

1                   **RESEARCH AND APPLICATION OF PERVIOUS CONCRETE AS A**  
2                   **SUSTAINABLE PAVEMENT MATERIAL: A STATE-OF-THE-ART AND**  
3                   **STATE-OF-THE-PRACTICE REVIEW**

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17                   **ABSTRACT**

18                   Pervious concrete (PC) has gained renewed interest in the past decade due to its positive  
19                   environmental impacts. Extensive research employing a variety of strategies has been conducted to  
20                   improve the overall performance of PC. Numerous literatures have been published. With the  
21                   advances in high performance pervious concrete (HPPC), widespread application of this material  
22                   has been made possible. This paper reviews the state-of-the-art and state-of-the-practice research  
23                   and application of PC. Emphasis has been laid on the pore system characterization (PSC) and its  
24                   influence on the mechanical, hydraulic and acoustical properties of PC. Among the various  
25                   applications of PC, this review focuses on its application as a sustainable pavement construction  
26                   material.

27 *Keywords:* pervious concrete; pore system characteristics; sustainable pavement; noise reduction;  
28 compressive strength

## 29 **1 INTRODUCTION**

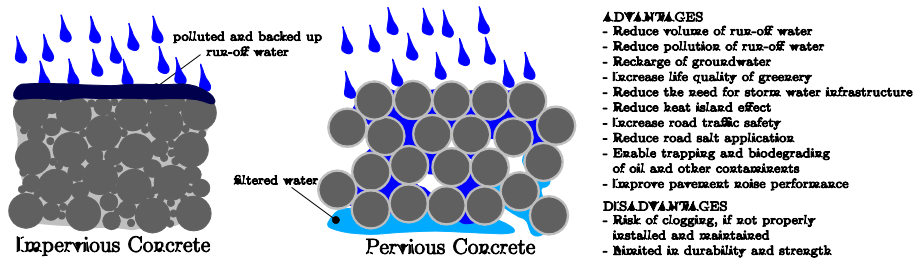
30 Pervious concrete (PC), also known as porous or permeable concrete, is a class of concrete  
31 characterized by a relative high volume of connected pores, typically in the range of 15% to 30%  
32 with pore sizes ranging from 2 to 8 mm [1], and a water permeability of about 2 – 6 mm/s [2-3].  
33 This is achieved by intentionally incorporating continuous voids through gap grading the coarse  
34 aggregate and eliminating or minimizing the usage of fine aggregate. Typical designs of PC are  
35 presented in **Table 1**. The American Concrete Institute (ACI) defines PC as “concrete containing  
36 little, if any, fine aggregate that results in sufficient voids to allow air and water to pass easily from  
37 the surface to underlying layers” [4].

38 **Table 1** Typical compositions of PC

Water to binder ratio (w/B)	Aggregate to binder ratio (A/B)	Fine sand (%)	Authors
0.26~0.33	3.2~3.7	0~15	Kevern et al. [5]
0.32~0.33	1.7~6.0	-	Deo at al. [6]
0.22~0.55	2.5~3.5	-	Zhong and Wille [7]
0.25~0.35	3.2~3.9	-	Yang [8]
0.37~0.42	2.9~4.2	-	Ghaffori and Dutta [9]
0.27~0.35	3.4~4.5	0~6.5	Huang et al. [10]
0.27~0.51	4.0~4.7	0~20	Meininger [11]

39 Although PC has been used for over 30 years, the material is attracting renewed interests recently.  
40 It is attributable to the Federal Water Pollution Control Act [12] and the Environmental Protection  
41 Agency (EPA) storm water regulations [13] that require control of both the quantity and quality of  
42 storm water runoff. The ability to allow water penetrating through its open pore structure makes  
43 PC a very effective tool to control storm water runoff. Additionally, the rapid expansion of  
44 impermeable surfaces and associated issues such as heat island effect, tire-pavement interaction  
45 noise, ground water depletion and traffic safety is another factor contributing to the increasing  
46 popularity of this material, as PC demonstrates potential to resolve these issues [14-15]. Therefore,

47 it is promoted as a construction material for parking lots and road surfaces. **Fig. 1** schematically  
48 illustrates the advantages and disadvantages of PC in comparison to conventional impervious  
49 concrete (CIC).



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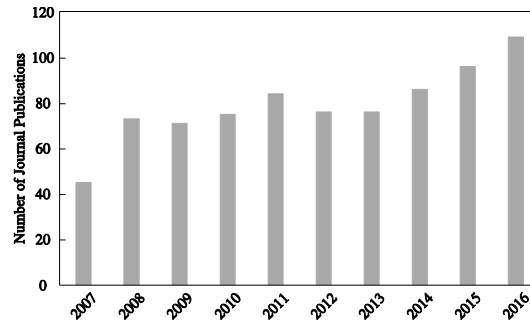
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**Fig. 1** Comparison of PC to CIC

53 **Fig. 2** shows the number of publications related to PC over the last decade. A survey of the literature  
54 indicates that this development spreads over the major continents, and is particularly intense in  
55 Europe, US and Japan. It indicates that we have entered an intense phase of research on the  
56 development of high performance pervious concrete (HPPC) and the pace of research will likely  
57 continue to accelerate. There has been a number of review articles on PC [16-18]. The purpose of  
58 this article is not to repeat these reviews, but rather to promote the concept of sustainable pervious  
59 concrete pavement (SPCP). Therefore, the literatures referenced are not meant to be exhaustive of  
60 what has been published. Specifically, this paper is aimed at highlighting the advantages and  
61 limitations of PC, to stimulate additional research to overcome current obstacles, and to generally  
62 accelerate convergence of PC technology developments that support the realization of SPCP. In  
63 light of that both structural and functional properties of PC pavement are dependent on the pore  
64 system characteristics (PSC) of PC [19-21], the review has been focused on the quantification of  
65 PSC and their influence on the mechanical, hydraulic and acoustic performance of PC. As for the  
66 structural application of PC, emphasis was laid on pavement.

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69

70 **Fig. 2** Archived journal publications on PC over the last decade (2007-2016) (Source: Google  
71 Scholar online)

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73 **2 PERVIOUS CONCRETE**

74 **2.1 PORE SYSTEM CHARACTERIZATION**

75 **2.1.1 POROSITY**

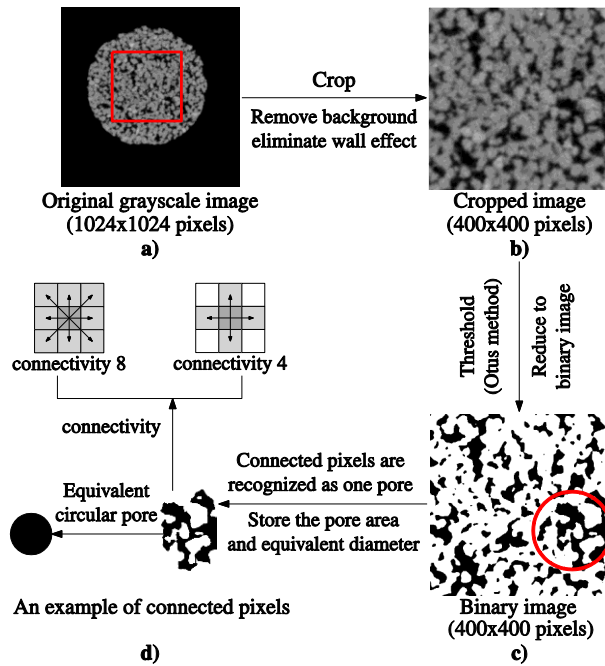
76 Porosity is one of the most important PSC. However, there is still confusion as to the definition of  
77 porosity and which type of porosity should be used to characterize PC. Not all voids in PC are  
78 effective for liquids to flow through. Some pores are isolated from others and therefore are not  
79 available for water penetration. In addition, there are smaller pores, such as capillary pores and  
80 dead-ended pores, that retain fluids due to surface tension and capillarity effect and therefore are  
81 not effective for transport of fluid. Nevertheless, these pores affect the strength of PC. Zhong and  
82 Wille suggested that the pore system in PC should be differentiated as effective porosity and total  
83 porosity [22]. Effective porosity is defined as the ratio of the volume of accessible pores (connected  
84 pores) to the total volume of the material. It is critical for the hydraulic property of PC. Total  
85 porosity is defined as the ratio of total pore volume to the total volume of material. It dictates the  
86 strength of PC. The total porosity takes into account the volume of non-accessible pores and  
87 accessible pores. The volume of non-accessible pores may be further divided into non-connected  
88 pores left in the space between aggregates and the pores in the matrix.

89 Mercury intrusion method specified in ASTM D4404 is commonly used to determine pore sizes  
90 and their distribution for CIC. However, this method is not applicable to PC due to its large pore  
91 volume and connected pore system. Therefore, a variety of methods is proposed to quantify the  
92 porosity of PC. Crouch et al. [23] modified the AASHTO T166 Standard Method and proposed a  
93 test method for the porosity of PC. This method differs from the other porosity test methods since  
94 it uses an InstroTek Corelok<sup>®</sup> System to improve the extraction of air and the penetration of water  
95 into the porous matrix of PC. It is expected that less air is entrapped in the sample and water  
96 penetrates into PC faster compared to a simple submersion approach due to the vacuum condition  
97 in the polymer bag. However, this method requires the use of a proprietary equipment. To overcome  
98 this limitation, Montes and his coworkers developed a method based on the Archimedes principle  
99 using standard laboratory equipment [24]. The porosity of PC can be calculated using **Eq. (1)**

$$P=[1-((W_D-W_S)/\rho_w)/V_T]\times 100 \quad (1)$$

100 where P is the porosity of the specimen;  $W_D$  and  $W_S$  are the dry mass and submerged mass of the  
101 specimen, respectively;  $\rho_w$  is the density of water;  $V_T$  is the total volume of the specimen.

102 Statistical determination of porosity using image analysis (IA) and statistical counts has also been  
103 proposed to quantify the porosity of PC. 16-bit greyscale X-ray planar images of a series of PC thin  
104 sections were employed by Zhong and Wille [19] to calculate the total porosity of PC. The original  
105 X-Ray images were cropped to remove the background and eliminate the wall effect (**Fig. 3**). Then  
106 the cropped images were reduced to binary images by thresholding. Otsu's method integrated in a  
107 Matlab Image Processing Toolbox was used to conduct the conversion and the porosity was  
108 calculated based on the converted binary images.



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110

**Fig. 3** Illustration of the IA process

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A similar IA approach was also employed by Akand et al. [25], Neithalath et al. [26] and Marolf

112

and his coworkers [27] to quantify porosity. Different facilities such as X-ray computed

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tomography and flatbed scanner have been used to obtain the original images. A variety of

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strategies were applied to improve the quality of the obtained images. Significant edge effect or

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irregularities were discarded [26]. The original images were cleaned by a filter and a mask was then

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applied to improve the contrast between the different phases [27]. The processed images were then

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analyzed using different software such as ImageJ<sup>®</sup> and ImagePro<sup>®</sup>. The porosity of pervious

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concrete is eventually extracted after thresholding. It is expected that the accuracy of the IA

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approach is higher than the experimental methods. Additionally, more detailed PSC can be revealed

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employing the IA approach. However, the general experimental method for porosity determination

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is easy to use. If the major parameter of interest for an application is porosity only, the experimental

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method is suggested. In contrast, if the hydraulic performance and mechanical response, both of

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which are dependent on the PSC, are concerned, IA approach may be a better option due to its

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ability to provide additional information such as pore size distribution.

## 125 **2.1.2 PORE SIZE, ITS DISTRIBUTION AND CONNECTIVITY**

126 While porosity is undoubtedly one of the most important PSC of PC, such a property alone cannot  
127 determine the performance of PC. For different PC with the same porosity, different pore size  
128 distribution and tortuosity, and thus microstructure can be present leading to varying mechanical,  
129 hydraulic and acoustic performance. Deo et al. found that the compressive response is influenced  
130 by the pore sizes, their distributions and spacing [1]. Zhong and Wille also confirmed the influence  
131 of pore size and its distribution on the compressive response of PC [19]. Rehder et al. demonstrated  
132 that although fracture toughness is primarily dependent on the porosity of PC, an increase in pore  
133 size leads to a reduction in fracture toughness at a given porosity [28]. Neithalath and his coworkers  
134 [21] reported that for a similar porosity, variations in acoustic absorption coefficient (an indicator  
135 of a material's ability to absorb acoustic energy) and permeability were observed. Zhong and Wille  
136 demonstrated that porosity is a necessary but insufficient parameter to determine the hydraulic  
137 conductivity of PC. The accuracy for the predicted hydraulic conductivity was improved once the  
138 pore tortuosity was taken into consideration for the modeling [20]. Based on these observations of  
139 performance dependency on pore size, its distribution and tortuosity, it is believed that appropriate  
140 characterization of these PSC can result in a better understanding and an enhanced prediction  
141 accuracy of the behaviors of PC and facilitate optimum mixture design for desired performance.

### 142 **2.1.2.1 IMAGE ANALYSIS**

143 Due to the random nature and spatial complexity of the microstructure of PC, it has been either  
144 over simplified or ignored in a variety of empirical models for the performance prediction of PC.  
145 Mathematical knowledge and techniques have been developed and actively used to analyse and  
146 characterize the microstructure of porous materials [29]. Stereology combined with IA, can be a  
147 powerful tool for inferring PSC from two-dimensional analysis. Recently, IA has been successfully  
148 adopted by many researchers to capture the details of the pore structure of PC. Digital images of  
149 two dimensional thin sections are usually obtained in the form of grey-scale image either by X-ray,

150 scanning electron microscopy (SEM) or high performance scanner. These grey-scale images are  
151 then be transformed to binary images by thresholding. A threshold can then be chosen, either  
152 manually or by a threshold algorithm, so that all pixels with grey levels above this threshold are  
153 white (solid) and all below are black (pores). Once the binary images with good quality are obtained,  
154 further processing is performed to eliminate artefacts and noise. Mathematical morphology is a  
155 very powerful tool for this purpose. Eventually, different PSC can be extracted.

#### 156 **2.1.2.2 PORE SIZE**

157 Stereological, morphological and probabilistic approaches have been proposed to extract pore size  
158 of PC based on IA. Marolf and his coworkers defined a characteristic pore size ( $d_c$ ) as an  
159 approximation for the representative pore size [27]. It is calculated from the median of all the pores  
160 with a size greater than 1mm obtained from IA. Pores smaller than 1mm were excluded since  
161 incorporation of a large number of such small features always led to an unrealistic pore size. An  
162 empirical equation as shown in **Eq. (2)** was proposed to correlate the characteristic pore size to a  
163 single sized aggregate ( $D_{agg}$ ).

$$d_c = 1.44 + 0.36D_{agg} \quad (2)$$

164 Neithalath et al. proposed the concept of effective pore size ( $d_{50}$ ) based on the stereological method  
165 [26]. The equivalent diameters were calculated assuming the pores were circular once the area of  
166 each individual pore was obtained from two dimensional images. Pore size histograms and  
167 cumulative frequency distribution curves of the pore size were then produced. The effective pore  
168 size is defined as the pore size at 50% of the cumulative frequency distribution. Pore diameter  
169 derived from morphological method using two-point correlation (TPC) function or granulometric  
170 density function were also employed to characterize the pore system in PC [26]. The characteristic  
171 pore diameter ( $d_{TPC}$ ) based on TPC function is estimated according to the correlation length ( $l_{TPC}$ )  
172 as follows:

$$d_{TPC} = \frac{l_{TPC}}{1 - \phi_A} \quad (3)$$



173 where  $\phi_A$  is the pore area fraction of the image, the correlation length ( $l_{TPC}$ ) is defined as the abscissa  
 174 of the intersection point of the slope of TPC function at  $l=0$  and the horizontal asymptote at which  
 175  $l \rightarrow \infty$ . Another morphological function has also been assessed. This method involves the application  
 176 of a series of morphological opening with structuring elements (SE) of increasing size. Progressive  
 177 transformations on the same object leads to a group of objects, from which information about the  
 178 size of the original object could be inferred. For each opening, the normalized pixel size distribution  
 179 function can be calculated according to **Eq. (4)**.

$$N(k) = 1 - \frac{P_s(k)}{P_s(0)} \quad (4)$$

180 where  $P_s(k)$  is the sum of all the pixels in the image opened by the  $k$ th structuring element, and  $P_s(0)$   
 181 is the sum of all the pixels in the original image. The granulometric density function  $G(k)$  can then  
 182 be derived as:

$$G(k) = N(k + 1) - N(k) \quad (5)$$

183 The local maximum of the granulometric density function is related to the critical pore radius of  
 184 the material. Therefore, the critical pore size ( $d_{cri}$ ) obtained in this way is a potential candidate to  
 185 characterize the pore system in PC. Zhong and Wille proposed the mean pore size ( $\overline{d_p}$ ) from the  
 186 cumulative pore size distribution derived from linear-path function (LPF) [19]. The LPF is the  
 187 cumulative distribution function (CDF) of the probability of finding a line segment of length  $z$   
 188 entirely in one phase of a two phase material when randomly placed into the sample. PC can be  
 189 treated as an impenetrable equal particle system based on two assumptions: 1) aggregates used to  
 190 produce PC are single sized or gap graded; 2) the matrix covering the aggregates is relatively thin  
 191 compared to the aggregate size. Therefore, the LPF for the impenetrable equal size particle two-  
 192 phase random system was selected and the explicit formulation to calculate the mean pore size was  
 193 then derived as follows:

$$\bar{d}_p = \begin{cases} \frac{2\phi_t d_a}{3(1 - \phi_t)} & \text{for two dimensions } (D = 2) \\ \frac{\pi\phi_t d_a}{4(1 - \phi_t)} & \text{for two dimensions } (D = 3) \end{cases} \quad (6)$$

194 where  $\phi_t$  and  $d_a$  are the total porosity and the diameter of the aggregate, respectively.

### 195 **2.1.2.3 PORE SIZE DISTRIBUTION**

196 While it is typical to represent the pore system in a porous material using a single characteristic  
 197 pore size, the pore size distribution (PSD) in a porous material is also of great importance. This is  
 198 especially true for a heterogeneous porous medium such as PC. Therefore, a single value of pore  
 199 size alone might not be sufficient to characterize the material behaviour. Indeed, a variety of studies  
 200 have demonstrated that the mechanical, hydraulic and acoustic performance of PC are dependent  
 201 on the PSD of the pore system [19, 21, 30]. Mercury intrusion porosimetry (MIP) is generally used  
 202 to determine PSD of CIC. However, this method is not applicable to PC due to its large pore volume  
 203 and connected pore system. A few innovative investigations have been conducted to overcome this  
 204 obstacle [1, 19]. Based on the observation that the PSD extracted from two dimensional images is  
 205 right-skewed, Neithalath et al. suggested to use Weibull and log-normal families of distributions to  
 206 model such right skewed process [1]. A two-parameter Weibull distribution (WBD) was selected  
 207 as shown in **Eq. (7)**:

$$F(d; \alpha, \beta) = \begin{cases} 1 - \exp[-(d/\alpha)^\beta] & d \geq 0 \\ 0 & d < 0 \end{cases} \quad (7)$$

208 where  $\alpha$ ,  $\beta$  and  $d$  are the scale parameter, shape parameter and pore size, respectively. It was  
 209 selected for the following reasons: (1) it is able to cover a wide range of statistical distributions by  
 210 changing the Weibull shape parameter; (2) it can be used for samples with small size; (3) it could  
 211 provide additional flexibility when an exponential distribution might be adequate. The parameters  
 212 in the Weibull distribution were determined using probability plotting method due to its best  
 213 performance for intermediate sample sizes ( $n < 20$ ).

214 Zhong and Wille proposed a PSD based on probability theory [19]. LPF was tailored for PC as  
 215 follows:

$$L(z) = \begin{cases} \phi_t \exp\left(-\frac{3(1-\phi_t)z}{2\phi_t d_a}\right) & \text{for three dimensions } (D = 3) \\ \phi_t \exp\left(-\frac{4(1-\phi_t)z}{\pi\phi_t d_a}\right) & \text{for three dimensions } (D = 2) \\ \phi_t \exp\left(-\frac{(1-\phi_t)z}{\phi_t d_a}\right) & \text{for three dimensions } (D = 1) \end{cases} \quad (8)$$

216 where  $\phi_t$  and  $d_a$  are the total porosity and the diameter of the aggregate, respectively.

217 Based on the definition of LPF and the physical meaning of the CDF, the following cumulative  
 218 pore size distribution frequency (CPSDF) equations as shown in **Eq. (9)** have been obtained:

$$F = \begin{cases} 1 - \exp\left(-\frac{3(1-\phi_t)z}{2\phi_t d_a}\right) & \text{for three dimensions } (D = 3) \\ 1 - \exp\left(-\frac{4(1-\phi_t)z}{\pi\phi_t d_a}\right) & \text{for three dimensions } (D = 2) \\ 1 - \exp\left(-\frac{(1-\phi_t)z}{\phi_t d_a}\right) & \text{for three dimensions } (D = 1) \end{cases} \quad (9)$$

219 It has been reported that the LPF method outperforms the WBD method based on the lower  
 220 variation of extracted cumulative pore size distribution frequency curve.

## 221 **2.2 HYDRAULIC CONDUCTIVITY**

222 Hydraulic conductivity, sometimes referred as water permeability, is a quantitative measure that  
 223 characterizes the flow of water through a porous media. It is an important design parameter for the  
 224 desired functionality of PC. Since the hydraulic conductivity of PC is several orders of magnitude  
 225 larger than water transport property of CIC, conventional methods of measuring water transport  
 226 properties of CIC are not applicable. Constant head and falling head are the two of the most  
 227 commonly used methods to experimentally determine the hydraulic conductivity of PC. For the  
 228 falling head method, water pressure head is recorded at predetermined intervals after the  
 229 experiment starts, until water stops flowing out of the system and the pressure head is reduced to a  
 230 desired level. The data obtained from each experiment is analysed using the following equation:

$$K = \frac{aL}{At} \ln \frac{H_1}{H_t} \quad (10)$$

231 where  $K$  is the hydraulic conductivity,  $a$  is the cross-sectional area of the pipe holding the sample,  
 232  $A$  and  $L$  are the cross-sectional area and the length of the PC sample, respectively;  $t$  is the time  
 233 water head reaches the predetermined final level. For constant head method, the water head  
 234 difference between the in-flow and out-flow is kept constant and the out-flow overtime is  
 235 continuously tracked. The hydraulic conductivity is given by **Eq. (11)** as follows:

$$K = \frac{QL}{Ah} \quad (11)$$

236 where  $K$  is the hydraulic conductivity,  $Q$  is the flow rate of water,  $L$  and  $A$  are the length and cross-  
 237 sectional area of the sample, respectively;  $h$  is the water head difference of the in-flow and out-  
 238 flow. It worth pointing out that these two equations are derived based on Darcy's law which is only  
 239 valid for laminar flow. Its applicability to PC remains a controversial issue. Previous study  
 240 demonstrated that Darcy's law may not be valid for pervious concrete due to its very high porosity  
 241 [10]. There are also experimental study [24] that substantiates the laminar flow assumption.  
 242 To achieve a successful design of PC with a desired hydraulic conductivity, it is critical to  
 243 understand the influence of PSC and be able to develop appropriate models to predict the  
 244 permeability. Water transport property has been extensively studied and a variety of empirical  
 245 equations correlating hydraulic conductivity to porosity were proposed (**Table 2**).

246 **Table 2** Equations to predict hydraulic conductivity of PC

No.	Model	Equation	Used by	Type
1	Linear	$K^* = m\phi + n$	Luck et al. 2006 [31]	empirical
2	Power	$K^* = m\phi^n$	Ghafoori et al. 1995 [9]	
3	Exponential	$K^* = me^{n\phi}$	Deo et al. 2010 [32]	
4	Kozeny-Carman	$K^* = 18 \frac{\phi^3}{(1-\phi)^2}$	Montes et al. [33]	semi-empirical
5	Katz-Thampson	$k^{**} = \left(\frac{1}{226}\right) \frac{\sigma}{\sigma_0} d^2$	Neithalath et al. 2010 [30]	

247 \* $K$  hydraulic conductivity,  $\phi$  porosity,  $m$  and  $n$  empirical constants

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\*\* $\sigma$  electrical conductivity of porous material,  $\sigma_0$  electrical conductivity of conducting medium

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However, the empirical models (linear, power and exponential model) only account for the

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influence of porosity. Research has demonstrated that pore tortuosity plays an important role in

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determining the hydraulic conductivity [20, 34]. Few studies have been carried out quantifying the

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influence of pore tortuosity on hydraulic conductivity of PC. By combining the Kozeny-Carman

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model and the modified parallel effective conductivity model, a hydraulic connectivity factor was

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introduced by Neithalath et al. [34] to account for the influence of pore tortuosity. But this method

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is difficult to be used for practical applications due to the requirement of complicated measurement.

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Zhong and Wille correlated tortuosity to relative mean pore size based on the fact that the mean

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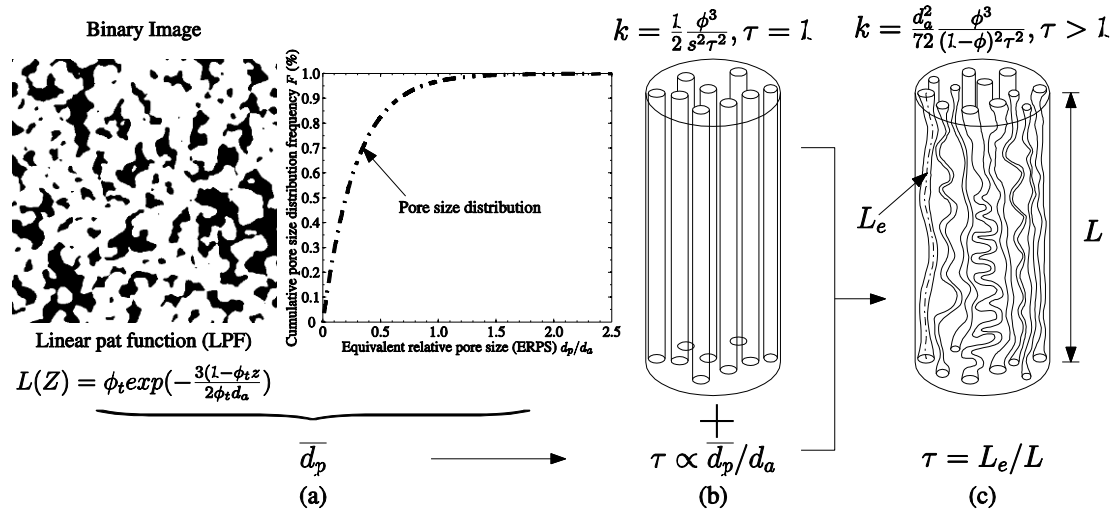
pore size is derived from PSD which is an indicator of the microstructure of the pore system [20]

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(Fig. 4). This method uses readily available design parameters to quantify the tortuosity thus is easy

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to use for practical design purpose.



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Fig. 4 Influence of tortuosity on the hydraulic conductivity of PC

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### 2.3 SOUND ABSORPTION

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Noise pollution is increasingly being recognized as a serious environmental issue. Although noise

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is usually generated from various sources, traffic noise has been shown to be a major source of the

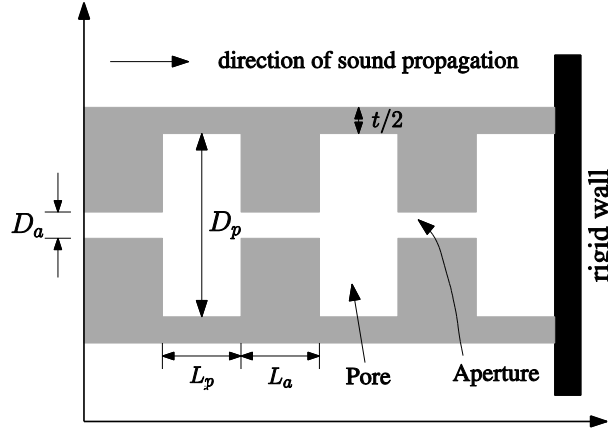
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total environmental noise [35]. Noise generated by traffic is annoying to nearby residents, which

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in turn undermines the quality of their life [36]. It can cause health problems such as sleeping

267 disturbances and learning disabilities [37]. Therefore, there is an increasing demand for quieter  
268 pavement. Among all sources of traffic noise, tire-pavement interaction noise (TPIN) is the  
269 dominant one, especially when vehicles move at medium to high speed [38]. Different noise  
270 reduction approaches have been adopted to combat TPIN. However, commonly used techniques,  
271 such as erection of noise barriers and use of insulating materials, are proven to be expensive [27,  
272 38]. Therefore, researchers throughout the world started using alternative pavement surfaces in an  
273 attempt to mitigate noise at source. Application of a PC layer on top of CIC has been suggested as  
274 one potential solution for this problem [34]. The effectiveness of PC as a TPIN abatement tool is  
275 attributable to lower noise generation and improved sound absorption. The porous surface is  
276 believed to reduce the structure borne air pumping effect (currently thought to be one of the primary  
277 noise generation mechanisms) to minimize noise generation while the pore system of the material  
278 absorbs the sound energy through internal friction. A large number of experimental studies have  
279 been conducted to develop acoustically efficient PC. Gerharz found that 4-8 mm aggregates were  
280 appropriate to produce PC for sound absorption purpose [39]. Marof et al. investigated the influence  
281 of aggregate size and gradation and concluded that PC produced using properly blended aggregates  
282 can lead to improved acoustic absorption [27]. Park and his co-workers assessed the influence of  
283 void ratio and recycled aggregates and reported the optimum void ratio and recycled aggregate  
284 content of 25% and 50%, respectively [40]. Khankhaje et al. used different sizes of oil palm kernel  
285 shell (KS) and cockle shell (CS) to partially replace natural coarse aggregate and suggested that it  
286 is practical to use KS and CS to produce cleaner and quieter PC pavement [41]. Kim and Lee  
287 evaluated the influence of cement workability and aggregate type [42]. It was found that the  
288 workability of cement did not drastically affect the acoustic absorption of PC within the  
289 investigated range and the size of aggregate had slight effects on the acoustic absorption for the  
290 aggregate size of 4-19mm. Neithalath et al. studied the influence of pore connectivity [34]. A pore  
291 connectivity factor was defined and extracted using electrical impedance approach. It was found  
292 that pore connectivity factor and the maximum acoustic absorption coefficient were linearly related.



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**Fig. 5** Idealized alternating cylinder model for sound propagation in PC

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In addition to experimental studies, limited theoretical investigation has also been conducted. A

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model originally proposed by Lu and Chen for sound absorption analysis of cellular metals with

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semi-open cells was adopted to predict the acoustic absorption of PC [21]. PC has been idealized

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as a series of alternating cylinders as shown in Fig. 5. A simplified structure factor proposed by

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Allard et al. as presented in **Eq. (12)** was used to account for the tortuosity of the pore system in

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PC [43].

$$\tau = \frac{4}{9} \frac{(D_a^2 L_a + D_p^2 L_p)}{(L_a + L_p) D_p^2} \quad (12)$$

301

where  $D_a$  and  $D_p$  are the diameters of the aperture and pore, respectively;  $L_a$  and  $L_p$  are the lengths

302

of the aperture and pore, respectively;

303

## 2.4 MECHANICAL PERFORMANCE AND DURABILITY

304

Typical compressive strength of conventional PC with porosities between 15% and 30% ranges

305

from 7 to 25 MPa [22]. Different strategies have been employed to improve the strength of PC.

306

Compressive strength over 20 MPa was reported by reducing the aggregate to binder ratio [44-45].

307

Compressive strength of PC exceeding 40 MPa was achieved through the incorporation of

308

supplementary cementitious materials (SCMs) such as silica fume (SF) and fly ash (FA), polymer

309

modification of the matrix or combination of SF and fine sand [46-47]. It is worth noting that PC

310

with compressive strength more than 50 MPa was reported in literature. However, a 2 MPa mold

311 pressure was applied during testing and the compressive strength was reduced to 27 MPa when the  
312 mold pressure decreased to 1 MPa [48]. Recently, HPPC characterized by a compressive strength  
313 in excess of 40 MPa was designed by Zhong and Wille [7, 22].

314 Prediction of the mechanical behaviour is essential for appropriate design and effective utilization  
315 of this material. Since porosity is one of the most important and easily obtainable PSC, studies have  
316 been focused to predict the compressive strength of PC using this parameter. Several exponential  
317 models have been proposed as shown in **Table 3** (model 1 and 3). Nevertheless, due to the random  
318 nature of the pores in PC, its mechanical performance cannot be reliably predicted using porosity  
319 alone. The mechanical response of PC is also strongly dependent on other PSC [19]. Deo et al. used  
320 IA technique to extract PSC such as area fraction of pores, number averaged pore size, specific area  
321 and mean free spacing of pores [1]. A multiple non-linear model was proposed to calculate the  
322 compressive strength of PC (**Table 3**, model 2). However, IA is time consuming and challenging  
323 to be used in practical application and the input parameters are not readily available or measurable.  
324 Motivated by the limitations, a model (**Table 3**, model 4) using easily accessible parameters was  
325 proposed by Zhong and Wille [19]. A LPF for impenetrable equal size particle two phase random  
326 system was selected and tailored for PC to extract the pore size distribution (PSD) of PC. The mean  
327 pore size is then derived based on probability theory. The obtained mean pore size along with total  
328 porosity and matrix strength were used to construct a model that predicted the compressive strength  
329 of PC very well. Attempts were also made to correlate the stress-strain response of PC and their  
330 PSC. Zhong and Wille employed Popovic's model and Carreira and Chu's model for the ascending  
331 part and descending part of the stress-strain relationship, respectively [22]. Three correction factors  
332 are correlated to relative mean pore size and pore size distribution density. The same models were  
333 adopted by Deo et al [1]. Multiple linear models were constructed to calculate different correction  
334 factors using a variety of PSC, such as porosity, number averaged pore size, three dimensional pore  
335 distribution density and mean free spacing of pores. Rehder and his coworkers investigated the  
336 fracture behaviour of PC [28]. It was found that porosity is the dominant factor that dictates the



337 facture toughness of PC. For a similar porosity, a reduction of facture toughness was observed for  
 338 samples with larger pore size.

339 **Table 3** Summary of compressive strength prediction models for PC

No.	Equation	Author	Year
1	$\sigma = \sigma_0 \exp[-(m - n\xi)\phi]^*$	Chindaprasirt [49]	2009
2	$\sigma = \alpha_0 + \alpha_1 \left[ \frac{\ln(d_{MFS})}{\ln(\bar{d}_n)} \right] + \alpha_2 \left( \frac{\phi_A}{S_p} \right)^{-1} + \alpha_3 \ln(\Gamma_{3D})^{**}$	Deo et al. [1]	2010
3	$\sigma = \sigma_0 \sqrt{(1 - \phi)^m e^{-n\phi}}$	Lian [47]	2011
4	$f'_c = f'_{c0} (1 - m\phi_t) \left( \frac{d_a}{d_p} \right)^n$	Zhong et al. [19]	2016

340 \*m and n are empirical constants,  $\sigma_0$  is the matrix strength,  $\xi$  is the fineness modulus of aggregate  
 341 \*\* $d_{MFS}$  is the mean free spacing of pores which is defined as the average value of uninterrupted surface-to-surface  
 342 distances between all the neighboring pores,  $\phi_A$  area fraction of pores from 2D images,  $S_p$  is specific area of pores,  
 343  $\Gamma_{3D}$  is three dimensional pore distribution density,  $\bar{d}_n$  is the number averaged pore size and can be calculated from  
 344  $\bar{d}_n = \frac{\sum N_i d_i}{\sum N_i}$ , in which  $N_i$  is the number of pores with an average diameter of  $d_i$   
 345 \*\*\* $f'_c$  and  $f'_{c0}$  are the compressive strength of PC and the matrix, respectively;  $\bar{d}_p$  is the mean pore size;  $m$  and  $n$  are  
 346 empirical constants

347 The high void content of PC makes it more vulnerable to different distresses than CIC. Therefore,  
 348 durability is a concern for the application of PC. Laboratory tests and field investigation have been  
 349 conducted for the freeze-thaw (F-T) and abrasion durability of PC. Zhong and Wille investigated  
 350 the matrix type, pore system characteristics and fiber reinforcement on the F-T durability of PC  
 351 [50]. It was demonstrated that use of higher strength matrix, smaller aggregate can lead to better F-  
 352 T durability whereas no improvement can be achieved through fiber incorporation unless ultra-high  
 353 strength matrix is employed. Comparative study on the laboratory and field performance of PC  
 354 were conducted by Shu et al. showing that addition of air entraining agent (AEA) could still help  
 355 to improve the F-T resistance [51]. Wu and his coworkers and Dong et al. assessed the abrasion  
 356 durability of PC [52-53]. Their results indicated that adding latex desirably improved the abrasion  
 357 resistance of PC.

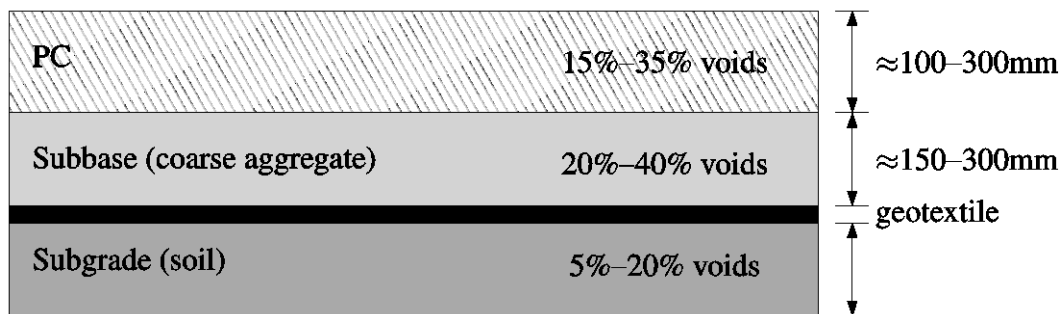
### 358 **3 PC PAVEMENT**

359 Despite the various positive environmental impacts, the application of PC remains limited to  
 360 parking lot, sidewalk and pathways. This is attributable to the drawbacks inherited from its random

361 porous structure, such as limited strength, susceptibility to clogging and concerns for durability.  
 362 Nowadays, with the increasing concern for sustainable development along with recent advances in  
 363 HPPC, there has seen a rise on the application of PC for pavement construction.

### 364 3.1 STATE-OF-THE-ART PAVEMENT APPLICATION

365 For highway applications, PC usually overlays a CIC as a wearing course as illustrated in **Fig. 6**,  
 366 providing both functionality of TPIN reduction and water drainage [54-55]. It is typically  
 367 constructed through a “wet-on-wet” process. Noise abatement of this multilayer composite system  
 368 is achieved due to PC’s acoustical absorption capacity, while strength and durability are improved  
 369 by the reinforcement of the underlying CIC pavement layer [54]. Both criteria of sufficient strength  
 370 and durability under site-specific loading and environmental exposure must be satisfied to achieve  
 371 a successful design of PC for the application of surface-wearing course. The following sections  
 372 reviews the current pervious concrete pavement (PCP) practice around the world.



373

374 **Fig. 6** Schematic cross section of PC wearing course on CIC

#### 375 3.1.1 USA

376 PC is typically used in the United States as a tool for storm water runoff management [56].  
 377 Currently, typical full-depth application of PC in the United States is parking lots, pathways, and,  
 378 in some cases, low-volume roads for storm water control applications [2]. Goede et al. conducted  
 379 distress surveys on two collector streets in use for 20 years. They were constructed using PC and  
 380 were subjected to equivalent traffic stresses. Pavement condition index (PCI) ratings demonstrated  
 381 that properly designed PC can be used for most residential streets and many collector streets for a

382 typical design service life (20-30 years) while maintaining satisfactory structural performance [57].  
383 While PCP is used primarily for low-volume facilities, PCP as a single structural layer is also  
384 possible due to recent advance of HPPC [22]. A project on the mixture design of freeze-thaw  
385 durable PC has recently been finished at Iowa State University (ISU) [3]. The results of this study  
386 have demonstrated that a strong, F-T durable PC mix design is possible. The strength can be  
387 enhanced through the incorporation of a small quantity of fine sand and/or latex admixture to  
388 improve the bond between aggregate particles in the mixture. A comprehensive investigation on  
389 the mixture design of PC for the application of wearing course has been completed at ISU partnered  
390 with the Minnesota Department of Transportation (MnDOT) [58]. PC was constructed in 2008 on  
391 Cell 39 of the Low-Volume Road facility at MnDOT. CIC originally constructed during July 1993  
392 was underneath the PC overlay. The nominal thickness of PC top surface is 4 in. (100 mm). Three  
393 condition surveys were conducted in June of 2009, 2010 and 2011, respectively. Detailed analysis  
394 of the survey data indicates that the overall performance and durability are good except localized  
395 distresses of pavement. Operations during rain events confirmed the good permeability of the PC  
396 overlay. The rainwater is quickly removed from the pavement surface and migrates laterally to the  
397 side of the pavement. These results demonstrated that PC is efficient in improving the safety of  
398 driving through splash and spray migration and hydroplaning reduction. TPIN was characterized  
399 using on-board sound intensity (OBSI) method according to AASHTO TP76. Results indicated that  
400 PC would be a promising method to mitigate TPIN in urban areas. A PC shoulder was installed  
401 along a rest stop on Interstate 4 in central Florida [59]. The PC shoulder was monitored over one  
402 year for wear and storm water management performance. No visual wear was noted on the PC. The  
403 average rate of infiltration over one year was higher than 1.5 inches per hour. It was concluded that  
404 this PC application was successful taking into consideration of wear, water quantity and quality.

405 **3.1.2 JAPAN**

406 Surface course of roadway is a typical application of PC in Japan [60]. Based on the current policy,  
407 all pavements in Japan will be replaced with pervious systems due to its improved safety and  
408 driving comfort [55]. Thin bonded PC overlays is considered as the most preferable option to switch  
409 from current concrete pavement to a pervious system. Simulation experiments in laboratories have  
410 shown that PC is rutting resistant and outperforms porous asphalt in terms of wear resistance to tire  
411 chains. Two experimental PC sections with a thickness of 200 mm (8 in.) were assessed in Japan  
412 [61]. In comparison with dense asphalt pavements, they demonstrated noise reduction of 6 to 8  
413 dBA and 4 to 8 dBA for dry surfaces and wet surfaces, respectively. This investigation was  
414 performed under a condition that cars travelled at a speed ranging from 40 to 75 km/h. For heavy  
415 tracks, noise reduction was 4 to 8 dBA for dry surfaces and 2 to 3 dBA for wet surfaces.

416 **3.1.3 EUROPE**

417 Field performance evaluation of several pavements constructed in Europe using PC has  
418 demonstrated great potential in TPIN abatement and wet weather spray reduction [39, 62]. PC has  
419 also been applied to high-volume roadways as a surface-wearing course in Europe [3, 63]. Based  
420 on the experience from the limited applications, it has shown to be promising in reducing roadway  
421 noise, improving splash and spray, and friction. Undesirable durability in freezing weather was  
422 observed when PC was first constructed in Belgium [64]. However, once polymer additives were  
423 incorporated and a higher cement content was used, significant improvement in the service life was  
424 achieved. In a separate Belgian investigation, 5 dBA decrease in roadway noise was reported when  
425 PC with 19% porosity was used [65]. A recent European Scan (sponsored by the Federal Highway  
426 Administration/FHWA) demonstrated that the European prefers exposed aggregate concrete (EAC)  
427 to PC. The issues of PC such as clogging, ravelling, and winter maintenance were the explanation  
428 for the declining use of pervious pavements in Europe. However, participants in the European Scan

429 pointed out that the concept of PC as a viable paving alternative was not given enough time for  
430 fully exploration [64].

#### 431 **3.1.4 CHINA**

432 Nowadays, China is facing increasingly serious issue of urban flooding. Significant expansion of  
433 impervious area resulted from rapid urbanization of China since 1980s is one of the major  
434 contributing factors for this issue. Construction of conventional storm water management facilities,  
435 such as drainage system, is the common practice to tackle this issue. However, conventional  
436 drainage system is not capable of absorbing, maintaining or purifying water to relieve the water  
437 scarcity or prevent ground water pollution and depletion. Additionally, the drainage system in  
438 China is underdeveloped and the designs are obsolete [66]. Therefore, China has launched a  
439 nationwide initiative called “Sponge City” in 2012. It is an integration of a series of specific  
440 rainwater management technologies which aims to reduce storm water runoff at source [67]. PCP  
441 has been proposed as an alternative to CIC and has been constructed or planned in the pilot cities  
442 selected in 2015 and 2016. In addition to urban pavement applications for the development of  
443 Sponge City, PC has also been used as surface layer for highway pavement and tunnel pavement.  
444 Chen et al. studied the performance of PC applied as a surface layer for highway pavement [68].  
445 They suggested that the top layer PC should be casted after initial setting of the underlying CIC.  
446 Smoothing should be avoided for improved interface bonding. Li and his coworkers assessed the  
447 influence of the timing of applying the PC overlay on the performance of a double-layer tunnel  
448 pavement [69]. The optimal time of paving for the PC top layer is between the initial setting time  
449 and the final setting time of the underlying layer. Zheng et al. investigated the joint space of PC  
450 used as a base for cement concrete pavement [70]. Recommendations were made for joint space  
451 when the thickness of concrete pavement is within the typical range.

### 452 3.2 FUTURE WORK FOR PC PAVEMENT APPLICATION

453 Use of PC as a pavement construction material is emerging but its application remains scare in  
454 comparison with the CIC. Further research is necessary to facilitate a wider application of PC as a  
455 sustainable pavement construction material.

456 • *Mixture design approach and standard test methods.* A large number of parameters affect  
457 the performance of PC. However, trial and error remains the common practice for the  
458 design of PC in the laboratory. Up to now, no standard approach is in place for the mixture  
459 design of PC. Therefore, meaningful comparison cannot be made between results from  
460 different laboratories. Additionally, either customized methods by individual researcher or  
461 standard test methods developed for CIC have been used for PC. Customized test methods  
462 vary from laboratory to laboratory. Test methods standardized for CIC may not be  
463 applicable to PC due to its unique porous structure. Thus, lack of standard test methods  
464 developed specifically for PC further complicates the comparison between different  
465 investigations and impedes the convergence of PC technology developments that support  
466 the realization of PCP.

467 • *Structural design code and procedure.* Due to the increasing concerns for sustainability,  
468 modern pavement design has shifted from a single objective task to a multi-objective  
469 systematic project. With the development of HPPC, this material can be designed to satisfy  
470 both the structural and functional requirements for pavement application. However,  
471 specifications such as minimum strength, threshold hydraulic conductivity, desired noise  
472 abatement or optimum thickness remain missing. There is a need to develop an universal  
473 structural design code and procedure for PC similar to those of other pavement materials  
474 such as CIC and asphalt concrete.

475 • *Field Construction, quality control, maintenance.* Discrepancy between laboratory results  
476 and field performance has been observed. It is inherent for PC due to its random and

477 heterogeneous nature. However, this may also be attributable to the inappropriate field  
478 construction practice. Therefore, such variation can be minimized if field construction  
479 procedure is standardized and quality control measures are put in place. Premature failure  
480 of PC pavement due to the loss of functionality such as clogging are frequently reported.  
481 Thus, regular maintenance policy and robust methods need to be developed to maintain or  
482 restore the functionality of PCP.

483 • *Applicability on high volume road.* Despite of recent advances in HPPC and pioneering  
484 study on the performance of PCP, those conclusions are usually drawn based on short term  
485 performance. Therefore, further research is still needed for the applicability of PC as a  
486 construction material for high volume road, especially its long term performance.

487 • *Microstructural characterization and structural performance modeling.* It is well known  
488 that the performance of PC is strongly dependent on its PSC. However, only limited  
489 investigation has been conducted and very few models are available. Finite element  
490 analysis is not suitable to predict the behavior of PC because of its discrete nature. Hence,  
491 future efforts should be made using advanced techniques such as discrete element method  
492 for structural performance modelling.

493 • *Life cycle cost analysis.* The very limited work on cost analysis might be another reason  
494 that contractors are hesitate to use this material. PC is mechanically weaker and the initial  
495 cost is higher in comparison to conventional CIC based on limited studies [71].  
496 Nonetheless, these conclusions are drawn based only on the initial construction cost. Life  
497 cycle cost analysis (LCCA) indicated that PC can reduce the LCCA by 30% [72].  
498 Furthermore, the added benefits coming from the storm water management and low  
499 environmental impact need to be accounted for as long as cost-benefits is concerned.  
500 Therefore, further research is needed to address the cost concern in order to achieve a wide  
501 spread application of PC.

## 502 **4 CONCLUSIONS**

503 Recently, PC has attracted renewed interests among the media, engineers, contractors and academia  
504 due to its positive environmental impacts. This paper reviewed the state-of-the-art and the state-of-  
505 the-practice research and application of PC as an alternative sustainable pavement construction  
506 material. Emphasis was laid on the microstructural characterization and the influence of PSC on  
507 the mechanical, hydraulic and acoustical performance. Previous studies have demonstrated that PC  
508 performed well in low-volume road applications implying its promising prospects as an alternative  
509 pavement construction material. However, there exist several knowledge gaps that impede the  
510 wider application of PC. Hence, further research is needed to bridge these knowledge gaps so that  
511 a better understanding and improved overall performance can be achieved and eventually standard  
512 test methods, design code and construction procedures can be implemented in the future.

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516 Compliance with Ethical Standards

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