TGA was performed using Rigaku Thermo Plus EVO2 and the temperature was gradually increased from  $30^\circ\text{C}$  to  $1000^\circ\text{C}$  with a heating rate of  $10^\circ\text{C}/\text{min}$ .

#### **3.1.5 Infrared spectroscopy measurement**

The sample preparation method for Fourier Transform Infrared Reflection (FTIR) test was the same as that for XRD and TGA. Around 5 mg dried powder with size under  $45\ \mu\text{m}$  was tested for each mix. In this study, a Bruker Tensor II spectrometer was used to conduct the FTIR test. The scanning was conducted by Attuned Total Reflection (ATR) method in the range of wavenumbers from 400 to  $4000\ \text{cm}^{-1}$  with a resolution of  $4\ \text{cm}^{-1}$ .

#### **3.1.6 Heavy metal tests**

Raw RM and FGR were digested through aqua-regia digestion and the total heavy metal concentration was detected using the Inductively Coupled Plasma-Optical Emission Spectrometer (ICP-OES).

The leaching test was performed on raw RM and FGR as well as four heat-cured geopolymer pastes. The paste samples were crushed to a size range under 4 mm and dried at  $105^\circ\text{C}$ . Two different leaching methods were used to investigate and compare the leaching behavior. One leaching test was the toxicity characteristic leaching procedure (TCLP) in line with US EPA Method 1311 [39]. The other test was the synthetic precipitation leaching procedure (SPLP) according to the US EPA Method 1312 [40]. The difference between these two tests was only the extraction liquid used. The TCLP test generally aims to examine the feasibility for soil landfilling or remediation while the SPLP test explores the leaching behavior in simulated acid rain. Liquid after leaching tests was filtered through the  $0.45\ \mu\text{m}$  glass fiber filter and then digested by nitric acid. The detailed procedures of these tests can be found in previous studies

[18, 41]. The leaching concentrations were also determined by the ICP-OES.

## 3.2 Test methods of GPA

### 3.2.1 Shape index, particle density and water absorption

The shape index was tested as per BS EN 933-4 [42]. Approximately 1500 g GPA with the diameter between 5 mm and 10 mm were soaked in water for 24 hours, then taken out and wiped with a dry towel to SSD condition. The mass of SSD GPA was denoted as  $m_1$ . After that, the GPA were put into a measuring glass bottle. Next, water was carefully poured into the bottle until no bubbles were found between the water surface and a glass cover. The total weight of the water, GPA, bottle and glass cover was recorded as  $m_2$ . The weight of the bottle filled with water as well as the glass cover was marked as  $m_4$ . The measured GPA were taken out and dried in an oven at 105°C until no further mass loss was observed. The mass of the oven-dried GPA was  $m_3$ . The oven-dried particle density  $\rho_{OD}$  (unit: kg/m<sup>3</sup>) can be calculated according to **Eq. (1)**, as follows:

$$\rho_{OD} = \frac{m_3}{m_1 - (m_2 - m_4)} \times 1000 \quad (1)$$

Additionally, the water absorption  $WA$  of the crushed GPA can be calculated by **Eq. (2)**:

$$WA = \frac{m_1 - m_3}{m_3} \times 100\% \quad (2)$$

Based on the oven-dried particle density and water absorption of GPA, the calculation of SSD particle density  $\rho_{SSD}$  is shown in **Eq. (3)**:

$$\rho_{SSD} = (1 + WA) \rho_{OD} \quad (3)$$

### 3.2.2 Bulk crushing strength

The bulk crushing strength of GPA was determined in accordance with GB/T 17431.2-2010 [43], which was similar to the method in EN 13055:2016 [44]. A steel cylinder container with an inner bottom area of 10000 mm<sup>2</sup> and inner height of 100 mm was used. A circular loading head with the same area of 10000 mm<sup>2</sup> was placed on the top of leveled GPA surface. A

compression load was applied with a loading rate of 0.3 kN/s until the compression of GPA reached 20 mm from the top surface. The ultimate load was recorded as  $F_1$  (unit: N). The bulk crushing strength ( $\sigma$ ) of GPA (unit: MPa) was calculated by **Eq. (4)**.

$$\sigma = \frac{F_1 + F_2}{A} \quad (4)$$

where  $F_2$  was the gravity of the loading head (unit: N), which was 17 N in this test; and  $A$  was the inner area of the cylinder container (10000 mm<sup>2</sup>).

### **3.3 Test methods of GAC**

#### **3.3.1 Workability, density and compressive behavior**

After mixing, the slump of concrete was tested and the value was taken as the nearest 5 mm, according to BS EN 12350-2 [45]. The strength development of GAC was studied by testing the UCS of GAC samples at 3, 7 and 28 days, respectively. Before the compressive test, the density at 28 days was measured first by the Archimedes method. The compressive test followed the ASTM C39/C39M-20 [46]. The elastic modulus test was performed at the age of 28 days according to ASTM C469/C469M-14 [47]. For the elastic modulus test, surface capping by gypsum on the top and bottom was performed for all the cylinder specimens. Three longitudinal strain gauges were fixed on each specimen with a length of 5 mm to measure the strain. During the test, the loading rate was set as 0.6 MPa/s.

#### **3.3.2 Microstructure characterization**

Some crushed fragments after the compressive strength were collected and further reduced into small pieces of 4-8 mm in size, where the aggregate-matrix interface could be observed. Samples were immersed into isopropanol for 7 days to stop the hydration process and then dried in vacuum freezer. Then, the samples were embedded into fresh epoxy resin and vacuumed to remove the air bubbles. After the setting of epoxy resin, the samples were polished to a smooth surface as explained in the author's previous study [18], and coated with the gold sputter. The micromorphology and element distribution of the aggregate-matrix interface was investigated by Tescan VEGA3 with an energy dispersive X-ray spectroscopy (EDS) detector.

## 4 Results

### 4.1 Geopolymer Paste

#### 4.1.1 Density and compressive strength

Fig. 2a and Fig. 2b depict the UCS of the synthesized geopolymer pastes under ambient and heat curing conditions, respectively. For ambient-cured samples, all four groups had a comparable UCS at 28 days. However, for heat-cured samples, the control group obtained the highest UCS (68.82 MPa) at 28 days and RM0.4 obtained slightly (3%) lower strength compared with the control group. The UCS of heat-cured FGR0.24 and FGR0.32 further decreased by 5.3% and 7.0% respectively compared with heat-cured RM0.4. Under ambient curing, FA, RM and FGR dissolved slowly at the early age and the dissolution increased with the curing time, which explained the comparable UCS to the control group at 28 days. However, under heat curing, more FA particles may be activated because of higher amorphous degree in FA than RM. The potential of FA was stimulated under heat curing and the differences between the RM-included samples and the control group were enlarged. It can also be found that the difference in UCS between the two FGR included samples was marginal. It was also observed that the UCS of heat-cured samples were higher than their corresponding ambient-cured ones at all curing ages. This has been commonly reported for geopolymer materials because high temperature could accelerate the reaction degree at the early age [48-50]. In addition, the UCS of ambient-cured pastes substantially increased with the curing time while the UCS of heat-cured pastes could achieve 75%-90% at 3 days of their 28-day UCS and the followed strength development was marginal.

Fig. 2a and Fig. 2b also show the 28-day dry density of all four groups under ambient curing and heat curing, respectively. The RM0.4 group had the largest density among the four groups regardless of curing condition due to the highest specific gravity of RM. Meantime, the control group had the lightest density due to the smallest specific gravity of FA. Compared with RM0.4, the density of FGR0.24 and FGR0.32 decreased with the addition of FGR. The reason may be that sodium metasilicate was completely dissolved in water before mixing with precursors



343 while the FGR particles occupied the volume at the beginning while dissolved and reacted at  
 344 later curing time. However, the initial space that FGR particles originally occupied remained,  
 345 hence the density was found to be decreased. Besides, another reason could be that the total  
 346 percentage of FGR and sodium metasilicate in the FGR-added groups (13.32% in FGR0.24 and  
 347 13.76% in FGR0.32) was slightly higher than that in RM0.4 group (12%) as listed in the mix  
 348 proportions (Table 2).

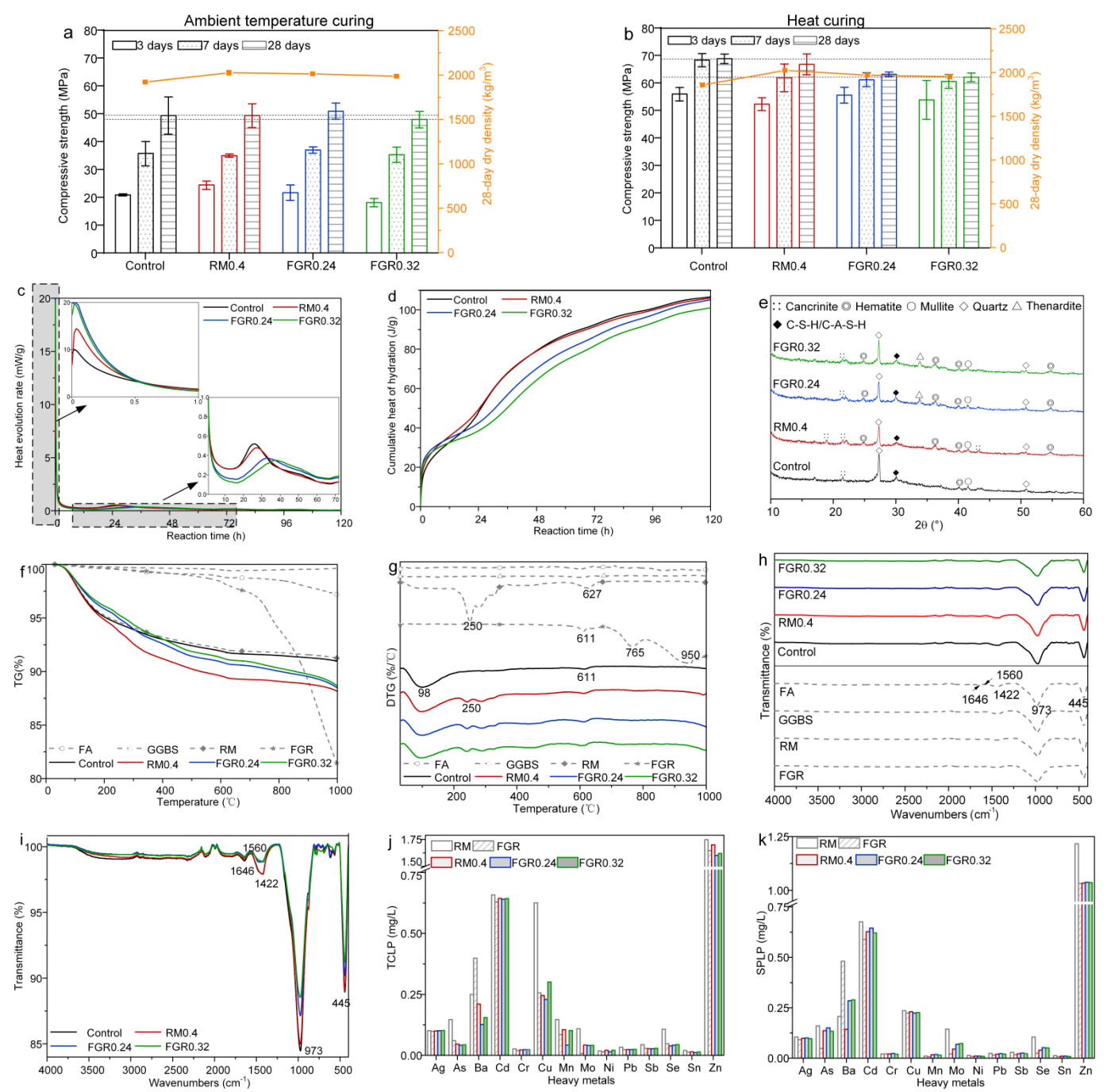


Fig. 2 Compressive strength and dry density of geopolymer pastes at 28 days: (a) Ambient temperature curing, (b) Heat curing, (c) Heat rate of evolution and (d) Cumulative heat release of geopolymer pastes, (e) XRD patterns of geopolymer pastes, (f) TG analysis and (g) DTG analysis of raw materials and geopolymer pastes, (h) FTIR of raw materials and geopolymer pastes and (i) Enlarged FTIR curves for geopolymer pastes, Leachability of heavy metals from raw materials and geopolymer pastes (j) TCLP and (k) SPLP. (XRD, TGA, FTIR and leaching tests were only performed for the heat-cured samples)

#### 4.1.2 Heat of evolution

The heat evolution rate and the cumulative heat during the first 120 hours were recorded as presented in Fig. 2c and Fig. 2d, respectively. All the results were normalized to the thermal power per gram of total precursors. In Fig. 2c, the first exothermic peak within the first hour was attributed to the wetting and chemical dissolution of the precursors and the precipitation of the dissolved silicate-aluminate. It was evident that the three RM-contained groups had higher peak intensity than the control group. This was also observed in previous studies [51], which was explained by a higher solubility of  $\text{Al}_2\text{O}_3$ ,  $\text{SiO}_2$  and  $\text{CaO}$  in a higher  $\text{NaOH}$  concentration due to the high alkalinity of RM. With further replacement by FGR, the two FGR-added groups presented higher peak values. This was because the wetting and dissolution of FGR released more heat. Accordingly, at the beginning stage the cumulative heat release amount of the control sample was the lowest (Fig. 2d).

The second peak, which also occurred commonly in geopolymer materials, was due to the nucleation, growth, and precipitation of reaction products. Compared to the other three groups, the second peak of the control group was the highest and its peak crest was also the earliest. RM0.4 had the second highest and earliest peak. This phenomenon may be explained by that RM reacted slower than FA, which was consistent with the above analysis for UCS results. While the FGR-added groups had delayed and the lowest peak intensities, which can be associated with the fact that a part of  $\text{Na}_2\text{O}$  in FGR was not able to participate in the reaction at the early stage. Moreover, the presence of impurities and heavy metals can also slow down the reaction of FGR in the geopolymer system. As a result, the total heat of the control group was the highest while the total heat of the RM0.4 was the second highest. The total heat of FGR0.32 was the lowest among all pastes and this trend was also consistent with the UCS results at 28

days.

#### **4.1.3 XRD patterns**

The phases in the geopolymer samples were characterized by XRD analysis and the results are displayed in Fig. 2e. For simplicity, only the heat-cured samples were analyzed in the following sections in this study. The XRD pattern of the control group showed peaks such as hematite, mullite, quartz, which were unreacted phases inherited from raw FA. Besides, the hump peak around  $29.5^\circ$  indicated the co-existence of poorly disordered C-S-H gels and calcite crystals due to the high content of CaO in GGBS [32]. It should be mentioned that the geopolymer gel was hard to identify through the XRD pattern due to its amorphous nature and coexistence with the other crystalline phases [52], however, the existence of geopolymer gel will be shown in the following sections. By comparing with the XRD patterns of FA and GGBS in Fig. 1b, a new crystalline phase namely cancrinite was identified, which has also been reported in the literature and was related to geopolymer paste [53].

For XRD patterns of samples with RM or FGR, the identified peak numbers were much lesser than those in raw RM and FGR as shown in Fig. 1b. Some phase peaks declined significantly such as quartz compared with raw RM. This can be associated with the reaction of some phases in RM and FGR under a highly alkaline environment. Compared with the control group, the number of hematite peaks was more in the three RM-added groups. It is interesting to mention that the three RM-added groups had no significant differences, which revealed that the FGR could effectively serve as an activator in this study. In addition, thenardite was found in the two FGR-included groups, which could act as weak alkaline activators as reported in previous research [54]. These XRD results agreed with the UCS results.

#### **4.1.4 Thermogravimetric analysis**

The thermal stability of raw materials and geopolymer pastes was analyzed by the thermal gravimetry (TG) and derivative thermal gravimetry (DTG) curves as displayed in Fig. 2(f-g). The mass loss values for GGBS and FA from  $30^\circ\text{C}$  to  $1000^\circ\text{C}$  were only 0.38% and 2.81%, respectively. Similar mass loss values were reported in literature [55, 56]. The mass loss for

406 RM from 30°C to 1000°C was higher (8.75%). Similar amounts of mass loss were also observed  
 407 in recent studies [8]. The mass loss of RM was initiated from the beginning of heating. DTG  
 408 curve of RM showed two main distinct peaks around 250°C and 627°C. The DTG peak around  
 409 250°C corresponded to gibbsite and goethite [57, 58], which were also confirmed in XRD  
 410 patterns. The DTG peak between 600-900°C was due to the release of CO<sub>2</sub> during the  
 411 breakdown of calcite. The mass loss of FGR was the highest (18.61%), which was similar to  
 412 the values reported in previous research [59, 60]. The mass of FGR was quite stable up to 600°C  
 413 and a sharp decrease was observed beyond that. DTG curve of raw-FGR showed three distinct  
 414 peaks around 611°C, 765°C and 950°C. The first and second DTG peak was due to the  
 415 decomposition of calcite. The third peak may be attributed to the melting and evaporation of  
 416 soluble complex Na, K and Cl salts as well as the decomposition of the sulfate phases [60].

417 From Fig. 2f, it is evident that the total mass loss of the control group was the lowest (9.04%)  
 418 and the loss of RM0.4 was the highest (11.83%). The mass losses of two FGR-added samples  
 419 were similar to the RM0.4 samples (FGR0.24: 11.52%; FGR0.32: 11.32%). The reason will be  
 420 explained after the illustration of DTG curves. From Fig. 2g, a broad peak between 30 °C and  
 421 180 °C was distinct in all the geopolymers, which indicated the evaporation of free water  
 422 initially at lower temperatures and physically bound water at higher temperature up to 180°C  
 423 [61, 62]. The control sample and the RM0.4 sample had a comparable rate of mass loss between  
 424 30 °C and 180 °C and slightly higher than the two FGR-added samples, which was in line with  
 425 the trend of UCS. At around 611°C, a small peak representing the decomposition of calcite  
 426 appeared in all four groups. Except for the control group, all the other three groups exhibited  
 427 similar trends and had a peak around 250 °C, which was due to the gibbsite and goethite in RM.  
 428 It is interesting to mention that this peak intensity was smaller than that of the raw RM. On one  
 429 hand, it was because the coexistence of other raw materials reduced the intensity. On the other  
 430 hand, part of RM particles reacted. This also explained the highest mass loss of RM0.4 sample.  
 431 It is important to mention that not all the DTA peaks of raw FGR were present in the FGR-  
 432 added geopolymers, which could be associated with the high reaction degree of FGR in the  
 433 geopolymer. The nearly complete reaction of FGR could explain the similar mass loss compared

with RM0.4.

#### 4.1.5 Infrared Spectroscopy

All FTIR curves for raw materials and geopolymer pastes are displayed in Fig. 2h. The FTIR curves of four geopolymer pastes are enlarged in Fig. 2i for clearer illustration. Two wavelength bands related to the tetrahedral  $\text{AlO}_4$  and  $\text{SiO}_4$  groups were observed in both FA and GGBS ( $447\text{ cm}^{-1}$  and  $1054\text{ cm}^{-1}$  in FA;  $495\text{ cm}^{-1}$  and  $911\text{ cm}^{-1}$  in GGBS). The shift of the larger band was attributed to the higher amorphous phase in GGBS [63]. Two typical bands in the RM were observed at  $984\text{ cm}^{-1}$  and  $433\text{ cm}^{-1}$  which were identified as quartz and hematite phases [64]. Three major bands in the FGR were found at  $1110\text{ cm}^{-1}$ ,  $615\text{ cm}^{-1}$  and  $450\text{ cm}^{-1}$ , which were linked to the stretching and bending vibration of  $\text{SO}_4$  group [18].

For the control sample, characteristic bands detected at  $\sim 973\text{ cm}^{-1}$  corresponded to Si–O–T (T: tetrahedral Al or Si) and peak at  $\sim 445\text{ cm}^{-1}$  was associated with the bending vibration of Si–O–Al [65]. These peaks were associated with the presence of geopolymer products and their intensities revealed the degree of geopolymerization, which confirmed a similar geopolymerization degree in the control sample and RM0.4 sample while the lower degree in the two FGR-added samples. The absorption bands at  $\sim 1422\text{ cm}^{-1}$  corresponded to stretching vibration of C–O which reflected the presence of carbonate in geopolymer. The two FGR-added groups had lower intensities at this band, which indicated the less carbonate products. The broad bands between  $3680\text{ cm}^{-1}$  and  $3000\text{ cm}^{-1}$  as well as a weak absorption peak at  $\sim 1646\text{ cm}^{-1}$  were due to the stretching vibration of O–H bond and H–O–H bending vibrations of interlayer adsorbed water, respectively [66]. The peaks in the FTIR curves of raw FA, GGBS and RM were not obvious in the curves of geopolymer pastes because these peaks were close to the peaks of geopolymer products. For the samples containing FGR, peaks around  $1110\text{ cm}^{-1}$  and  $615\text{ cm}^{-1}$  present in raw FGR were also not found due to its participation in the geopolymerization.

#### 4.1.6 Heavy metal leaching

Leaching concentrations of different heavy metals determined from TCLP and SPLP methods

are provided in Fig. 2(j-k). It should be mentioned that all the leached values of all geopolymer samples were lower or similar to that of raw FGR. However, the concentrations of leached heavy metals from all the paste samples by TCLP and SPLP tests were far below the regulatory limits advised by the EPA TCLP regulatory limits (Ag, As, Cr, Pb, and Cr: 5 mg/L; Cd: 1 mg/L) [67]. GPA were designed to be used in concrete and the leached values are expected to be further reduced significantly, when GPA is encapsulated inside concrete. The leached values of geopolymer samples incorporating RM were reduced compared with the raw RM samples. This indicated that geopolymer shows high immobilization efficiency for the RM waste as compared to FGR [68].

## **4.2 GPA**

### **4.2.1 GPA properties**

From the UCS results of geopolymer pastes in Section 4.1, FGR0.32 group utilized the highest amounts of waste materials (RM and FGR) while maintaining a slightly lower UCS compared to the other three samples. Besides, the heat-cured samples obtained higher strength than their corresponding ambient-cured samples. As demonstrated by previous studies, early heat curing was effective to mitigate the shrinkage problem of geopolymer [69, 70]. Therefore, the heat-cured FGR0.32 mix was selected to manufacture the GPA. The cubic paste specimens were crushed into GPA before heat curing to minimize the crushing energy for GPA production. The heat curing condition for GPA was the same as that for the pastes. According to the UCS results, the 7-day UCS of heat-cured FGR0.32 could reach 97% of their 28-day UCS, hence the crushed GPA at 7 days were used for casting concrete. The bulk crushing strength of GPA and NA was measured as 9.2 MPa and 13.6 MPa, respectively. The strength of GPA in this study was higher than that of most cold-bonded artificial aggregates in literature as displayed in Table S1. This was due to their different raw materials, mix proportions and curing conditions. Moreover, as indicated by Table S1, a smaller aggregate size range could lead to a higher bulk crushing strength due to size effect and more flaws in bigger particles [71, 72]. The particle size distributions of GPA and NA in this study meet the grading requirements in GB/T 17431.2-

2010 [43] as shown in Fig. 1a. A slightly smaller shape index of GPA (2.05) than that of NA (2.12) was found. The average values of oven-dried particle density and water absorption of the GPA were measured to be 1620.5 kg/m<sup>3</sup> and 24.38%, respectively, which were also similar to those values in Table S1. According to Eq. (3), the SSD particle density was calculated as 2015.6 kg/m<sup>3</sup>.

#### 4.2.2 Cost and carbon emissions of GPA

The material cost and carbon emissions of GPA in this study were calculated as listed in Table 3. All of the material costs and carbon emissions are provided based on the laboratory supply list and previous research [32]. Two types of GPA with the best mechanical performances [GPA1 and GPA2] or another two types of GPA with the highest waste utilization [GPA3 and GPA4] were selected as references. Here the use of RM, FGR and incineration bottom ash were assumed to require zero cost and generate zero carbon footprint because it consumes hazardous solid waste that brings a positive effect on the environment. The results showed that the GPA in this study have the lowest cost (465.0 HKD/metric ton) and carbon emission (0.122 kgCO<sub>2</sub>e/kg) among the four types of GPA mainly due to the decreased dosage of commercial activator. The greatest concern of geopolymer in terms of cost and environment is the activator. However, this issue can be mitigated in this study because the use of FGR can reduce the commercial activator dosage up to 32%. Moreover, the cost and carbon emission of GPA was normalized by the cubic compressive strength of their pastes. Obviously, the cost and embodied carbon per MPa of GPA in this study were also the lowest benefitted by the highest cubic strength of pastes. The current price of NA is around 160 HKD/metric ton in the market. The carbon emission of the crushing processes for NA was around 0.00618 kgCO<sub>2</sub>e/kg [73]. However, the crushing energy of GPA is lower than that of NA due to the their lower compressive strength when being crushed, hence resulting in lower carbon emissions. The cost and carbon emission for the heat curing were estimated based on the energy consumption [73] and the result was 849.6 HKD/metric ton and 0.164 kgCO<sub>2</sub>e/kg, respectively. However, this part of energy could be replaced by the waste heat from incineration plants. Besides, heat curing could improve the production efficiency, thus compensating the cost and emissions.

**Table 3** Costs and carbon emissions of GPA

Raw Materials	Cost (HKD/metric ton)	Embodied carbon (kgCO <sub>2</sub> e/kg)	GPA Mix Proportion (metric ton)				
			GPA0 (This study)	GPA1 [32]	GPA2 [32]	GPA3 [73]	GPA4 [73]
FA	400	0.008	0.3391	0.6849	0.5634	0.5929	0.5714
GGBS	450	0.083	0.1412	-	0.1408	0.1071	0.1071
RM	0	0	0.2260	-	-	-	-
FGR	0	0	0.0396	-	-	-	-
Incineration bottom ash	0	0	-	-	-	0.0143	0.0357
Na <sub>2</sub> SiO <sub>3</sub> -Anhydrous	4600	1.860	0.0576	0.1096	0.0845	0.0714	0.0714
Water	4.5	0.001	0.1964	0.2055	0.2113	0.2143	0.2143
GPA Cost (HKD/metric ton)	-	-	465.0	776.3	678.4	614.75	606.16
GPA Embodied carbon (kgCO <sub>2</sub> e/kg)	-	-	0.122	0.210	0.174	0.147	0.146
Cubic Strength (MPa)	-	-	62.1	62.2	57.3	44.5	36.4
GPA cost per MPa (HKD/metric ton/MPa)	-	-	7.49	12.48	11.84	13.81	16.65
GPA embodied carbon per MPa (kgCO <sub>2</sub> e/kg/MPa)			0.0019	0.0034	0.0030	0.0033	0.0040

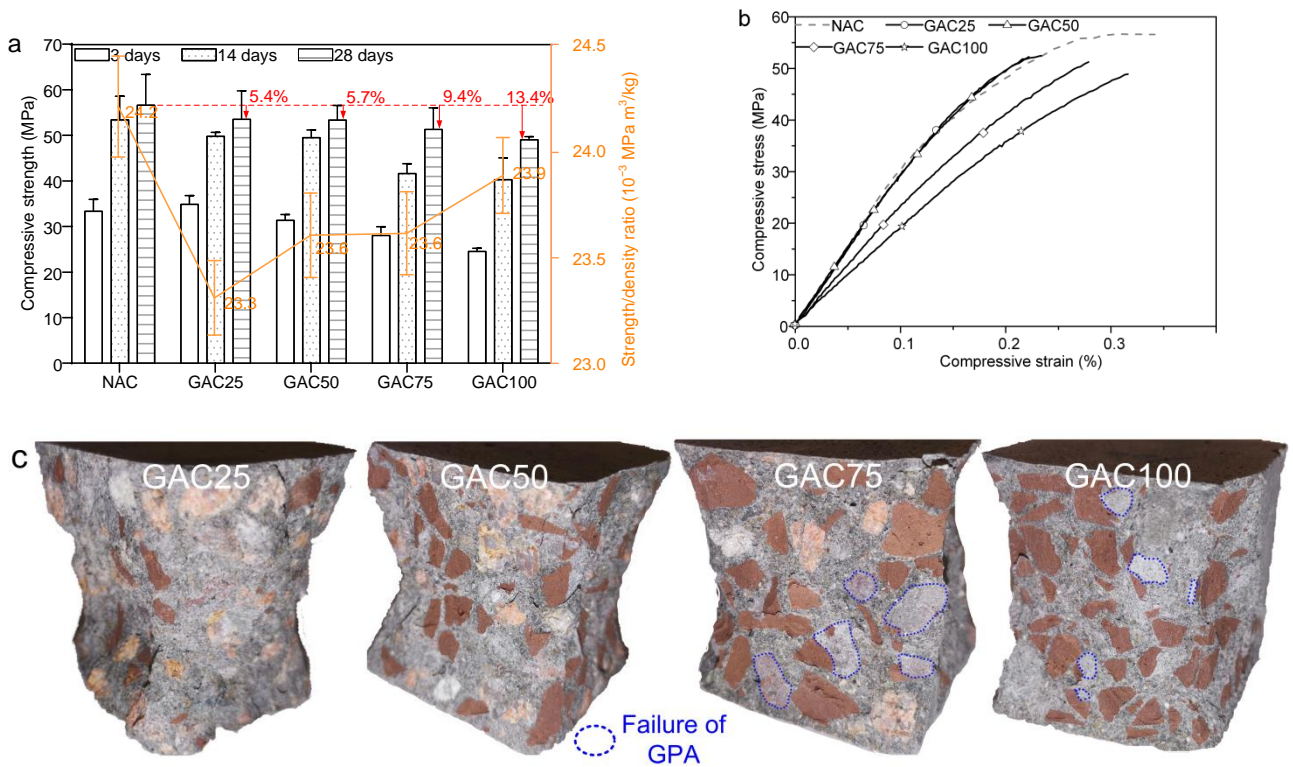
## 517 4.3 GAC

### 518 4.3.1 Slump, density and Compressive Strength

519 The slump of NAC, GAC25, GAC50, GAC75 and GAC100 was measured as 150, 155, 160,  
520 170 and 180 mm, respectively, which met the requirements of flowability [45]. The slump  
521 increase with the volume of GPA was due to the lighter density of GPA. The UCS development  
522 and strength/density ratio at 28 days as well as the stress-strain curves of GAC are plotted in  
523 Fig. 3a and Fig. 3b, respectively. The 28-day density of GAC reduced from 2413 to 2082 kg/m<sup>3</sup>  
524 with the increased replacement level of GPA from 0% to 100% due to the lightweight feature  
525 of GPA. The maximum reduction of density reached 13.7% compared with NAC. It can be



526 observed from Fig. 3a that although the strength/density ratios of all GAC groups were lower  
 527 than that of NAC, the strength/density ratio was increased with the replacement ratio of GPA.  
 528 The strength/density ratio ( $23.9 \times 10^{-3} \text{ MPa m}^3/\text{kg}$ ) of GAC100 in this study was slightly higher  
 529 than the highest ratio ( $22.7 \times 10^{-3} \text{ MPa m}^3/\text{kg}$ ) in previous GAC research according to a recent  
 530 review [26, 32]. The incorporation of GPA decreased the concrete strength. The decrease in  
 531 strength was only 5.4% and 5.7% at 25% and 50% replacement levels, respectively. However,  
 532 a slightly higher reduction in strength was observed for the 75% and 100% replacement level  
 533 which was 9.4% and 13.4%, respectively. It was also reported in past studies that artificial  
 534 aggregates could reduce concrete strength [74]. As can be seen from Fig. 3b, the elastic modulus  
 535 of GAC25 and GAC50 was similar to that of NAC (31.4 GPa). However, the elastic modulus  
 536 of GAC75 and GAC100 decreased to 23.8 GPa (24.2% reduction) and 19.2 GPa (38.9%  
 537 reduction), respectively. The reduction of elastic modulus was attributed to the lower elastic  
 538 modulus of GPA compared with NA [74].



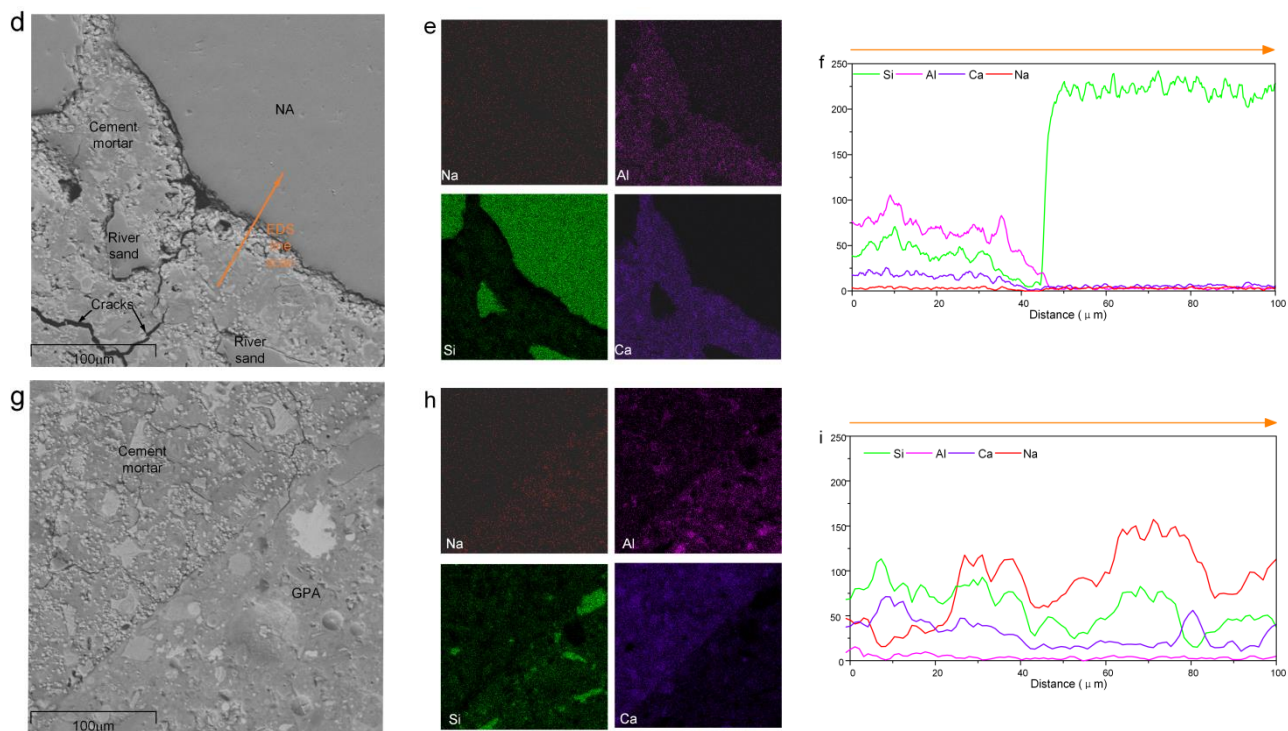


Fig. 3 (a) Compressive strength development and strength/density ratio, (b) Stress-strain curves of concrete, (c) Failure mode of GAC, (d)-(f): BSE image of NA-matrix interface, elemental mapping and EDS line scanning, respectively, (g-i): BSE image of GPA-matrix interface, elemental mapping and EDS line scanning, respectively.

Fig. 3c displays the failure modes of four GAC groups. Generally, interface failure is common for NAC, and hence such failure mode is not shown here. For artificial aggregate concrete, aggregate failure was considered as the primary failure mode [32]. However, the combination of interface failure and aggregate failure was observed for the GAC as marked by the blue dotted circles. Such interface failure in GAC was more evident in GAC75 and GAC100, which indicated that the strength of partial GPA was higher than that of surrounding cement mortar.

#### 4.3.2 Microstructure analysis

The aggregate-matrix interfaces between GPA or NA and cement mortar were further investigated by microscopic analysis in the Backscattered-Electron (BSE) mode and element mapping as shown in Fig. 3(d-e) or Fig. 3(g-h). EDS line scan was also conducted at the interface (along an orange line) which passed through GPA or NA and cement mortar as displayed in Fig. 3f and Fig. 3i. The boundary between cement mortar and NA could be distinctly identified according to the BSE image and the sudden change of silicon element. In

comparison, the interface between cement mortar and GPA was denser than that between the mortar and NA as observed in the BSE image and elemental mapping. As shown in Fig. 3d and Fig. 3g, lower number of micro-cracks with narrow width were found along the GPA-matrix interface compared with the NA-matrix interface, indicating an excellent GPA-matrix bond in GAC. In addition, the element contents at the interface zone between GPA and cement mortar were higher than that between NA and cement mortar (Fig. 3f and Fig. 3i), which could be due to the chemical reaction between GPA and cement mortar [50].

## 5 Discussion

In this study, red mud (RM) and flue gas residues (FGR) were utilized as partial replacements of common precursors and commercial alkaline activator respectively to synthesize geopolymer aggregates (GPA), aiming to recycle as much industrial wastes as possible into one product. It was found that the geopolymer pastes incorporating RM (40% mass replacement of FA) and FGR (24% or 32% mass substitute of sodium metasilicate) could maintain a similar compressive strength level to the control samples. The lesser identified peaks and lower intensity of some peaks in geopolymer pastes compared with raw RM and FGR from XRD, TGA and FTIR curves as shown in Fig. 1b and Fig. 2(e-i), which revealed that RM and FGR have participated in the geopolymer reaction. Besides, the high alkalinity of RM revealed by the higher first peak in heat evolution curves (Fig. 2c) may also have positive effect on the reaction [11].

On the other hand, owing to many kinds of crystal phases existing in RM as shown in Fig. 1b, most of the  $\text{SiO}_2$  and  $\text{Al}_2\text{O}_3$  in raw RM were difficult to be dissolved. Previous studies have indicated that mechanical activation or thermal or alkali-thermal pre-treatment could improve their dissolution efficiencies [12, 75-78]. However, raw RM was directly used as precursor to save cost in this study. Hence the dissolution degree of  $\text{SiO}_2$  and  $\text{Al}_2\text{O}_3$  was low. Besides, Fe in raw RM was mainly presented in hematite or octahedral  $\text{Fe}^{3+}$  form, which were also difficult to participate in gel generation [12, 75]. These unreacted RM particles would act as fillers. Various crystal phases were also detected in FGR as displayed in Fig. 1b and some phases were remained in the produced geopolymer pastes as shown in Fig. 2e. Moreover, the lower peak

intensities in FTIR curves also indicated the incomplete participation of Na<sub>2</sub>O in FGR. In addition, the dissolution and reaction of RM and FGR demanded time as revealed by the heat evolution rate as plotted in Fig 2(c-d). Moreover, the presence of impurities and heavy metals as described in Section 4.1.6 may also slow down the reaction of RM and FGR in the geopolymer system. It should be mentioned here that despite the incomplete reaction of RM and FGR, their produced geopolymer maintained a comparable performance. In the future, the possibility of increasing the utilization ratio of RM and FGR can be further investigated.

As observed from Fig. 3(d-f), although the mechanical property of GPA was comparatively weaker than that of NA, the enhanced aggregate/matrix interaction (due to chemical reaction) in GAC could produce a thick ITZ between GPA and the cementitious matrix. Therefore, a considerable decrease in the strength of GAC was not observed even at 100% replacement level of natural aggregate. The strength/density ratio of GAC was lower than that of NAC. Hence, the structure efficiency of GAC still has space to be improved in future study.

## 6 Conclusions

This study has confirmed the feasibility and good performance of GPA using RM and FGR as partial substitutes of traditional precursors and commercial activator, respectively. The detailed conclusions are as follows:

- Geopolymer pastes produced with 40% mass of FA replaced by RM and 32% Na<sub>2</sub>O in commercial activator replaced by FGR showed only a slight decrease in compressive strength.
- The reaction mechanism through heat evolution, XRD, TG and FTIR investigations have demonstrated that RM could effectively participate in the chemical reaction along with the nearly complete dissolution of FGR. However, reaction rate of RM and FGR was lower at the early stage.
- TCLP and SPLP tests revealed that the leaching concentrations of all measured heavy metals in GPA were far below the limits. The cost and carbon emissions per MPa of GPA in this study were the lowest compared with other GPA in literature.
- When GPA were used to prepare concrete, GAC could achieve a similar strength to NAC

and had an improved ITZ compared to NAC as observed by the failure patterns of samples and BSE-EDS analysis. The elastic modulus of GAC did not show significant changes when replacement level of GPA was less than 50% GPA, while it decreased after percentage of GPA was increased over 50%. In the future, the durability properties of GAC should be further studied in the future.

#### **Conflict of interest**

The authors declare that they have no conflict of interest.

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#### **CRedit Author Statement**

**LP Qian:** Conceptualization, Methodology, Investigation, Visualization, Writing - Original Draft. **MR Ahmad:** Investigation, Methodology, Validation, Writing - Review & Editing. **JC Lao:** Investigation, Visualization, Writing - Review & Editing. **JG Dai:** Conceptualization, Funding Acquisition, Supervision, Writing - Review & Editing.

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