

Material properties and residual stresses of octagonal high strength steel hollow sections

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Abstract

This paper presents an experimental investigation to quantify the variation of material properties and residual stresses in the octagonal high strength steel hollow sections from different fabrication routes involving welding or combinations of welding and press-braking. Tensile coupon tests were conducted on the specimens extracted from different locations of the hollow sections with different fabrication routes and static mechanical properties and stress-strain relationship for the specimens were measured. The influence of welding on the material properties was found to be insignificant while strength enhancement was observed for the material at corners formed by press-braking. A stress-strain curve model was proposed for the material across octagonal high strength steel hollow sections. The magnitudes and distributions of longitudinal residual stresses of the octagonal high strength steel hollow sections with different fabrication routes were also measured using the sectioning method and were also found to be dependent on the fabrication route. Based on the measured residual stress results, residual stress models were developed for the hollow sections from different fabrication routes. The obtained variation of material properties and longitudinal residual stresses can be employed to accurately analyse the performance of octagonal high strength steel hollow section structural members for efficient structural designs.

Keywords

Octagonal hollow section; Material properties measurements; Residual stress; High strength steel; Fabrication route

28 **1 Introduction**

29 High strength steel (HSS) tubular members have been increasingly used in structural applications due
30 to their combined advantages of strong buckling resistance, high strength-to-weight ratio,
31 environmental efficiency, aesthetic appearance, and cost efficiency. Extensive experimental and
32 numerical research studies focusing on HSS tubular structures with square, rectangular and circular
33 sections have been conducted to determine the material properties and residual stresses of the hollow
34 sections [1-6] and to investigate the cross-sectional and member behaviour under quasi-static
35 compression, bending and combined loadings [1, 7-25]. In recent years, octagonal steel tubular
36 members have also been used in civil structural applications such as transmission line structures,
37 towers and lattice structures [26-28]. Octagonal hollow sections demonstrate stronger local buckling
38 resistance than that of square and rectangular hollow sections and also provide the flat surfaces for the
39 easier connection construction compared with circular hollow sections. Hence, HSS octagonal cross-
40 section members have attracted the attention from researchers and structural manufacturers to apply
41 the members in long-span truss structures [29-30]. In order to accurately predict the strength and
42 behaviour of the HSS octagonal tubular members for efficient structural design, the variation of
43 material properties and residual stresses in the HSS octagonal hollow sections which can influence the
44 strength and buckling behaviour of the structures, need to be well understood.

45 The variation of properties and residual stresses existing in the members without being loaded are
46 primarily induced by the structural fabrication processes [31-33]. Aoki et al. [34] investigated the
47 compressive strength of octagonal steel tubular stub columns which were formed by welding eight
48 steel plates, as depicted in Fig. 1(a). Godat et al. [26] used a different fabrication route by welding two
49 half-sections to form octagonal tubular structures. In their study, each half-section had three cold-
50 bended corners, as shown in Fig. 1(b). Mitiga et al. [35] and Migita and Fukumoto [36] also
51 investigated the compressive strength of octagonal tubular structures produced using another
52 fabrication route for which each half-section had four corners obtained by cold-bending, as shown in
53 Fig. 1(c). In these fabrication routes, welding or combined welding and cold-bending processes were
54 applied. Welding process induces heat-input to the materials around the welding seam and causes heat
55 affected zone (HAZ) in which the material properties can be different from those of the materials
56 outside HAZ [37]. Cold-bending process also affects the material properties due to strain hardening
57 effect at the cold-bending region subject to large plastic deformations [32, 38-39]. Besides, the
58 welding and cold-bending also lead to non-uniform thermal and plastic strains in the hollow section
59 structural members and subsequently induce residual stresses. Since the buckling resistance and
60 strength of the structures are dependent on the material properties and residual stresses, ignoring the
61 variation of the properties and residual stresses in octagonal hollow sections can lead to inaccurate
62 estimation of the structural performance. However, to date, no investigations have been performed to

63 determine the variation of material properties and residual stresses in HSS octagonal hollow sections
64 formed using the three different fabrication routes.

65 Therefore, in this study, the variation of material properties and residual stresses in HSS octagonal
66 hollow sections formed using the aforementioned three fabrication routes are investigated
67 experimentally. Tensile coupon tests were conducted on specimens extracted at different locations in
68 the sections. The measured properties were compared in order to examine the effect of fabrication
69 route on the variation of properties in the HSS octagonal hollow sections. Furthermore, residual
70 stresses in the HSS octagonal hollow sections formed using the three fabrication routes were also
71 measured and compared. The effect of fabrication route on the residual stress distribution in the
72 sections is also discussed.

73 **2 Octagonal hollow section specimens**

74 HSS octagonal hollow section specimens were formed using the three fabrication routes introduced in
75 Section 1. Specimens from the fabrication routes presented in Figs. 1(a)-(c) respectively are named
76 W-Series, CF1-Series and CF2-Series. S690 steel plates with nominal yield strength of 690 N/mm²
77 and with thicknesses of 6 and 10mm were used to form the specimens. The steel plates with each
78 thickness were produced in the same batch, allowing the direct comparison of experimental
79 investigations on the properties and residual stress of the specimens. For each specimen in W-Series,
80 eight steel plates were welded together through gas metal arc welding (GMAW) and full penetration
81 weld was used. The selected electrode wire was 1.2mm of the category ER110S-G according to the
82 specification AWS A5.28 [40]. Preheating at about 150°C was applied prior to the start of each
83 welding process. The applied shielding gas was Ar80%+CO₂20%. For the welding, the voltage was
84 about 26-29V while the amperage was about 220-240A. While fabricating the specimens using the
85 CF1 and CF2 routes, the steel plates were longitudinally folded at room temperature through press-
86 braking to form the half octagonal sections. Two half octagonal sections were subsequently welded
87 through GMAW to form each specimen in CF1 or CF2 series. Three cross-sectional dimensions were
88 chosen for specimens with each fabrication route. The measured dimensions for the specimens with
89 different plate thicknesses and plate width-to-thickness ratios between 6.7 and 23.7 are shown in
90 Table 1 using the nomenclature defined in Fig. 1. The specimens are labelled based on the fabrication
91 route and nominal dimensions. For example, the label “CF2-75×10” defines the specimen formed
92 using fabrication route CF2 shown in Fig. 1(c) and with the nominal edge length (B) and thickness (t)
93 of 75 and 10mm respectively.

94 **3 Material properties investigations**

95 **3.1 Tensile coupon tests**

96 Tensile coupon tests were conducted to measure the material properties of HSS octagonal hollow
97 sections and to examine the heterogeneity in the material of the hollow sections due to fabrication
98 processes. Longitudinal tensile coupons were taken from both flat and corner regions of the HSS
99 octagonal hollow section specimens. During each tensile coupon test, the loading was paused near
100 yield and ultimate strength for 90s to obtain the static loads [41]. Static stress-strain curves obtained
101 from the tensile coupon tests were used to determine the static 0.2% proof stress ($\sigma_{0.2}$), static ultimate
102 tensile strength (σ_u), modulus of elasticity (E), static ultimate tensile strain (ϵ_u) and elongation at
103 fracture (ϵ_f) of the material.

104 **3.1.1 Flat coupon tests**

105 Flat tensile coupons were extracted from the centre of the faces of HSS octagonal hollow sections, as
106 shown in Fig. 1. The dimensions of the flat coupons conformed to the EN10002-1 [42]. The coupons
107 had 6 mm width along the gauge length. The test set-up of flat coupon is shown in Fig. 2(a). A
108 calibrated mechanical extensometer was mounted onto each coupon specimen to measure the
109 longitudinal strain during the test. Two linear strain gauges were also attached at the midpoint on each
110 of the faces of any coupon specimen. The average strain measured by the two strain gauges was used
111 to determine the modulus of elasticity for each coupon specimen. The strains and elongation at
112 fracture (ϵ_f) measured using the extensometer enable the evaluation of material properties in general
113 [1, 41]. By consistently applying this measurement methodology in tensile coupon tests, the variation
114 of material properties in HSS octagonal hollow sections from different fabrication routes can be
115 examined. The precise measurements of strain field at the necking region and fracture point appeared
116 during a tensile coupon test requires the usage of special technique [43] such as digital image
117 correlation (DIC). The strains and elongation of the coupon specimens were measured by strain
118 gauges and extensometer, and DIC technique was not used in this research study. The static stress-
119 strain curves for flat coupons obtained from different HSS octagonal hollow sections are presented in
120 Fig. 3. The material properties determined from the static stress-strain curves in Fig. 3 are given in
121 Table 2. The value of $\sigma_{0.2}$ for the coupon specimens ranges from 753 to 780 MPa while the σ_u varies
122 from 795 to 821 MPa. For the HSS octagonal hollow sections with the same plate thickness,
123 consistent stress-strain behaviour was obtained for the materials at the centre of the flat portions from
124 these sections formed using different fabrication routes, as can be observed in Fig. 3. This observation
125 indicates that the influence of welding and press-braking on materials at the centre of the flat faces
126 was relatively insignificant due to the distances between the location of flat coupons and the welded
127 or corner regions.

128 3.1.2 Corner coupon tests

129 Corner coupons were extracted from the corner regions of CF1 and CF2 series sections and the
130 locations of the coupons in the sections are shown in Figs. 1(b) and (c). The width of the corner
131 coupons is 4mm. For each corner coupon, two holes with the diameter of 8.5mm were drilled at a
132 distance of 15mm from both ends of the coupon. The two holes were used for the installation of two
133 specially fabricated pins for gripping the coupon specimen so that tensile loading can be applied to the
134 corner coupon through its centroid [41], as shown in Fig. 2(b). The test procedures for the corner
135 coupon tests are identical to those for the flat coupon tests. Static stress-strain curves were obtained,
136 as presented in Fig. 4. The material properties determined based on these static stress-strain curves are
137 summarised in Table 3. By comparing the stress-strain curves of corner coupons with those of flat
138 coupons, the strength enhancement accompanied by the decrement of ductility can be clearly
139 observed for the corner material due to the cold-working effect of the press-braking fabrication
140 process. The $\sigma_{0.2}$ of each corner coupon was slightly larger than that of the flat coupon from the
141 centreline of the face of the same HSS octagonal hollow section while the σ_u of the corner coupons
142 was increased by 3 to 8% compared to that of the flat coupons. Contrary to the tensile strength
143 enhancement, the ϵ_u of the corner coupons decreased significantly and was about 69 to 77% lower
144 than that of the flat coupon from the same section. The ϵ_f for the corner coupons also decreased by 19
145 to 23%.

146 Both strength enhancement and reduction of ductility in corners with different ratios of inner corner
147 radius over plate thickness (r_i/t) were also compared. For CF1-75×10 and CF2-75×10 sections with
148 the r_i/t ratio of about 1.50, the static $\sigma_{0.2}$ and σ_u of the corner materials respectively increased by 2.7
149 and 7.6% on average compared with those of the material at the centre of the flat regions while the ϵ_u
150 and ϵ_f decreased by 73.5% and 20.5% on average respectively. For CF1-75×6, CF1-160×6, CF2-75×6
151 and CF2-160×6 sections with r_i/t ratios ranging from 2.22 to 2.50, the static $\sigma_{0.2}$ and σ_u of the corner
152 materials increased by 2.7 and 5.1% on average respectively while the ϵ_u and ϵ_f decreased by 73.2%
153 and 20.5% on average respectively. Comparing the effect of press-braking fabrication on the material
154 properties of corners with different r_i/t ratios, both the strength enhancement and reduction of ductility
155 occurred at corners were insensitive to the r_i/t ratio. The minimal influence of r_i/t ratio was due to the
156 relatively small differences between the r_i/t ratios of corners from different sections. Besides, the level
157 of strain hardening for the high strength steel material is relatively low, as can be seen from the stress-
158 strain curves in Fig. 3 for flat coupons. Thus, no obvious differences in both strength enhancements
159 and reduction of ductility for the corner materials in different sections were obtained although the
160 corners in CF1-75×10 and CF2-75×10 sections with lower r_i/t ratios may experience slightly larger
161 plastic strains [44].

162 3.1.3 One-eighth and quarter section coupon tests

163 Differences between the mechanical properties of the material at the centre of the flat surface and
164 those at the corners of CF1 and CF2 series HSS octagonal hollow sections were found, as presented in
165 Sections 3.1.1 and 3.1.2. Thus, the material properties at different locations in the octagonal hollow
166 sections are heterogeneous. As for the W series octagonal hollow sections, effect of welding on the
167 material properties near the welding seam was also unknown. In order to examine the variation of
168 material properties, especially the $\sigma_{0.2}$ and σ_u , around the HSS octagonal hollow sections, tensile
169 coupons were taken for testing from different locations on one-eighth of W-75×6 section and on a
170 quarter of CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6 sections due to symmetrical geometry
171 of the cross-sections. The location and labels of the tensile coupon specimens from these sections are
172 presented in Figs. 5-9.

173 The measured static $\sigma_{0.2}$ and σ_u for the coupon specimens were plotted against the locations where the
174 coupon specimens were extracted in each octagonal hollow section, as shown in Figs. 5-9. The tensile
175 properties of the coupons from different HSS octagonal hollow sections are summarised in Table 4.
176 As can be seen in the figures and Table 4, the tensile strength and ductility of the material at the weld
177 are quite different from those of the material at the flat and corner regions of the hollow sections. The
178 $\sigma_{0.2}$ is lower than that of the material at other different locations in HSS octagonal hollow sections
179 while the elongation is much larger than that of the material at other locations in the hollow sections.
180 This is because the width of the welding seam ranges from 8.5 to 11.3mm and is larger than the width
181 of the middle part of each coupon specimen. Therefore, the part of each coupon under tensile loading
182 during the tests was located well within the welding seam which had the infill of electrode material.
183 Thus, the measured material properties at the weld depends on the properties of the electrode material
184 and are different from the material properties of the steel plates at other locations in the hollow
185 sections.

186 For the W-75×6 section, no obvious variation of material properties in the flat portions of the section
187 was obtained, as shown in Fig. 5. The properties of the coupon specimens closest to the welding
188 seams were compared with those of the coupon specimens at the centre of the flat faces for W-75×6
189 section as well as CF1 and CF2 series sections. As shown in Figs. 5-9 and Table 4, the influence of
190 welding on the material properties near the welding seams is quite low. For the W-75×6, CF1-75×6
191 and CF2-75×6 sections, the static $\sigma_{0.2}$ and σ_u measured for the coupons near the welding seams
192 respectively were found to be only 0.5-3% and 0.25-1.4% lower than those of the coupons at the
193 centre of the flat faces in the sections. As for the CF2-75×10 and CF2-160×6 sections, no influence of
194 welding on the $\sigma_{0.2}$ and σ_u of the material near the welding seam was observed. Since the differences
195 between the properties of materials near the welding seams and those of the materials at the centre of
196 flat faces are minimal, no influence of cooling rate after welding on the material properties of

197 specimens closest to the welding seams is observed [45-46]. The influence of press-braking
 198 fabrication on the material properties in the CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6
 199 sections can also be revealed in Figs. 6-9 and Table 4. Highest strength enhancements occurred at the
 200 corners for these sections because the stresses and plastic deformations induced by press-braking
 201 process concentrated at the corner regions [38]. For CF1-75×6, CF2-75×6, CF2-75×10 and CF2-
 202 160×6 section respectively, the static $\sigma_{0.2}$ of corners increased by 2.3, 5.7, 1.9 and 2.8% on average
 203 compared with that of the material at the centre of the flat region while the static σ_u increased by 3.7,
 204 4.9, 7.2 and 6.9% on average.

205 3.2 Proposed stress-strain curve model

206 The material stress-strain relationship is needed for the analysis and design of HSS octagonal hollow
 207 section structures. A stress-strain curve model is given in European standard [47] and describes the
 208 stress-strain relationship in the form of multi-linear curves. However, the variation of stress with
 209 increasing strain for the HSS material in the octagonal hollow sections, especially the material at the
 210 corners formed by press-braking, is non-linear when the stress is above the proportional limit and
 211 behaviour of the material becomes inelastic under loading, as shown in Figs. 3 and 4. Thus, using the
 212 stress-strain model in European standard can lead to inaccurate stress-strain relationship for structural
 213 design and analysis. In order to describe the non-linear stress-strain behaviour, Hill [48] proposed a
 214 stress-strain curve model based on the Ramberg and Osgood equation [49], as given as Eq. (1). The
 215 parameter n , as the strain hardening exponent is constant.

$$216 \quad \varepsilon = \frac{\sigma}{E} + 0.002 \left(\frac{\sigma}{\sigma_{0.2}} \right)^n \quad (1)$$

217 This model was found to provide suitable stress-strain curves up to $\sigma_{0.2}$ and overestimate the stress at
 218 the strains greater than 0.2% since the parameter n related to strain hardening may vary with
 219 increasing strains [2]. Therefore, a concept proposed by Mirambell and Real [50] of using two-stage
 220 Ramberg and Osgood equations to replicate the non-linear stress-strain behaviour was employed in
 221 this study. The two-stage Ramberg and Osgood material model [51] given as Eq. (2) was used to
 222 replicate the stress-strain relationship for the material at the flat and corner regions in HSS octagonal
 223 hollow sections.

$$224 \quad \varepsilon = \begin{cases} \frac{\sigma}{E} + 0.002 \left(\frac{\sigma}{\sigma_{0.2}} \right)^{n_1} & \text{for } \sigma \leq \sigma_{0.2} \\ \frac{\sigma - \sigma_{0.2}}{E_{0.2}} + \left(\varepsilon_{tu} - \frac{\sigma_u - \sigma_{0.2}}{E_{0.2}} - \varepsilon_{t0.2} \right) \left(\frac{\sigma - \sigma_{0.2}}{\sigma_u - \sigma_{0.2}} \right)^{n_2} + \varepsilon_{t0.2} & \text{for } \sigma > \sigma_{0.2} \end{cases} \quad (2)$$

225 In the equation, $E_{0.2}$ is the stiffness at the $\sigma_{0.2}$, ε_{tu} is the strain at the ultimate strength, $\varepsilon_{t0.2}$ is the total
 226 strain at the $\sigma_{0.2}$, n_1 and n_2 are strain hardening exponents. Eqs. (3) and (4) are proposed for

227 calculating the values of n_1 and n_2 respectively. In Eq. (3), $\sigma_{0.01}$ is the 0.01% proof stress. $E_{0.2}$ for
 228 estimating the stress-strain relationship can be calculated from Eq. (5).

$$229 \quad n_1 = \begin{cases} \frac{\ln(\sigma^{0.2}/\sigma_{0.01})}{\ln(\sigma^{0.2}/\sigma_{0.01})} + \log\left(\frac{\sigma_u}{\sigma_{0.2}}\right) \left(\frac{\varepsilon_{tu}}{0.002}\right) & \text{for flat portion material} \\ \frac{\ln(\sigma^{0.2}/\sigma_{0.01})}{\ln(\sigma^{0.2}/\sigma_{0.01})} & \text{for corner material} \end{cases} \quad (3)$$

$$230 \quad n_2 = \frac{\ln(\varepsilon_{tu}/\varepsilon_{t0.2})}{\ln(\sigma_u/\sigma_{0.2})} \quad (4)$$

$$231 \quad E_{0.2} = \frac{E}{1 + \frac{0.002n_1 E}{\sigma_{0.2}}} \quad (5)$$

232 The stress-strain curves for the material at flat and corner regions across octagonal hollow sections
 233 were estimated using the proposed model and compared with the curves from tensile coupon tests, as
 234 shown in Fig. 10. As can be seen in the figure, the estimated stress-strain curves compared well with
 235 the curves obtained from experiments. Therefore, the proposed stress-strain curve model can be
 236 applied to develop stress-strain curves for the design and analysis of the octagonal hollow section
 237 structural members formed using the fabrication routes in Fig. 1.

238 **4 Residual stress investigations**

239 Residual stresses existing in the HSS octagonal hollow sections in the unloaded state may cause
 240 premature yielding in part of the material around the hollow sections and subsequently influence the
 241 strength and stability of the HSS octagonal hollow section structures [2-3]. For the HSS octagonal
 242 hollow sections, the residual stresses can be caused by the fabrication processes including welding,
 243 press-braking and flame-cutting. Since the residual stresses in the longitudinal direction along the
 244 length of the structures have the most influence on the structural behaviour [2, 52], the magnitude and
 245 distribution of the longitudinal residual stresses were measured for the sections with different
 246 geometry properties and formed using the three fabrication routes shown in Fig. 1.

247 **4.1 Residual stress measurement technique**

248 Sectioning method was adopted in this investigation and applied to measure the residual stresses for
 249 W-75×6, CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6 sections. The specimens prepared for
 250 measuring residual stresses are 300mm in length. During the preparation, one-eighth of W-75×6 and
 251 a quarter of CF1-75×6, CF2-75×6 and CF2-75×10 sections were marked into longitudinal strips of
 252 10mm widths while a quarter of CF2-160×6 section was marked into longitudinal strips of 13mm
 253 widths. This arrangement was consistent with that for tensile coupon tests described in Section 3.1.3

254 because the distribution of residual stresses can be regarded to be symmetrical in the hollow sections
255 with the symmetrical locations of corner regions and welding seams. The strain gauges of 3mm gauge
256 length were then attached on both outer and inner surfaces of the longitudinal strips at the mid-length
257 of each strip. Each strain gauge was also protected with the cover of water proof glue to avoid any
258 contamination and damage in the strain gauge due to sectioning. Initial readings of strain gauges were
259 recorded before the start of sectioning process. Subsequently, sectioning was performed using the
260 wire cutting method and coolant was used to minimise the heat generation during sectioning, as
261 presented in Fig. 11. After the completion of sectioning, the readings of strains were also taken for all
262 longitudinal strips. The initial strain readings before the sectioning process and the final strain
263 readings after sectioning were used to analyse the residual stresses in the HSS octagonal hollow
264 sections, as explained in Section 4.2.

265 **4.2 Measurement results and discussion**

266 The residual strains on the outer and inner surfaces (ϵ_o and ϵ_i respectively) of each longitudinal strip
267 were estimated as the differences between the measured strains before and after the sectioning process.
268 For each strip, the ϵ_o was found to be different from ϵ_i , indicating that both membrane and bending
269 residual stresses existed in the investigated HSS octagonal hollow sections. Therefore, these residual
270 stresses were determined based on the ϵ_o and ϵ_i for W-75×6, CF1-75×6, CF2-75×6, CF2-75×10 and
271 CF2-160×6 sections.

272 The residual stresses in the W-75×6 section were first calculated. The membrane residual stresses
273 were calculated as the average of ϵ_o and ϵ_i multiplied by elastic modulus of the material. The bending
274 residual stresses were calculated as the difference in ϵ_o and ϵ_i multiplied by elastic modulus since the
275 bending residual stresses were suggested to vary linearly through the thickness [53] of welded
276 sections. The magnitude and distribution of the calculated membrane residual stresses and bending
277 residual stresses on the outer surfaces are presented in Fig. 12 in which the maximum residual stresses
278 are marked. Positive and negative values indicate the tensile and compression residual stresses
279 respectively. As can be seen in the figure, the membrane residual stresses are much larger than the
280 bending residual stresses in the W-75×6 section. High tensile membrane residual stresses were
281 obtained at the welding seam and its surrounding two strips while compressive membrane residual
282 stresses were obtained at the other parts of the section. The compression bending residual stresses
283 appeared along the outer surfaces of the one-eighth section. The membrane residual stresses ranged
284 from 13 to 58 % of the $\sigma_{0.2}$. The maximum value of the tensile and compression membrane residual
285 stress respectively were about 58 and 39% of the $\sigma_{0.2}$ of the material while the maximum bending
286 residual stresses on the outer surface was about 7.5% of the $\sigma_{0.2}$.

287 The residual stresses in the CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6 sections were also
288 estimated. Different from the W-75×6 section, the variation of bending residual stresses through
289 thickness for the sections formed using press-braking was found to be non-linear [52-55]. Rectangular
290 stress block through thickness distribution was recommended [52-53] to be suitable for simulating the
291 distribution of bending residual stresses through thickness for sections formed using press-braking
292 and was adopted in this study for estimating the bending residual stresses for CF1-75×6, CF2-75×6,
293 CF2-75×10 and CF2-160×6 sections based on the measured ε_o and ε_i . The obtained membrane
294 residual stresses and bending residual stresses on the outer surface of the sections are presented in
295 Figs. 13-16. For these sections made with folded plates using press-braking, large tensile bending
296 residual stresses were obtained on the outer surface at the corners which experienced large plastic
297 deformations during fabrication and their magnitudes were much larger than those of the bending
298 residual stresses at the other locations of the outer surface. The maximum values of the tensile
299 bending residual stresses in CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6 sections respectively
300 were 20, 20, 27 and 18% of the $\sigma_{0.2}$ of the material at corners. As for the membrane residual stresses,
301 both tensile and compression stresses existed in the sections. The maximum magnitude of membrane
302 residual stresses in CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6 sections respectively were 20,
303 24, 26 and 45% of the $\sigma_{0.2}$ of the material at centre of flat proportions.

304 The magnitude and distribution of the membrane and bending residual stresses in the CF1-75×6 and
305 CF2-75×6 sections are also compared with the residual stresses of W-75×6 section to reveal the
306 influence of fabrication process on the residual stresses. The pattern of residual stresses shown in Fig.
307 12 for the W-75×6 section is quite different from that of the CF1-75×6 and CF2-75×6 sections shown
308 in Figs. 13 and 14 respectively. In the CF1-75×6 and CF2-75×6 sections formed using press-braking,
309 the maximum bending residual stresses were much larger than that of the W-75×6 section while the
310 maximum membrane residual stresses were lower than that of the W-75×6 section. The different
311 distributions and magnitudes of residual stresses in octagonal hollow sections formed using different
312 fabrication routes indicate that the residual stresses measured for the hollow sections with the specific
313 fabrication route should be used in the design and analysis of the structures.

314 **4.3 Proposed residual stress models**

315 The obtained residual stress measurement results were also used to develop membrane and bending
316 residual stress models for HSS octagonal hollow sections. For the W-series section, the longitudinal
317 membrane and bending residual stress distribution models were proposed, as presented in Fig. 17. The
318 magnitudes of the residual stresses given in the models were obtained as the average membrane and
319 bending residual stresses over different locations of the hollow section and given as the normalised
320 values by the measured $\sigma_{0.2}$ of the material. The positive and negative signs in the figure represent

321 tensile and compressive stresses. The bending residual stress for the outer surface of the hollow
322 section is in compression, as shown in Fig. 17.

323 The distribution models for longitudinal membrane and bending residual stresses in CF1-series
324 sections were also proposed based on the experimental results, as shown in Fig. 18. Multi-linear
325 curves and constant values at corners were used to simulate the distribution pattern of membrane
326 residual stresses in the CF1-series octagonal hollow sections. The magnitude of the membrane
327 residual stress at any location in the model was given based on the measured stress value shown in Fig.
328 13. Constant tensile or compressive bending residual stresses on the outer surface of the flat and
329 corner portions were provided since the variation of the bending residual stress magnitudes in the flat
330 or corner regions was low, as shown in Fig. 13. For the CF2-series sections, the proposed longitudinal
331 membrane and bending residual stress distribution models are presented in Fig. 19. Magnitudes of the
332 membrane and bending residual stresses in the model were obtained as the average of the stresses
333 measured at the specific location in the sections.

334 **5 Conclusions**

335 The material properties and residual stress distribution in HSS octagonal hollow section from three
336 fabrication routes have been investigated experimentally. For each fabrication route involving
337 welding or a combination of welding and press-braking processes, hollow sections with different
338 dimensions and plate width-to-thickness ratios were used for the investigations. The effects of
339 welding and press-braking on the material properties of the HSS octagonal hollow sections were
340 investigated by conducting coupon tests on specimens extracted from different locations in the cross-
341 sections. The mechanical properties of the materials in the hollow sections formed through welding
342 eight steel plates were found to be insensitive to the welding process. The static $\sigma_{0.2}$ and σ_u measured
343 for the coupons near the welding seams were less than 3% lower than the properties measured for the
344 coupons at the centre of the flat faces in the sections. For the hollow sections formed using press-
345 braking and welding, strength enhancements at the corners were observed. Comparing with the
346 material properties measured for the flat coupons, the static 0.2% proof stress and ultimate tensile
347 strength of the corner coupons increased up to 5.7 and 7.2% respectively, due to the cold-working of
348 the press-braking process. A stress-strain curve model was developed and found to be capable of
349 accurately estimating the material stress-strain relationship. The model can be used to obtain the
350 stress-strain curves for structural design and analysis.

351 The distribution and magnitude of longitudinal residual stresses of HSS octagonal hollow sections
352 from different fabrication routes were determined using the sectioning method. It was found that both
353 membrane and bending residual stresses existed in the octagonal hollow sections and their magnitudes
354 were determined and plotted. The results showed that the residual stress distribution and magnitudes

355 of the section formed through welding are quite different from those of the sections formed using both
356 press-braking and welding. For the section formed by welding eight steel plates, the membrane
357 residual stresses ranged from 13% to 58% of the 0.2% proof stress of the material while the bending
358 residual stresses were below 7.5% of the material 0.2% proof stress. As for the sections formed by
359 combined press-braking and welding processes, the bending residual stresses in the flat regions were
360 relatively low while the bending residual stresses at the corners which were subject to large plastic
361 deformation during fabrication were quite large and found to be up to 27% of the 0.2% proof stress of
362 the material at corners. The membrane residual stresses in these sections varied between 0.2 and 45%
363 of the material 0.2% proof stress. Residual stress models for the HSS octagonal hollow sections
364 formed using different fabrication routes were also proposed based on the experimental results.

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371 **References**

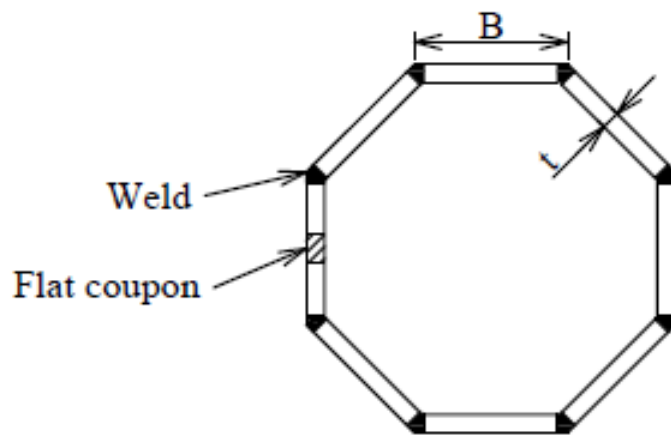
- 372 [1] Wang, J., Afshan, S., Schillo, N., Theofanous, M., Feldmann, M. and Gardner, L. 2017. Material
373 properties and compressive local buckling response of high strength steel square and rectangular
374 hollow sections. *Engineering Structures*, 130, 297-315.
- 375 [2] Ma, J.L., Chan, T.M. and Young, B. 2015. Material properties and residual stresses of cold-formed
376 high strength steel hollow sections. *Journal of Constructional Steel Research*, 109, 152-165.
- 377 [3] Somodi, B. and Kövesdi, B. 2017. Residual stress measurements on cold-formed HSS hollow
378 section columns. *Journal of Constructional Steel Research*, 128, 706-720.
- 379 [4] Yang, C., Yang, J.F., Su, M.Z. and Li, Y. 2017. Residual stresses in high-strength-steel welded
380 circular tube. *Proceedings of the Institution of Civil Engineers*, 170, 631-640.
- 381 [5] Fatemeh, J., Heidarpour, A., Zhao, X.L., Hutchinson, C.R. and Minkkinen, J. 2016. Effect of weld
382 on mechanical properties of high strength and ultra-high strength steel tubes in fabricated hybrid
383 sections. *Engineering Structures*, 118, 16-27.
- 384 [6] Heidarpour, A., Tofts, N.S., Korayem, A.H., Zhao, X.L. and Hutchinson, C.R. 2014. Mechanical
385 properties of very high strength steel at elevated temperatures. *Fire Safety Journal*, 64, 27-35.

- 386 [7] Shi, G., Zhou, W.J. and Lin, C.C. 2015. Experimental investigation on local buckling behaviour of
387 960 MPa high strength steel welded section stub columns. *Advances in Structural Engineering*, 18,
388 423-437.
- 389 [8] Ma, J.L., Chan, T.M. and Young, B. 2016. Experimental investigation on stub-column behaviour
390 of cold-formed high strength steel tubular sections. *Journal of Structural Engineering*, 142(5),
391 04015174-1 to 04015174-11. ASCE.10.1061/(ASCE)ST.1943-541X.0001456.
- 392 [9] Yang, D.M. and Hancock, G.J. 2006. Numerical simulation of high-strength steel box-shaped
393 columns failing in local and overall buckling modes. *Engineering Structures*, 132, 541-549.
- 394 [10] Rasmussen, K.J.R. and Hancock, G.J. 1995. Test of high strength steel columns. *Journal of*
395 *Constructional Steel Research*, 34, 27-52.
- 396 [11] Wang, Y.B., Li, G.Q., Chen, S.W. and Sun, F.F. 2014. Experimental and numerical study on the
397 behavior of axially compressed high strength steel box-columns. *Engineering Structures*, 58, 79-91.
- 398 [12] Ban, H.Y., Shi, G., Shi, Y.J. and Bradford, M.A. 2013. Experimental investigation of the overall
399 buckling behaviour of 960 MPa high strength steel columns. *Journal of Constructional Steel Research*,
400 88, 256-266.
- 401 [13] Li, T.J., Li, G.Q., Chan, S.L. and Wang, Y.B. 2016. Behavior of Q690 high-strength steel
402 columns: Part1: Experimental investigation. *Journal of Constructional Steel Research*, 123, 18-30.
- 403 [14] Li, T.J., Liu, S.W., Li, G.Q., Chan, S.L. and Wang, Y.B. 2016. Behavior of Q690 high-strength
404 steel columns: Part2: Parametric study and design recommendation. *Journal of Constructional Steel*
405 *Research*, 122, 379-394.
- 406 [15] Somodi, B. and Kövesdi, B. 2017. Flexural buckling resistance of cold-formed HSS hollow
407 section members. *Journal of Constructional Steel Research*, 128, 179-192.
- 408 [16] Ma, J.L., Chan, T.M. and Young, B. 2016. Experimental investigation of cold-formed high
409 strength steel tubular beams. *Engineering Structures*, 126, 200-209.
- 410 [17] Tran, A.T. 2016. Lateral-torsional buckling resistance of cold-formed high strength steel
411 rectangular hollow beams. *In Proceedings of Sixth International Conference on Structural*
412 *Engineering, Mechanics and Computation*, Cape Town, South Africa.
- 413 [18] Wang, J., Afshan, S., Gkantou, M., Theofanous, M., Baniotopoulos, C. and Gardner, L. 2016.
414 Flexural behaviour of hot-finished high strength steel square and rectangular hollow sections. *Journal*
415 *of Constructional Steel Research*, 121, 97-109.

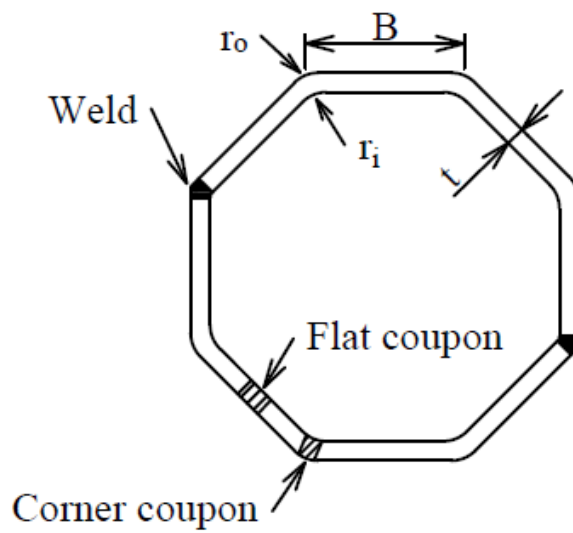
- 416 [19] Pournara, A.E., Karamanos, S.A., Mecozzi, E. and Lucci, Antonio. 2017. Structural resistance of
417 high-strength steel CHS members. *Journal of Constructional Steel Research*, 128, 152-165.
- 418 [20] Gkantou, M., Theofanous, M. and Baniotopoulos, C. 2017. Structural response of high strength
419 steel hollow sections under combined biaxial bending and compression. In proceedings of *8th*
420 *European Conference on Steel and Composite Structures*, Copenhagen, Denmark. p.3547-3556.
- 421 [21] Ma, J.L., Chan, T.M. and Young, B. 2017. Experimental investigation on cold-formed high
422 strength steel circular hollow sections under combined compression and bending. In proceedings of
423 *8th European Conference on Steel and Composite Structures*, Copenhagen, Denmark. p.3622-3630.
- 424 [22] Ma, J.L., Chan, T.M. and Young, B. 2017. Tests on high-strength steel hollow sections: a review.
425 *Proceedings of the Institution of Civil Engineers – Structures and Buildings*, 170 (SB9), 621-630.
- 426 [23] Ma, J.L., Chan, T.M. and Young, B. 2018. Design of cold-formed high strength steel tubular stub
427 columns. *Journal of Structural Engineering*, 144(6), 04018063.
- 428 [24] Fang, H. and Chan, T.M. 2018. Axial compressive strength of welded S460 steel columns at
429 elevated temperatures. *Thin-walled Structures*, 129, 213-224.
- 430 [25] Nassirnia, M., Heidarpour, A., Zhao, X.L. and Minkkinen, J. 2015. A benchmark analytical
431 approach for evaluating ultimate compressive strength of hollow corrugated stub columns. *Thin-*
432 *walled Structures*, 117, 127-139.
- 433 [26] Godat, A., Legeron, F. and Bazonga, D. 2012. Stability investigation of local buckling behaviour
434 of tubular polygon columns under concentric compression. *Thin-walled Structures*, 53, 131-140.
- 435 [27] Gonçalves, R. and Camotim, D. 2013. Elastic buckling of uniformly compressed thin-walled
436 regular polygonal tubes. *Thin-walled Structures*, 71, 35-45.
- 437 [28] Zhu, J.Y. and Chan, T.M. 2018. Experimental investigation on octagonal concrete filled steel
438 stub columns under uniaxial compression. *Journal of Constructional Steel Research*, 147, 457-467.
- 439 [29] Alechnavičius, V. and Bálint, J. 2014. Long span high strength steel trusses. Master thesis.
440 Department of Civil, Environmental and Natural Resources Engineering, Luleå University of
441 Technology.
- 442 [30] Manoleas, P., Koltsakis, E. and Veljkovic, M. 2017. Multiplanar K-joints on cold-formed open
443 sections. In proceedings of *8th European Conference on Steel and Composite Structures*, Copenhagen,
444 Denmark. p.629-638.

- 445 [31] Yu, W.W. and LaBoube, R.A. 2010. *Cold-formed steel design*. 4th Edition, Hoboken, USA: John
446 Wiley & Sons, Inc.
- 447 [32] Sun, M. and Packer, J.A. 2014. Direct-formed and continuous-formed rectangular hollow
448 sections - Comparison of static properties. *Journal of Constructional Steel Research*, 92, 67-78.
- 449 [33] Li, T.J., Li, G.Q. and Wang, Y.B. 2015. Residual stress tests of welded Q690 high-strength steel
450 box- and H-sections. *Journal of Constructional Steel Research*, 20, 283-289.
- 451 [34] Aoki, T., Migita, Y. and Fukumoto, Y. 1991. Local buckling strength of closed polygon folded
452 section columns. *Journal of Constructional Steel Research*, 20, 259-270.
- 453 [35] Migita, Y., Aoki, T., and Fukumoto, Y. 1992. Local and interaction buckling of polygonal
454 section steel columns. *Journal of Structural Engineering*, 118, 2659-2676.
- 455 [36] Migita, Y. and Fukumoto, Y. 1997. Local buckling behaviour of polygonal sections. *Journal of*
456 *Constructional Steel Research*, 41, 221-233.
- 457 [37] Sefcikova, K., Brtnik, T., Dolejs, J., Keltamaki, K. and Topilla, R. 2015. Mechanical properties
458 of heat affected zone of high strength steels. *Material Science and Engineering*, 96, 012053.
- 459 [38] Cruise, R.B. and Gardner, L. 2008. Strength enhancements induced during cold forming of
460 stainless steel sections. *Journal of Constructional Steel Research*, 64, 1310-1316.
- 461 [39] Quach, W.M. and Young, B. 2015. Material properties of cold-formed and hot-finished elliptical
462 hollow sections. *Advances in Structural Engineering*, 18, 1101-1114.
- 463 [40] AWS A5.28/A5.28 M, 2005. Specification for Low-Alloy Steel Electrodes and Rods for Gas
464 Shielded Arc Welding.
- 465 [41] Huang, Y. and Young, B. 2014. The art of coupon tests. *Journal of Constructional Steel*
466 *Research*, 96, 159-175.
- 467 [42] EN 10002-1, 2001. *Metallic materials-Tensile testing-Part 1: Method of test at ambient*
468 *temperature*. Brussels: European Committee for Standardization.
- 469 [43] Golling, S., Östlund, R. and Oldenburg, M. 2016. Characterization of ductile fracture properties
470 of quench-hardenable boron steel: Influence of microstructure and processing conditions. *Material*
471 *Science and Technology: A*, 658, 472-483.

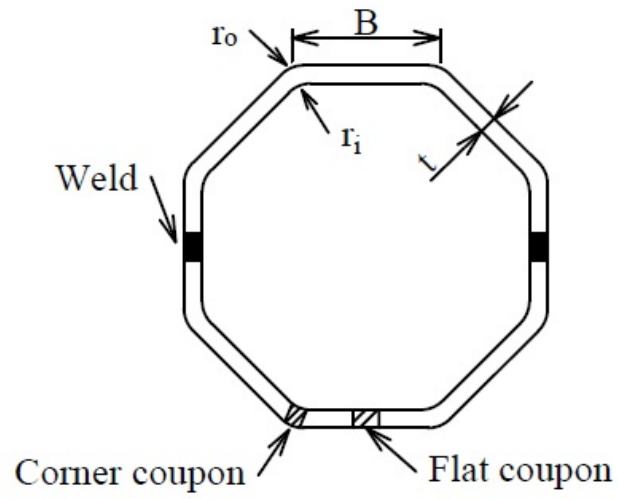
- 472 [44] Rossi, B., Afshan, S. and Gardner, L. 2013. Strength enhancements in cold-formed structural
473 sections - Part II: Predictive models. *Journal of Constructional Steel Research*, 83, 189-196.
- 474 [45] Keehan, E., Zachrisson, J. and Karlsson, L. 2010. Influence of cooling rate on microstructure and
475 properties of high strength steel weld metal. *Science and Technology of Welding and Joining*, 15, 233-
476 238.
- 477 [46] Hu, J., Du, L.X. and Wang, J.J. 2012. Effect of cooling procedure on microstructures and
478 mechanical properties of hot-rolled Nb-Ti bainitic high strength steel. *Material Science and*
479 *Engineering A*, 554, 79-85.
- 480 [47] EN 1993-1-1, 2005. *Eurocode 3: Design of steel structures - Part 1-1: General rules and rules*
481 *for buildings*. Brussels: European Committee for Standardization.
- 482 [48] Hill, H.N. 1944. Determination of stress-strain relations from the offset yield strength values.
483 Technical note no. 927. National Advisory Committee for Aeronautics. Washington, DC.
- 484 [49] Ramberg, W. and Osgood, W.R. 1943. Description of stress-strain curves by three parameters.
485 Technical note no. 902. National Advisory Committee for Aeronautics. Washington, DC.
- 486 [50] Mirambell, E. and Real, E. 2000. On the calculation of deflections in structural stainless steel
487 beams: an experimental and numerical investigation. *Journal of Constructional Steel Research*, 54,
488 109-133.
- 489 [51] Gardner, L. and Nethercot, D.A. 2004. Experiments on stainless steel hollow sections - Part 1:
490 Material and cross-sectional behaviour. *Journal of Constructional Steel Research*, 60, 1291-1318.
- 491 [52] Cruise, R.B. and Gardner, L. 2008. Residual stress analysis of structural stainless steel sections.
492 *Journal of Constructional Steel Research*, 64, 352-366.
- 493 [53] Gardner, L. and Cruise, R.B. 2009. Modeling of residual stresses in structural stainless steel
494 sections. *Journal of Structural Engineering*, 135, 42-53.
- 495 [54] Quach, W.M., Teng, J.G. and Chung, K.F. 2006. Finite element predictions of residual stresses in
496 press-braked thin-walled steel sections. *Engineering Structures*, 28, 1609-1619.
- 497 [55] Amouzegar, H., Schafer, B.W. and Tootkaboni, M. 2016. An incremental numerical method for
498 calculation of residual stresses and strains in cold-formed steel members. *Thin-walled Structures*, 106,
499 61-74.



(a) W-series

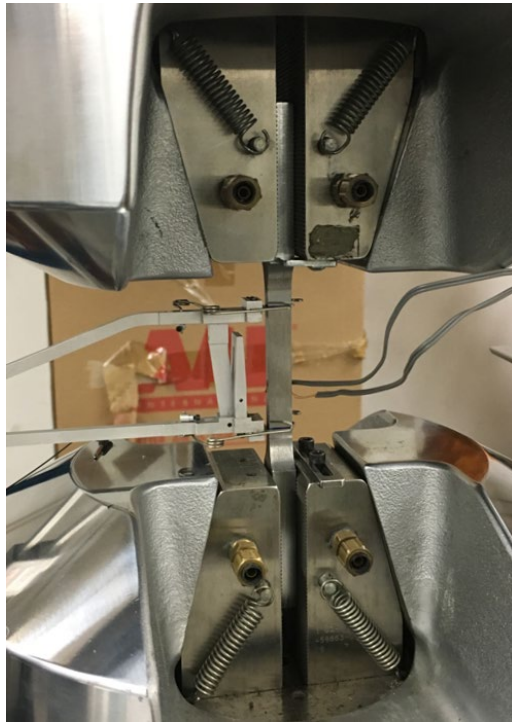


(b) CF1-series



(c) CF2-series

Figure 1. Fabrication routes for octagonal steel hollow sections.

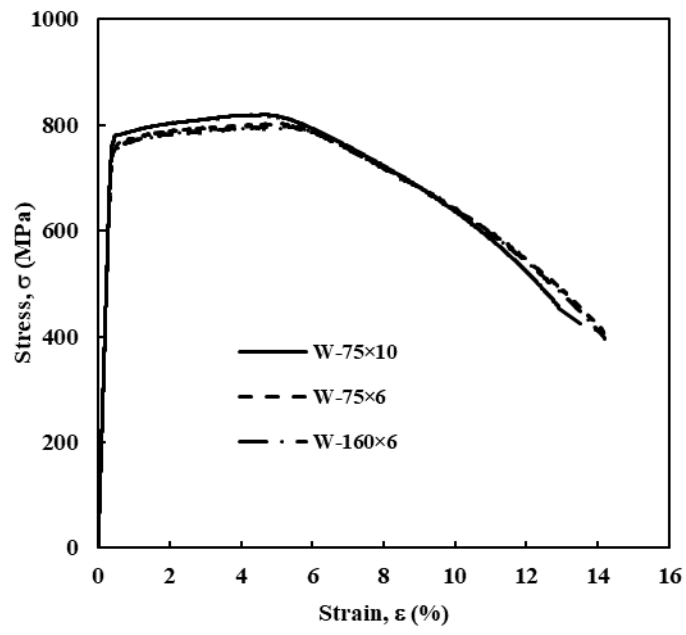


(a)

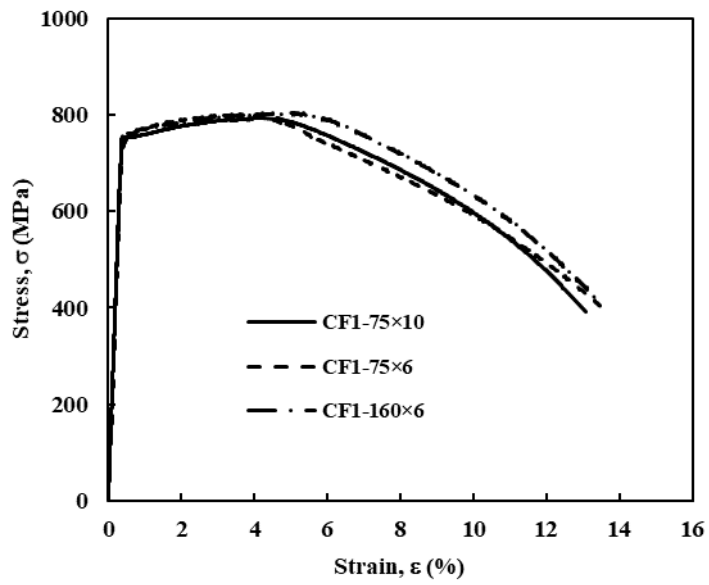


(b)

Figure 2. Set-up of tensile coupon test for (a) flat coupon and (b) corner coupon.

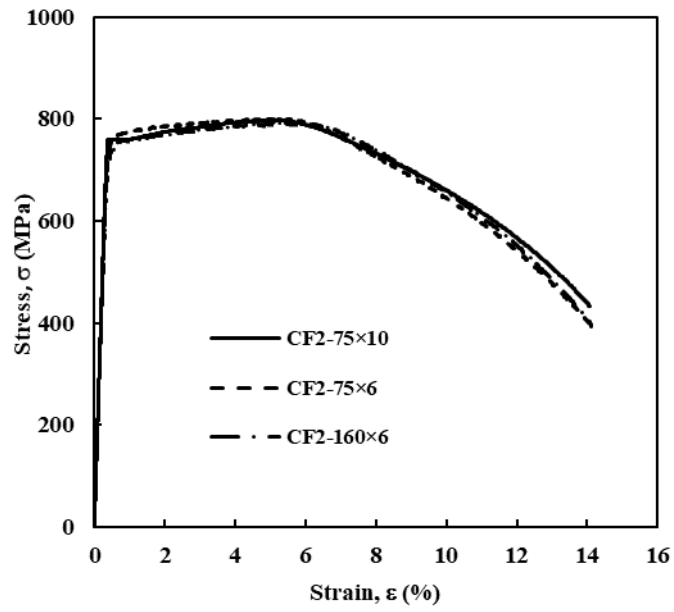


(a)



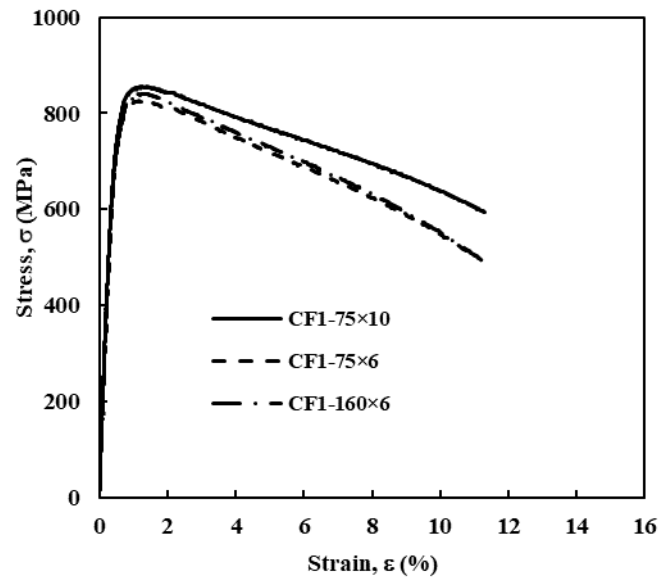
(b)

Figure 3/19

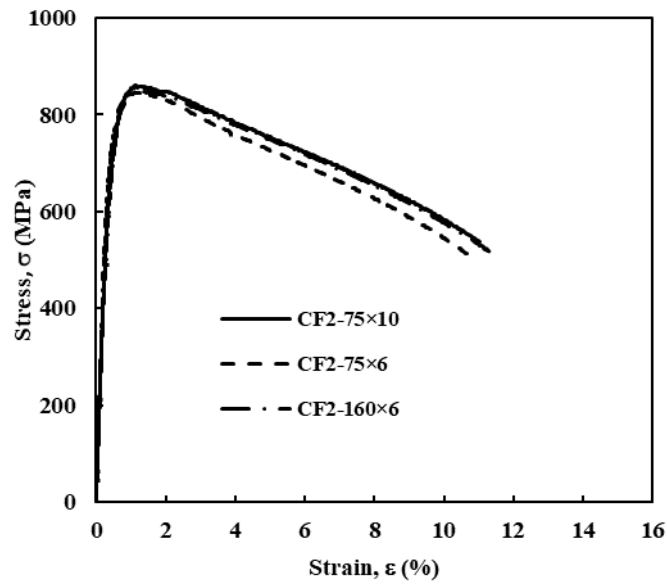


(c)

Figure 3. Measured static stress-strain curves for flat coupons obtained from (a) W series sections; (b) CF1 series sections and (c) CF2 series sections.



(a)



(b)

Figure 4. Measured static stress-strain curves for corner coupons obtained from (a) CF1 series sections and (b) CF2 series sections.

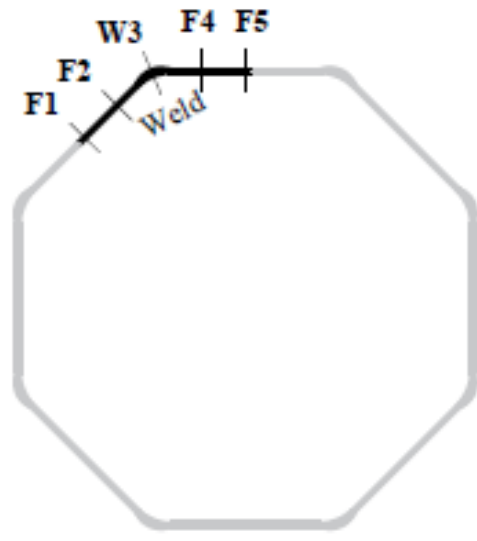
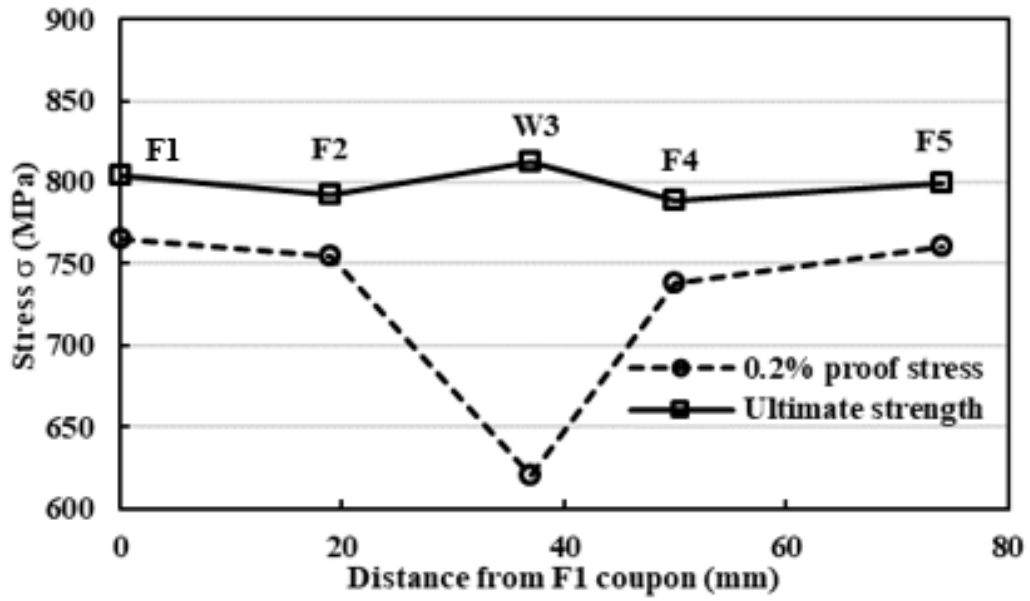


Figure 5. Variation of 0.2% proof stress ($\sigma_{0.2}$) and ultimate strength (σ_u) in W-75x6 section.

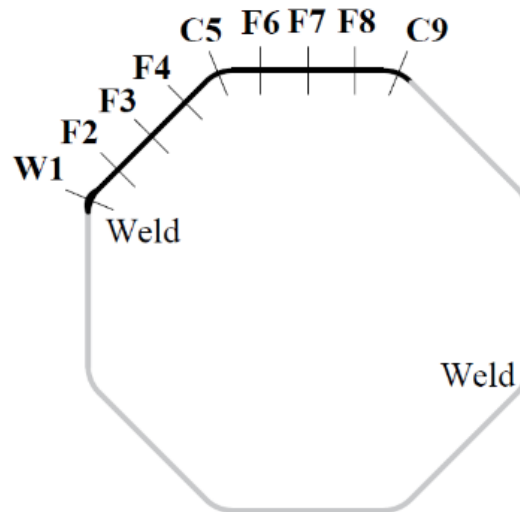
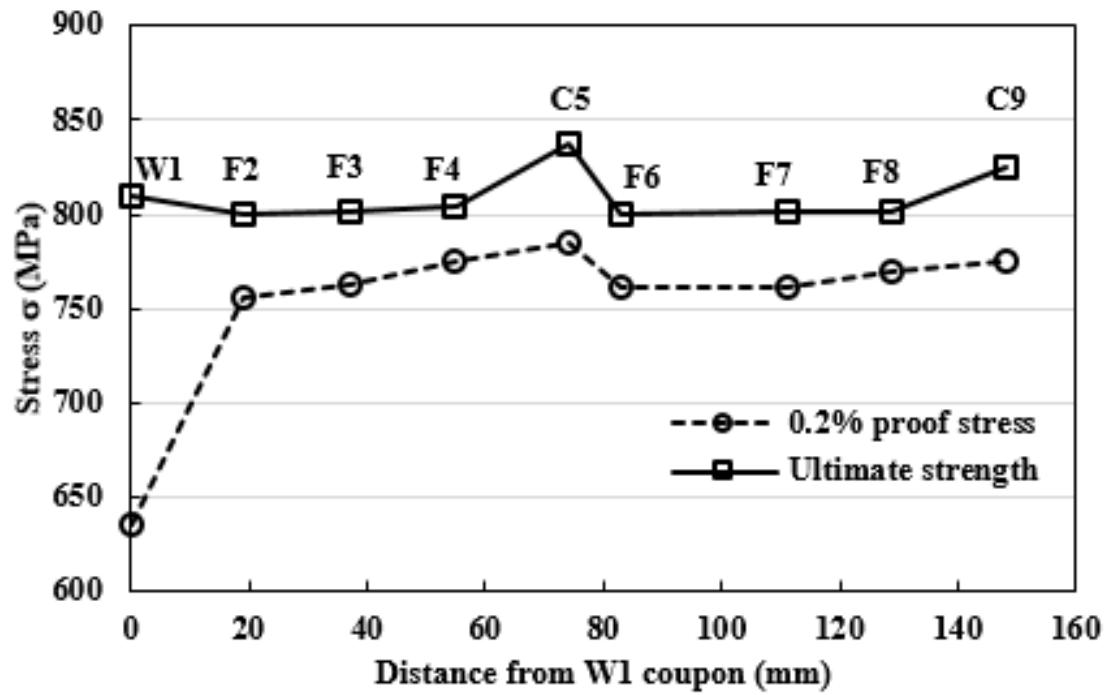


Figure 6. Variation of 0.2% proof stress ($\sigma_{0.2}$) and ultimate strength (σ_u) in CF1-75 \times 6 section.

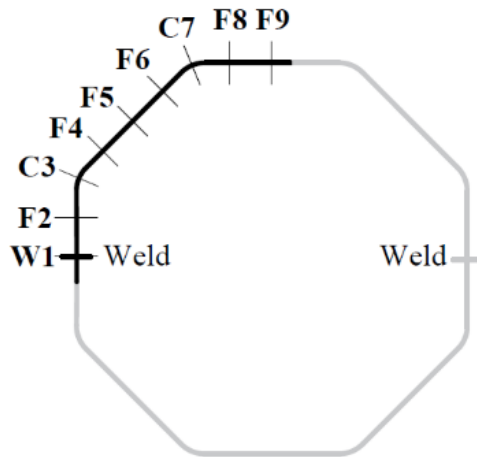
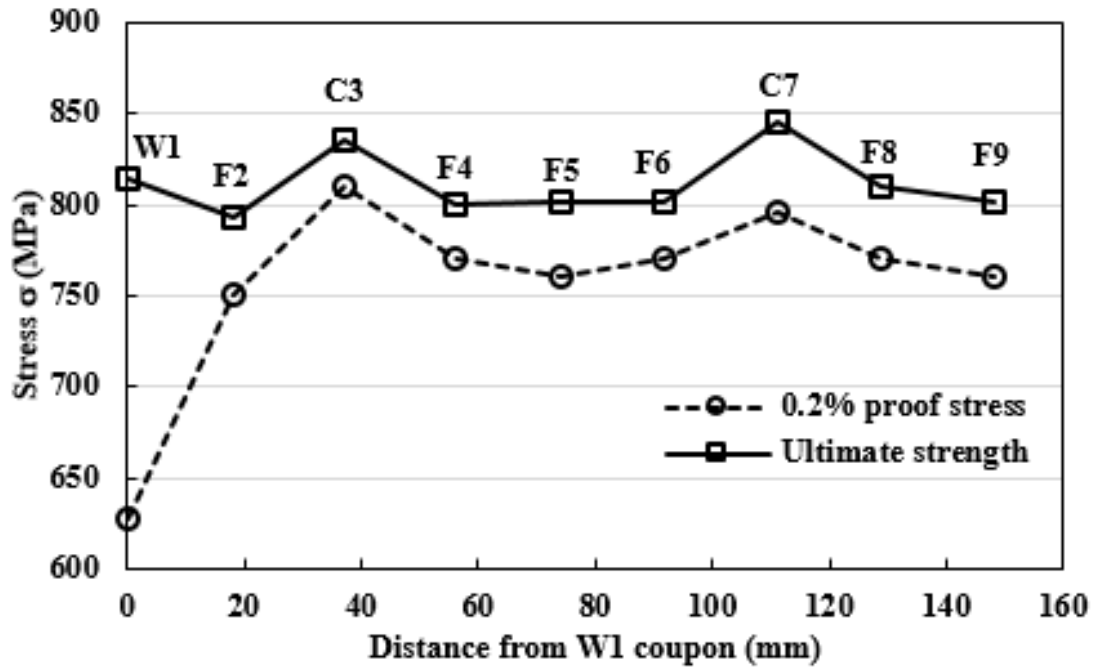


Figure 7. Variation of 0.2% proof stress ($\sigma_{0.2}$) and ultimate strength (σ_u) in CF2-75x6 section.

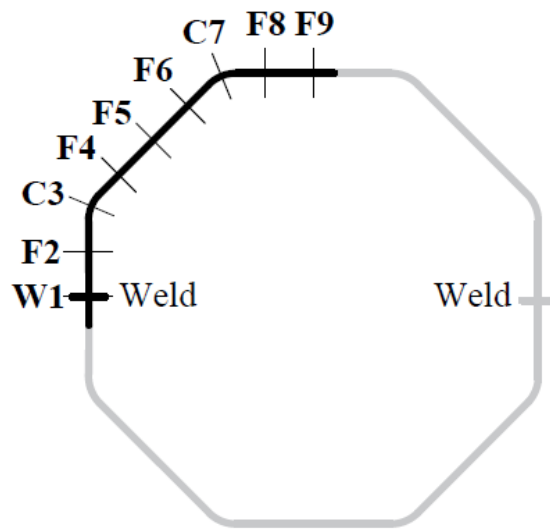
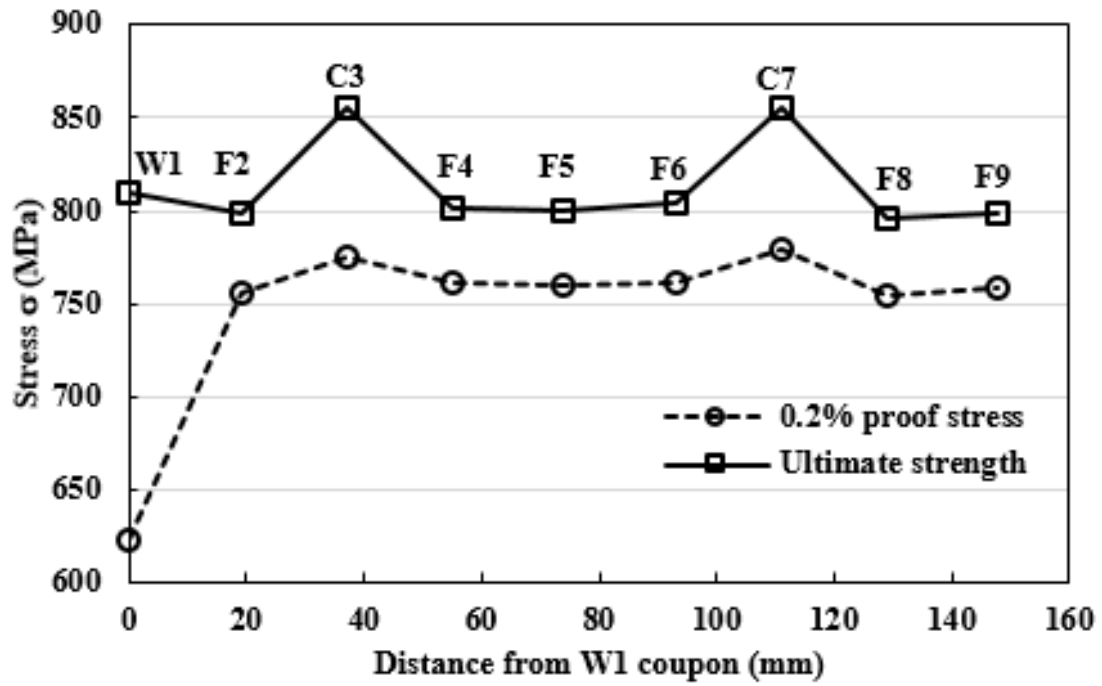


Figure 8. Variation of 0.2% proof stress ($\sigma_{0.2}$) and ultimate strength (σ_u) in CF2-75x10 section

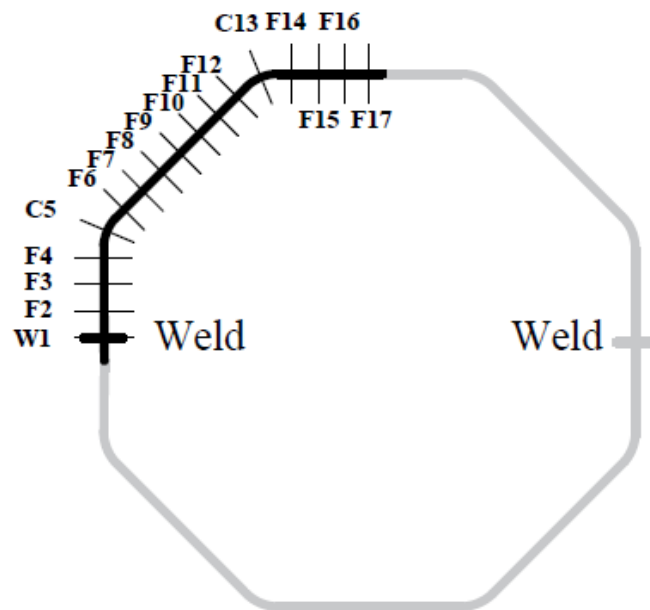
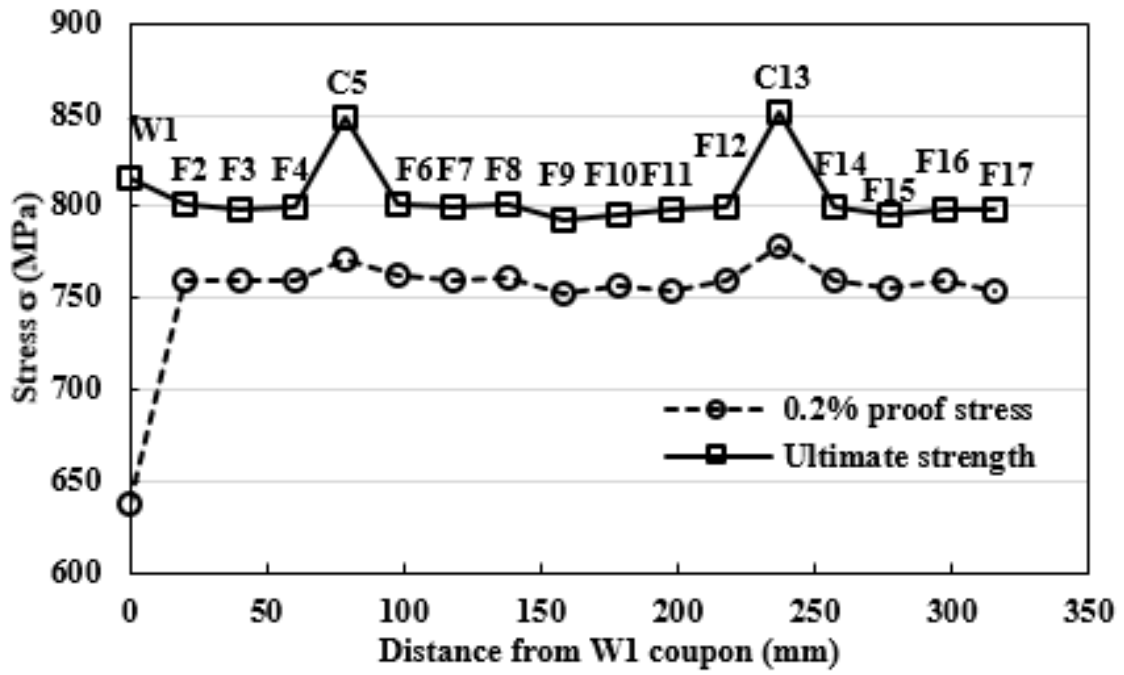


Figure 9. Variation of 0.2% proof stress ($\sigma_{0.2}$) and ultimate strength (σ_u) in CF2-160×6 section.

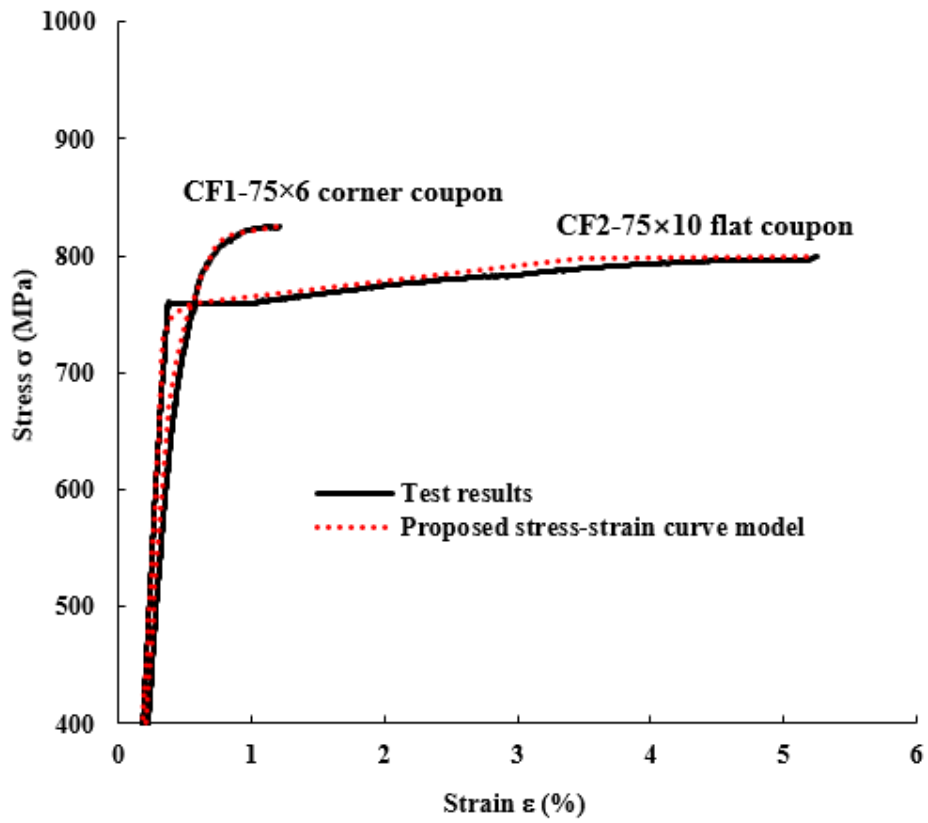


Figure 10. Comparison of stress–strain curves obtained from test results with the curves predicted using the proposed stress–strain curve model.

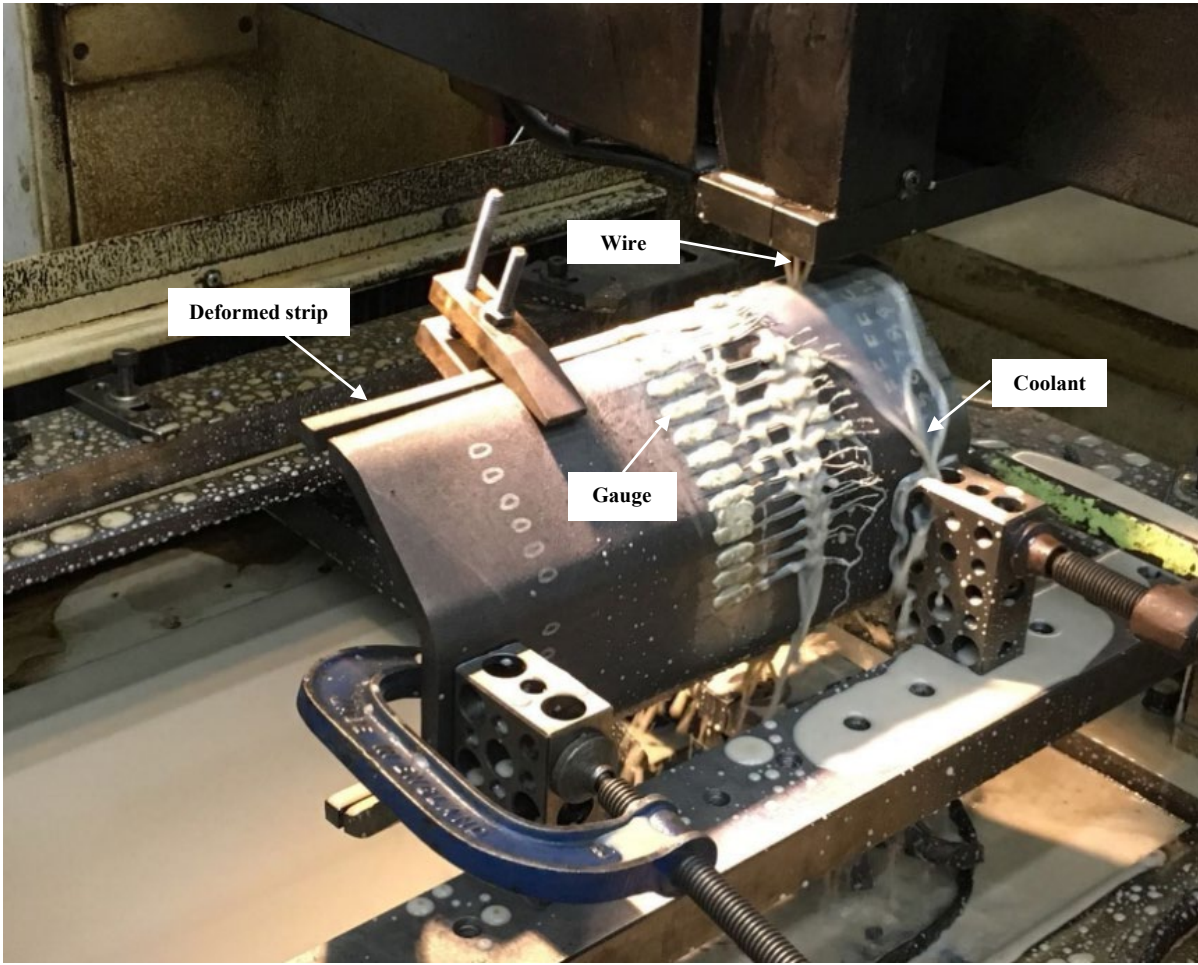


Figure 11. Sectioning process using wire-cutting with coolant.

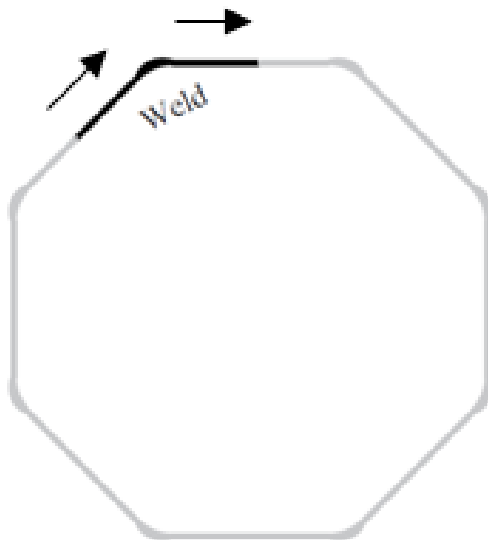
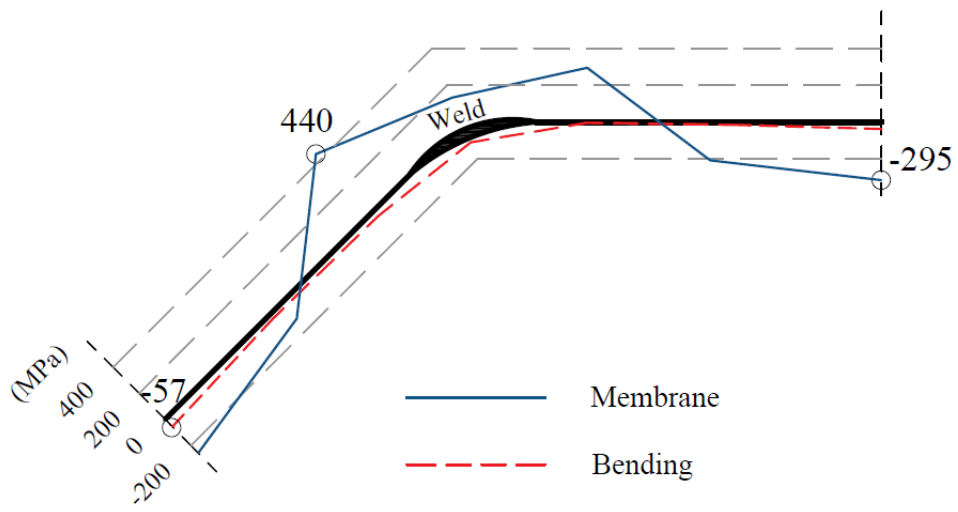


Figure 12. Membrane residual stresses and bending residual stresses on the outer surface along one-eighth of the W-75×6 section.

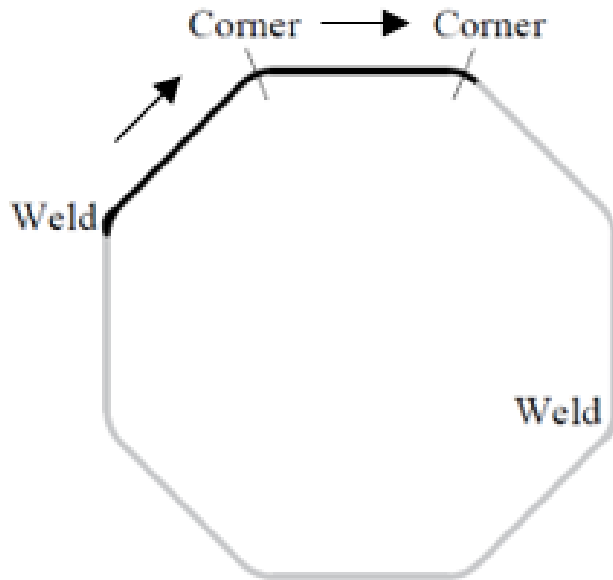
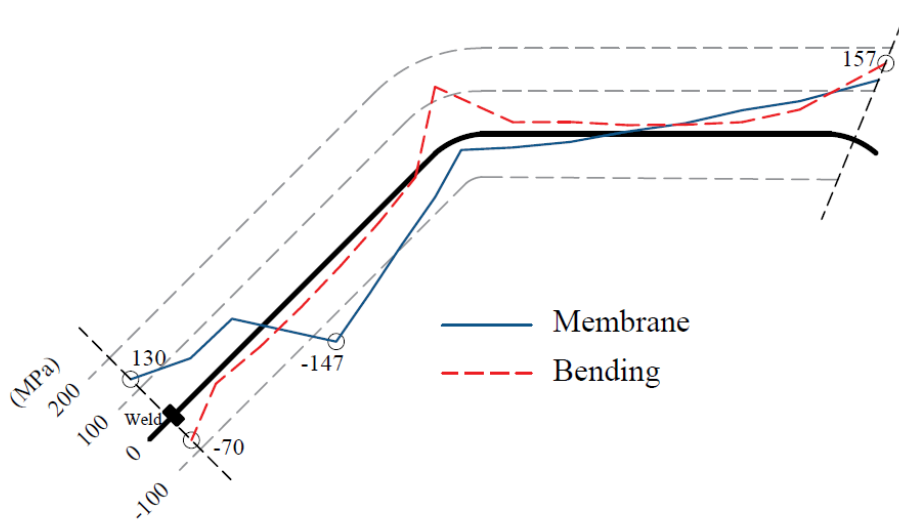


Figure 13. Membrane residual stresses and bending residual stresses on the outer surface along one-eighth of the CF1-75×6 section.

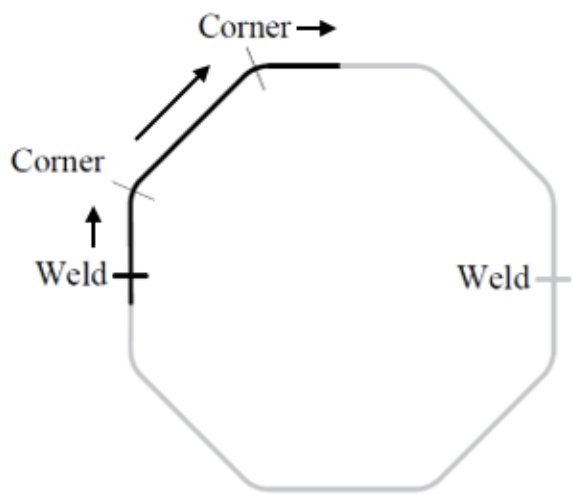
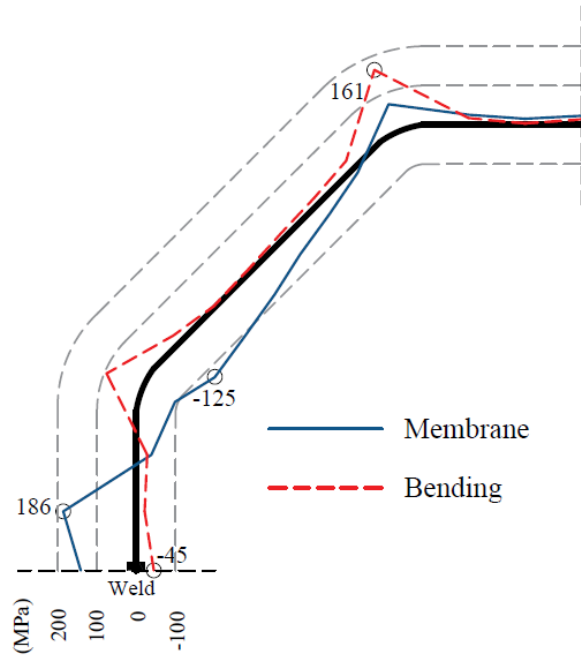


Figure 14. Membrane residual stresses and bending residual stresses on the outer surface along one-eighth of the CF2-75×6 section.

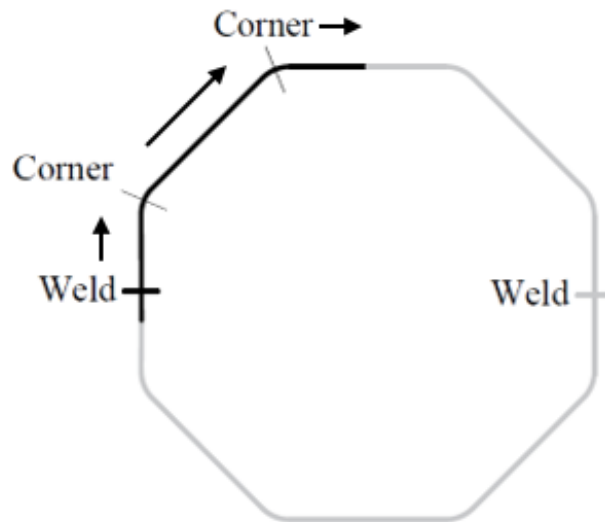
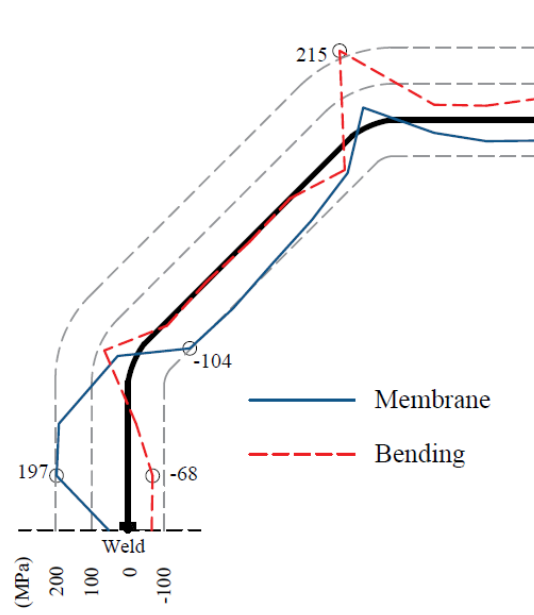


Figure 15. Membrane residual stresses and bending residual stresses on the outer surface along one-eighth of the CF2-75×10 section.

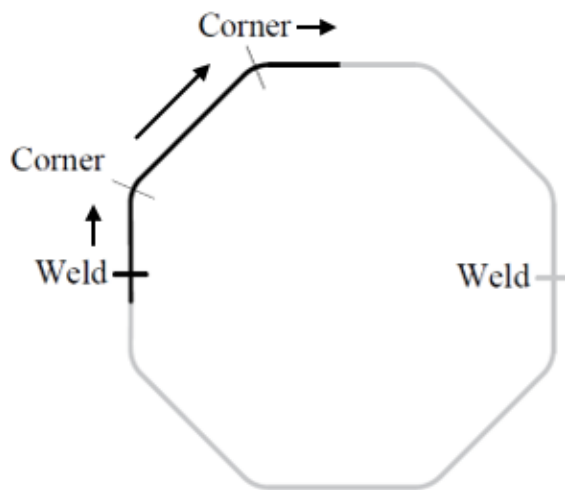
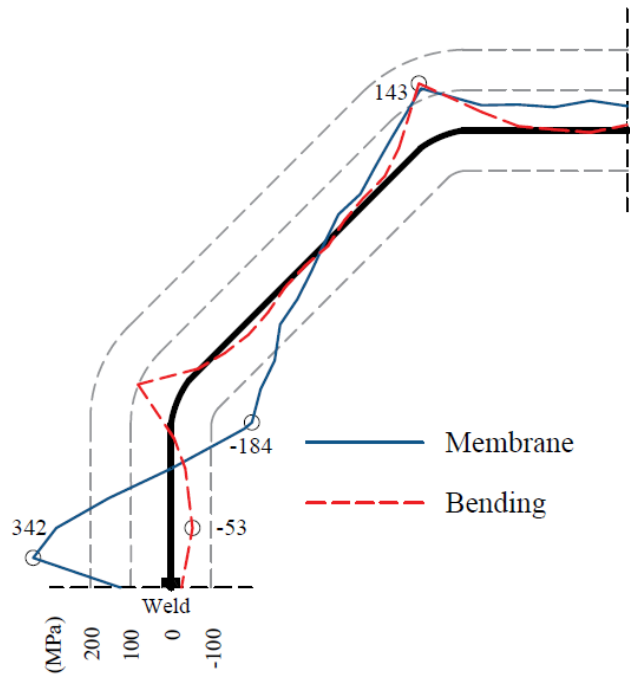


Figure 16. Membrane residual stresses and bending residual stresses on the outer surface along one-eighth of the CF2-160×6 section.

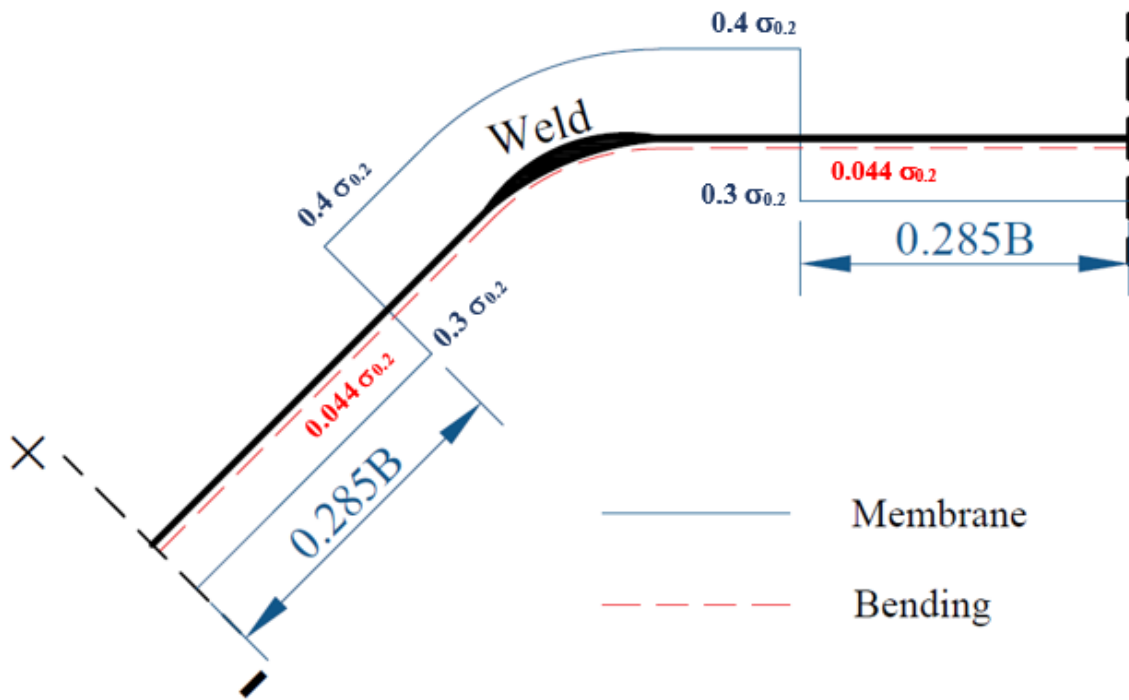


Figure 17. Residual stress distribution model for W-series sections.

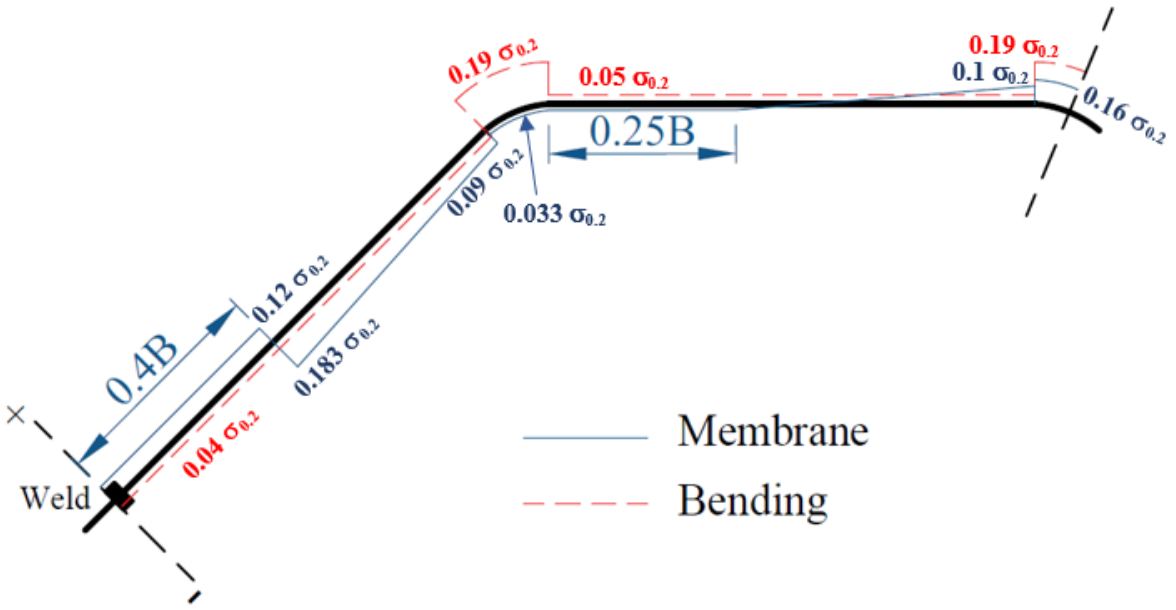


Figure 18. Residual stress distribution model for CF1-series sections.

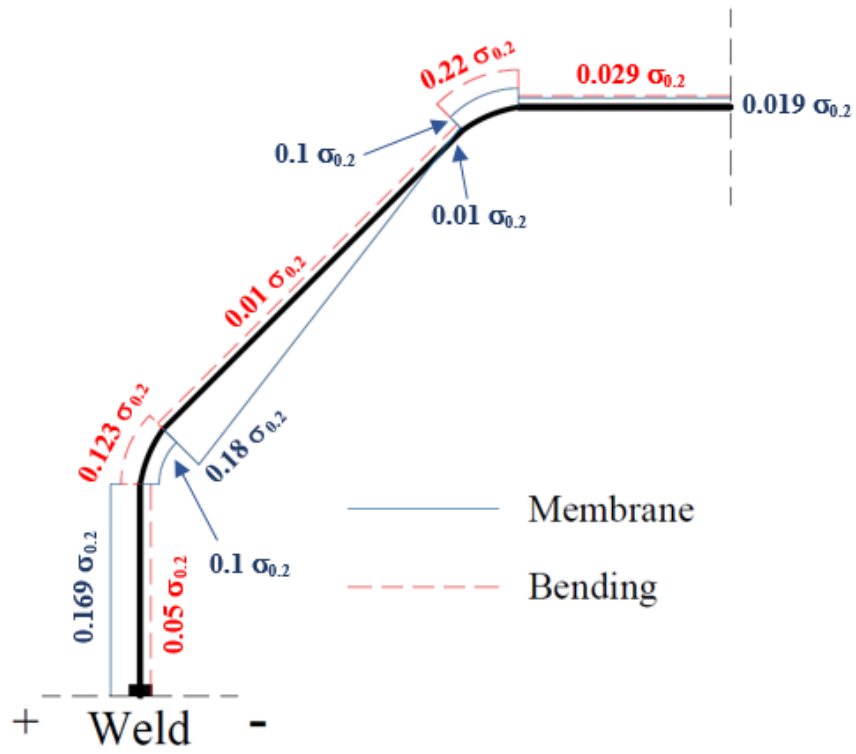


Figure 19. Residual stress distribution model for CF2-series sections.

Table 1. Dimensions of octagonal hollow sections.

Section	Edge length B (mm)	Thickness t (mm)	Outer corner radius r_o (mm)	Inner corner radius r_i (mm)
W-75×10	74.8	9.86	-	-
W-75×6	73.9	6.00	-	-
W-160×6	159.6	5.98	-	-
CF1-75×10	74.2	9.92	25.0	15.3
CF1-75×6	74.1	6.08	19.4	13.5
CF1-160×6	157.9	6.01	21.0	15.0
CF2-75×10	74.1	9.97	25.0	15.0
CF2-75×6	74.5	6.10	20.2	14.0
CF2-160×6	159.7	6.02	20.3	14.3

Table 2. Flat tensile coupon test results.

Section	E (GPa)	$\sigma_{0.2}$ (MPa)	σ_u (MPa)	ϵ_u (%)	ϵ_f (%)
W-75×10	215	780	821	4.6	13.5
W-75×6	214	764	804	4.5	14.2
W-160×6	213	764	808	4.7	14.2
CF1-75×10	215	753	795	4.6	14.0
CF1-75×6	210	762	802	4.1	13.7
CF1-160×6	215	766	805	5.1	13.5
CF2-75×10	216	759	799	5.3	14.1
CF2-75×6	215	760	801	4.5	14.1
CF2-160×6	215	754	798	5.0	14.1

Table 3. Corner tensile coupon test results.

Section	E (GPa)	$\sigma_{0.2}$ (MPa)	σ_u (MPa)	ϵ_u (%)	ϵ_f (%)
CF1-75×10	202	780	856	1.4	11.3
CF1-75×6	191	775	825	1.2	10.9
CF1-160×6	192	775	841	1.3	11.4
CF2-75×10	201	773	859	1.2	11.0
CF2-75×6	200	796	845	1.1	10.9
CF2-160×6	192	778	852	1.4	11.2

Table 4. Static mechanical properties measured for coupons taken from different locations of one-eighth of W-75×6 section and of a quarter of CF1-75×6, CF2-75×6, CF2-75×10 and CF2-160×6 sections.

Section	Coupon	E (GPa)	$\sigma_{0.2}$ (MPa)	σ_u (MPa)	ϵ_u (%)	ϵ_f (%)
W-75×6	F1	214	765	804	4.5	14.1
	F2	212	755	793	3.7	13.3
	W3	200	621	811	12.7	22.1
	F4	215	738	789	4.2	13.2
	F5	214	761	800	4.5	14.1
CF1-75×6	W1	202	635	810	12.5	22.2
	F2	214	756	800	3.7	12.0
	F3	210	763	802	3.8	13.6
	F4	214	775	804	4.0	12.6
	C5	190	785	838	1.4	11.0
	F6	213	762	800	4.0	12.5
	F7	212	762	802	4.1	13.8
	F8	210	770	802	3.7	12.5
	C9	191	775	825	1.2	10.9

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CF2-75×6	W1	199	628	814	12.3	21.5
	F2	215	751	793	4.5	12.0
	C3	200	810	835	1.4	11.1
	F4	211	770	800	4.5	12.6
	F5	213	760	801	4.5	13.4
	F6	210	771	801	2.5	11.8
	C7	200	796	845	1.1	10.9
	F8	214	770	810	4.1	11.3
	F9	214	760	801	4.5	13.9
CF2-75×10	W1	195	623	810	11.9	21.3
	F2	214	756	799	4.7	13.8
	C3	202	775	855	1.3	11.1
	F4	215	762	801	4.9	13.4
	F5	215	760	800	4.3	13.0
	F6	216	761	804	5.1	13.5
	C7	201	773	859	1.2	11.0
	F8	215	754	796	4.6	13.5

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	F9	216	759	799	5.3	14.1
CF2-160×6	W1	198	638	815	13.0	22.6
	F2	213	760	801	3.9	12.5
	F3	214	760	799	4.0	13.4
	F4	211	759	800	3.8	12.9
	C5	195	771	848	1.2	10.9
	F6	214	762	801	3.7	12.7
	F7	211	760	800	4.0	13.2
	F8	210	761	801	4.4	13.6
	F9	211	753	793	4.2	13.0
	F10	212	757	795	4.3	13.2
	F11	209	754	799	3.6	12.6
	F12	210	760	800	4.2	12.8
	C13	192	778	852	1.4	11.2
	F14	210	760	800	4.9	14.0
	F15	208	756	795	4.5	13.7
	F16	209	760	798	4.3	13.8

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	F17	215	754	798	5.0	14.1
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